APPENDIX G: ENGINEERING REPORT APPENDIX

2020 SUPPLEMENT NO. 2 TO THE 2007 FINAL SUPPLEMENT NO. 1 TO THE 1982 YAZOO AREA PUMP PROJECT FINAL ENVIRONMENTAL IMPACT STATEMENT (FEIS)

APPENDIX G

ENGINEERING SUMMARY

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SECTION 1 - GENERAL

AUTHORIZATION

PROJECT AUTHORIZATION

1. The Yazoo Basin Reformulation Study was an evaluation of a remaining unconstructed feature of the authorized Federal flood control project for the Yazoo Basin. The Reformulation Study was divided into four major features and included a thorough analysis of engineering, economic, and environmental aspects of project alternatives. The Reformulation Study included the following features: (1) Upper Steele Bayou Project, (2) Upper Yazoo Projects (UYP), (3) Yazoo Backwater Project, and (4) Headwater Tributaries Project. Reports for project features (1) and (2) were completed in 1993 and 1994, respectively. This Engineering Summary discusses and documents the proposed plan for Feature 3 – Yazoo Backwater Project. The Headwater Tributaries Project Study has not been completed.

REPORT AUTHORITY

2. The Flood Control Act (FCA) of 1941, dated 18 August 1941 (House Document (HD)/359/77/1), as amended by FCAs of 22 December 1944 and 27 October 1965 (HD/308/88/2), and the Water Resources Development Act of 1986 and 1996, authorized the Yazoo Backwater Project. The FCA of 1941 provided for the extension of a levee along the west bank of the Yazoo River from the Mississippi River levee to Yazoo City, Mississippi. Also included in the authorized plan of 1941 was a structure at Little Sunflower River and a combination structure and pump station at Big Sunflower River, Deer Creek, and Steele Bayou with a total pumping capacity of 14,000 cubic feet per second (cfs).

3. The FCAs of 1944 and 1965 extended the project to include approximately 38 miles of levee on the east bank of the Yazoo River and features for fish and wildlife.

PURPOSE OF REPORT

4. This Engineering Summary documents engineering studies performed on the design, operation, maintenance, and their relationship with the proposed plan.

PRIOR STUDIES, REPORTS, AND EXISTING WATER PROJECTS

MISSISSIPPI RIVER LEVEES

5. The Mississippi River Levees project was authorized by the FCA of 15 May 1928, as modified and amended in subsequent Acts of 23 April 1934, 15 June 1936, 18 August 1941, 24 July 1946, and 27 October 1965. The Mississippi River levees prevent inundation of the alluvial valley of the lower Mississippi River which begins at Cape Girardeau, Missouri and gently slopes to the Gulf of Mexico. The main stem levees protect a number of major cities and towns as well as industrial areas, farmland, and wildlife habitats of woodlands and marshes. The Mississippi River levees protect the alluvial valley against the flooding from the Mississippi River by confining flow to the leveed channel except where it enters natural backwater areas or is diverted purposely into floodway areas.

6. A major Mississippi River flood in 1973 led to the development of the Refined 1973 Mississippi River and Tributaries (MR&T) Project Flood Flowline, which enabled levee deficiencies along the main stem levees to be identified. An Environmental Impact Statement (EIS) was prepared in 1976 to address environmental impacts of the work needed to address the identified deficiencies. A reevaluation of the project was completed in 1998 on the remaining work along with a Supplement to the final EIS. This report documented that of the 460.4 miles of levee in the Vicksburg District, 216.8 miles need to be enlarged and raised to grade with placement of approximately 57.4 miles of seepage control measures. Of these amounts, 69.4 miles of levee enlargement and approximately 30 miles of associated seepage control are required in Mississippi generally in the area south of Greenville, Mississippi. This work is ongoing. During high stages on the Mississippi River, seepage enters into the Yazoo Study Area from beneath the Mississippi River levee. Although the U.S. Army Corps of Engineers cannot prevent the seepage, it is managing it by the construction of relief wells and seepage berms to protect the integrity of the Mississippi River levee.

PRIOR STUDIES AND REPORTS IN THE YAZOO STUDY AREA

7. Previous reports and studies that are pertinent to the Yazoo Basin Reformulation Study and the proposed plan are listed below:

a. Big Sunflower, Little Sunflower, Hushpuckena, and Quiver Rivers, and their Tributaries, and Deer Creek, Steele Bayou, and Bogue Phalia, Mississippi, General Design Memorandum (GDM) No. 1, September 1955. This report proposed a system of channel improvement along these area rivers and tributaries.

b. Annex M to the Mississippi River and Tributaries, Comprehensive Review Report, Big Sunflower River Basin, 16 November 1959. This report recommended that the scope of the existing authorized project for the Big Sunflower River Basin be increased to provide greater channel capacity on Steele Bayou and its tributaries.

c. Big Sunflower, Little Sunflower, Hushpuckena, and Quiver Rivers, and their Tributaries, and Deer Creek, Steele Bayou, and Bogue Phalia, Mississippi, Supplement A (to GDM No. 1), April 1962. This report recommended modifications to project streams as proposed in GDM No. 1.

d. Supplement B (to GDM No. 1), October 1963. Prompted by local interests, this report modified GDM No. 1 to add channel improvement to a reach of Quiver River.

e. Steele Bayou, Main Canal - Riverside Drainage District (Canal No. 9) and Black Bayou, Supplement C (to GDM No. 1), February 1964. This supplement recommended more extensive improvement on Steele Bayou, Main Canal, and Black Bayou than those proposed in GDM No. 1 and modified in Annex M.

f. Muddy Bayou Report (Eagle Lake), December 1969, was prepared in response to requests by the Warren County Board of Supervisors, the Mississippi Game and Fish Commission, and other local interests. As a result of the report, the Yazoo Backwater Project was modified to include the Muddy Bayou Control Structure. The water control structure, approved and completed in 1970 and 1977, respectively, allows manipulation of lake levels between Eagle Lake and Steele Bayou for improvement of water quality and fishery resources in the lake. The structure also provides incidental flood protection for properties along Eagle Lake.

g. Yazoo Basin, Yazoo Backwater Area, Fish and Wildlife Mitigation Plan Report, dated July 1976, and approved by the Chief of Engineers on 03 December 1976, authorized construction of nine greentree reservoirs and nine slough control structures in the Delta National Forest. These features as proposed would mitigate the fish and wildlife losses caused by the Yazoo Backwater Project. Six greentree reservoirs and five slough control structures have been completed. The others were eliminated due to unsuitable site conditions and problems with existing easement.

h. Steele Bayou Basin, Plan Formulation, GDM No. 18, August 1976. This report recommended modifying the authorized project to provide additional channel improvements on Steele Bayou and Black Bayou.

i. Yazoo Basin, Yazoo Backwater Area Pump Project Report, July 1982, presented a reevaluation of the economic feasibility of the pumping stations features of the backwater project. This report recommended installation of a 17,500-cfs pumping station at Steele Bayou. In December 1985, the plan changed because budgetary guidance directed by the Work Allowance of 1986 did not provide funds for the 17,500-cfs pumping station. Instead, the allowance provided funds for Engineering and Design for a 10,000-cfs capacity pumping station to be located approximately one mile west of the existing Steele Bayou structure.

j. Fish and Wildlife Mitigation Report, July 1982, was prepared in conjunction with the reevaluation efforts of the Yazoo Area Pump Project, Yazoo Area, and the Satartia Area Backwater levee Projects. This report was used as a basis for determining the modifications that should be made to achieve a balance in the use of the backwater area's natural resources. The report included the mitigation analyses for the construction and operation of the Yazoo Area and Satartia Area Backwater Levee Projects, including the connection channel, structures, the recommended Yazoo Area Pump Project, and other appurtenances. The Fish and Wildlife Mitigation Report recommended the acquisition of 40,000 acres of woodlands through perpetual easements in the project area.

k. Yazoo Basin, Yazoo Study Area, Mississippi, Mississippi Mitigation Plan Report, October 1989, presented a proposal for mitigation implementation to compensate for terrestrial wildlife losses incurred during construction and operation of the Yazoo Area and Satartia Area levees. This report recommended the purchase of 8,400 acres of frequently flooded cleared farmland to be reforested for terrestrial wildlife habitat through the acquisition of fee title. In 1990, the U.S. Army Corps of Engineers, Vicksburg District, purchased a tract of land containing 8,800 acres – this property is referred to as the Lake George Property. It is located in Yazoo County between the Delta National Forest and the Panther Swamp National Wildlife Refuge.

l. Upper Steele Bayou Reformulation Report, December 1992. Recommendations were made in this report for additional flood control improvements in the upper Steele Bayou Basin for Black Bayou, Main Canal, Ditch 6, and Robertshaw Ditch.

m. Memorandum for President, Mississippi River Commission, 02 December 1993, subject: FC/MR&T, Yazoo Basin, Mississippi, Big Sunflower, Bogue Phalia, Little Sunflower, Holly Bluff Cutoff, Bogue Phalia Cutoff, and Dowling Bayou Channel Maintenance Project. This memorandum outlined the plan for preparing the Supplement D (to GDM No. 1) report.

n. Flood Control, Mississippi River and Tributaries, Yazoo Basin, Big Sunflower River Basin Channel Maintenance, November 1994, Supplement D to GDM No. 1. Supplement D was approved by Mississippi River Commission 1st endorsement, 1 February 1995, subject to resolution of comments.

o. Flood Control, Mississippi River and Tributaries, Yazoo Basin, Yazoo Backwater Area, Draft Reformulation Report and SEIS, September 2000.

p. Flood Control, Mississippi River and Tributaries, Yazoo Basin, Yazoo Backwater Area, Final Reformulation Report and SEIS, November 2007.

EXISTING WATER PROJECTS

8. There are five existing projects within the subarea of the Yazoo Backwater Area: Yazoo area, Satartia area, Satartia Extension area, Rocky Bayou, and Carter area. Although these projects are separate elements of the Yazoo Basin Backwater Project, they are part of the flood control measures authorized in 1941, 1944, 1965, and 1986. A brief description of the authorized improvements for these existing projects follows:

a. <u>Yazoo Area (926,000 acres)</u>. This project area is located between the east bank Mississippi River levee and the Will M. Whittington Auxiliary Channel. The area extends north from Vicksburg, Mississippi, a distance of approximately 60 miles to Belzoni, Mississippi. Authorized work in the Yazoo Area consists of a levee system 30.5 miles long, extending from the end of the east bank Mississippi River levee, generally along the west bank of the Yazoo River to a connection with the west levee of the Will M. Whittington Auxiliary Channel. This levee system includes two structures, one at Steele Bayou with a design capacity of 19,000 cfs and one at Little Sunflower River with a design capacity of 8,000 cfs, and a channel between the Sunflower River and Steele Bayou to connect the upper and lower ponding areas within the Yazoo Study Area. The levee system is completed to an interim grade of 107.0 feet, National Geodetic Vertical Datum (NGVD 29). The work also includes 24 miles of channel work, two major structures, and two river closures. This work is complete and now operational.

b. <u>Satartia Area (28,800 acres)</u>. The Satartia area is located south of Satartia, Mississippi, between the Yazoo River on the west and the hill line on the east. Authorized work in the area consists of 20 miles of levee and one major structure. Protection of this area was completed in November 1976.

c. <u>Satartia Extension Area (3,200 acres)</u>. This area is located south of the Satartia area, and protection includes 8.2 miles of levee and floodgate for drainage. Currently, no flood control features are authorized for the Satartia Extension Project.

d. <u>Rocky Bayou (14,080 acres)</u>. The Rocky Bayou area is located south of the city of Yazoo City, Mississippi, between the Yazoo River on the west and the hill line on the east.

Authorized improvements consist of about 19 miles of levee and one major structure. Levee Item 1, which is the reach along O'Neal Creek, was separated into two construction contracts: Items 1A and 1B. Item 1A, a 3.0-mile levee item, was awarded 25 March 1985 and Item 1B, a 0.7-mile reach and a small structure, was awarded on 12 November 1986, and both are complete.

e. Carter Area (102,400 acres). The Carter Area is bounded by the Yazoo River on the east and the Will M. Whittington Auxiliary Channel on the west. The area begins upstream of the confluence of the Big Sunflower and the Yazoo Rivers and extends northward to the latitude of Yazoo City. Improvements authorized for the Carter area consist of about 29 miles of levee and one major structure. No work has been initiated on this project.

PROJECT LOCATION

9. This appendix is concerned specifically with the Yazoo Study Area for the proposed plan. The area, as depicted in Figure 1-1, lies in west-central Mississippi between the Mississippi River east bank levee and the Will Whitington Channel on the east. The triangular-shape area extends northward approximately 60 miles to the latitude of Hollandale and Belzoni, Mississippi, and comprises about 926,000 acres. Big Sunflower and Little Sunflower Rivers, Deer Creek, and Steele Bayou flow through the project area. Interior drainage of the area is provided by structures at Little Sunflower River (upper ponding area) and Steele Bayou (lower ponding area).

Figure 1-1. The Yazoo Study Area for the proposed plan.

ALTERNATIVES

GENERAL

10. There were many alternative plans considered during the evaluation of the Yazoo Backwater Reformulation Study. A brief synopsis of past alternatives is given in the following paragraphs.

PAST ALTERNATIVES

11. The Yazoo Backwater Reformulation Study began by analyzing structural flood control features consisting of five pump size alternatives and a levee alternative. The five pump alternatives that were originally analyzed in the 1982 Reevaluation Report were reanalyzed. The 10,500-, 14,000-, 17,500-, 21,000-, and 24,500-cfs pumping stations were reanalyzed, and their location was to be adjacent to the Steele Bayou structure.

12. A levee alternative was developed to basically open the Big Sunflower River Basin back to Mississippi River Backwater flooding. The Yazoo Backwater levee would be realigned along the Big Sunflower and Little Sunflower Rivers to a point near Highway 49 West, where it would tie back into natural ground as shown in Figure 1-2. The levee alignment was designed to skirt the wildlife management forested areas along the Big and Little Sunflower Rivers such that minimal damage to the environment would occur. Approximately 61 structures would be required to protect the landside areas of the levee and some lengthy landside drainage ditches would also be required. The connecting channel between the Big Sunflower Basin and the Steele Bayou Basin would be closed off, thereby establishing a drainage divide between the two basins and the closure at Big Sunflower River opened to pass flows and protected to serve as a way to maintain low water levels. The Little Sunflower structure would be modified to maintain a minimum ponding area for waterfowl and aquatic habitat.

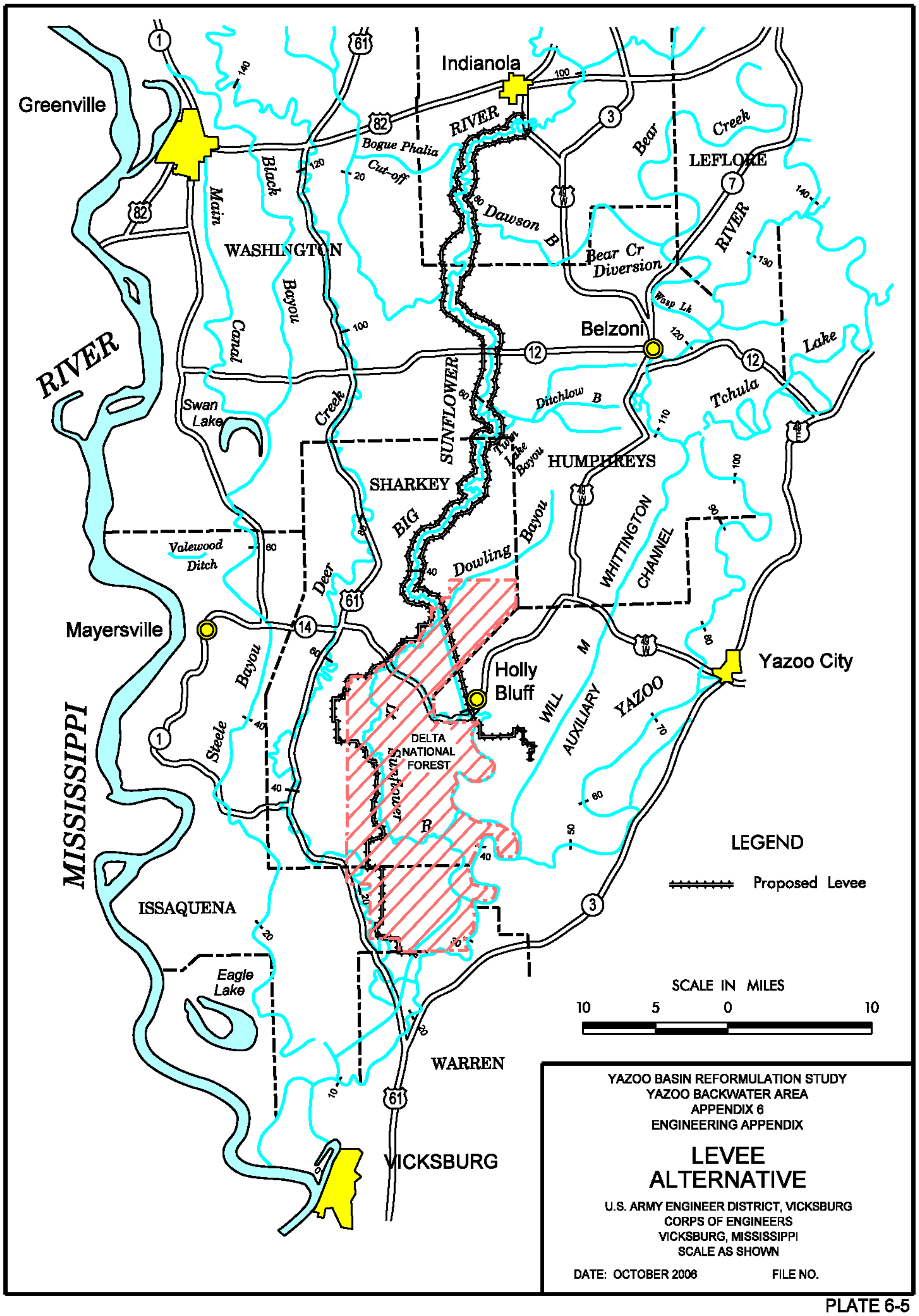

Figure 1-2. The previous levee alternative for the Yazoo Basin Reformulation Study.

13. Through the scoping and review process for the 2007 FSEIS, the 14,000-cfs pump was selected. This plan had a pump on/off elevation of 85.0 feet (NGVD 29) from December through February and an on/off elevation of 80.0 feet (NGVD 29) from March through

November. Shortly after this, several workshops were held, and a consensus group was formed with interested Federal agencies, state agencies, wildlife interests, environmental agencies, and other groups. After the workshops and consensus group meetings, a large array of alternatives were considered. These 30 alternatives (Figure 1-3) included not only structural flood control measures, but also the combination of structural and nonstructural flood control. Nonstructural flood control measures include reforestation by buying easements on open lands, nontraditional operation of the pumping station to include various ponding levels and pump on/off operation, and the purchasing of lands below the 100-year frequency flood level.

Plan	Easements: Conservation Woodlands	Easements: Reforestation Open Lands a/	Easements: Flow/Water Management	Easements: Total ($ Million)	Reforestation Acres	Reforestation ($ Million)	Environmental Impacts (HU)	Mititgation Cost ($ Million)	Structural Modifications ($ Million)	Pump ($ Million)	Total ($ Million)	Average Annual Cost $000	Average Annual Benefit $000	Excess Benefits $000
	Construction Cost													
1	Preserve Below 100.3	Use Retained	N/A	261.4	0	0	0	0	0	0	261	19,238	0	-19,238
2	Preserve Below 100.3	Reforest Below 90.0	N/A	307.8	101,800	14.3	80,070	0	0	0	330	24,265	-4,452	-28,717
NONSTRUCTURAL PLANS														
3	Preserve Below 85.0	Use Retained Below 85.0	N/A	42.1	0	0	-49,151	31.3	0	120	193	16,365	16,242	-123
4	Preserve Below 85.0	Use Retained Below 85.0	Below 80.0 b/	63.5	0	0	-41,104	26.2	0.35	120	210	17,548	16,242	-1,306
5	Preserve Below 85.0	Use Retained Below 85.0	Below 85.0 c/	81.7	0	0	-41,200	26.2	0.35	120	228	18,890	16,242	-2,648
6	Preserve Below 85.0	Reforest Below 85.0	N/A	56.0	53,000	7.4	10,608	0	0	120	187	15,574	16,900	1,326
7	Preserve Below 85.0	Reforest Below 85.0	Below 80.0 b/	70.2	53,000	7.4	21,533	0	0.35	120	202	16,654	16,900	246
8	Preserve Below 85.0	Reforest Below 85.0	Below 85.0 c/	81.7	53,000	7.4	21,390	0	0.35	120	213	17,503	16,900	-603
9	Preserve Below 90.0	Use Retained Below 90.0	N/A	85.2	0	0	-30,927	19.1	0	120	224	18,522	13,387	-5,135
10	Preserve Below 90.0	Use Retained Below 90.0	Below 80.0 b/	102.0	0	0	-9,232	5.8	0.35	120	228	18,675	13,387	-5,288
11	Preserve Below 90.0	Use Retained Below 90.0	Below 85.0 c/	117.0	0	0	-9,223	5.8	0.35	120	243	19,783	13,387	-6,396
12	Preserve Below 90.0	Reforest Below 90.0	N/A	135.0	101,800	14.3	36,022	0	0	120	276	22,155	13,883	-8,272
13	Preserve Below 90.0	Reforest Below 90.0	Below 80.0 b/	139.0	101,800	14.3	66,607	0	0.35	120	280	22,466	13,883	-8,583
14	Preserve Below 90.0	Reforest Below 90.0	Below 85.0 c/	141.0	101,800	14.3	66,616	0	0.35	120	282	22,615	13,883	-8,732
COMBINATION PLANS - 14,000 CFS PUMP a/														
15	Preserve Below 85.0	Use Retained Below 85.0	N/A	42.1	0	0	-53,614	34.2	0	143	219	18,562	18,052	-510
16	Preserve Below 85.0	Use Retained Below 85.0	Below 80.0 b/	63.5	0	0	-45,832	29.2	0.35	143	236	19,756	18,052	-1,704
17	Preserve Below 85.0	Use Retained Below 85.0	Below 85.0 c/	81.7	0	0	-45,828	29.2	0.35	143	254	21,097	18,052	-3,045
18	Preserve Below 85.0	Reforest Below 85.0	N/A	56.0	53,000	7.4	3,932	0	0	143	210	17,532	18,159	627
19	Preserve Below 85.0	Reforest Below 85.0	Below 80.0 b/	70.2	53,000	7.4	14,414	0	0.35	143	225	18,612	18,159	-453
20	Preserve Below 90.0	Reforest Below 85.0	Below 85.0 c/	81.7	53,000	7.4	14,417	0	0.35	143	236	19,461	18,159	-1,302
21	Preserve Below 90.0	Use Retained Below 90.0	N/A	85.2	0	0	-35,692	22.8	0	143	251	20,783	14,794	-59,889
22	Preserve Below 90.0	Use Retained Below 90.0	Below 80.0 b/	102.0	0	0	-11,473	7.3	0.35	143	253	20,763	14,794	-5,969
23	Preserve Below 90.0	Use Retained Below 90.0	Below 85.0 c/	117.0	0	0	-11,469	7.2	0.35	143	268	21,855	14,794	-7,061
24	Preserve Below 90.0	Reforest Below 90.0	N/A	135.0	101,800	14.3	29,534	0	0	143	299	24,113	14,917	-9,196
25	Preserve Below 90.0	Reforest Below 90.0	Below 80.0 b/	139.0	101,800	14.3	63,519	0	0.35	143	303	24,424	14,917	-9,507
26	Preserve Below 90.0	Reforest Below 90.0	Below 85.0 c/	141.0	101,800	14.3	63,523	0	0.35	143	305	24,573	14,917	-9,656
STRUCTURAL PLANS a/														
27 (14K P) d/	N/A	N/A	N/A	0.0	0.0	0.0	-63,743	40.5	0	120	161	13,990	17,539	3,549
28 (17.5K P) d/	N/A	N/A	N/A	0.0	0.0	0.0	-75,884	48.2	0	143	191	16,636	19,664	3,028
29(LEV)	N/A	N/A	N/A	0.0	0.0	0.0	-30,081	19.1	0	215	234	19,552	15,102	-4,450
30 (14K P)	Preserve Below 100.3	N/A	N/A	73.3	0.0	0.0	-63,743	39.4	0	120	233	19,348	17,539	-1,809

a/ Pump would be operated to provide flood damage reduction for cleared lands above easement elevation.
b/ 1 December - 1 March.
c/ 80 feet, NGVD, 1 December - 1 January and 15 February - 1 March; 85 feet, NGVD, 1January - 15 February.
d/ Pump would be operated to provide flood damage reduction for cleared lands above elevation 80 feet except during 1 December - 1March when pump would be operated at 85.0 feet, NGVD.
e/ Does not reflect cost of pump but of the levee.

PLATE 6-6

Figure 1-3. The 30 previous alternatives for the Yazoo Backwater Reformulation Study.

FINAL ARRAY

14. This SEIS will not reformulate the broad array of alternatives examined in the 2007 FSEIS, but will analyze an proposed plan in light of new environmental data. The proposed plan addressed in this SEIS is the remaining flood damage reduction feature of the Yazoo Basin, Yazoo Backwater, Mississippi, Project, which will include both structural (construction and operation of the pump station) and nonstructural (flood damage reduction features through acquisition and reforestation/conservation) features by updating the 2007 FSEIS recommended plan.

SECTION 2 - HYDROLOGY AND HYDRAULICS

PURPOSE OF HYDROLOGIC ANALYSIS

15. The purpose of these hydrologic analyses is to identify the base hydrologic conditions in the Yazoo Study Area and estimate the changes to those conditions resulting from various flood control alternatives. Hydrologic information summarized in this appendix has been used in other analyses, including the economic and environmental analyses.

16. This section presents the methodology used in the hydrologic analyses and explains the types of data used in the analysis which support the formulation of the various plans. Engineer Manual (EM) 1110-2-1413 was used as guidance and criteria for the hydrologic analyses.

OBJECTIVE

17. This report will update the information from, or provide new information to, the 2007 FSEIS.

INTRODUCTION

18. There are several areas with updated or completely new information that will be discussed in this Hydrology Section. This information would result in significant changes to the 2007 FSEIS. Updated information includes flooding since 1997, revising the period-of-record (POR) used in the hydrologic analysis of the project, the acquisition of a higher resolution digital elevation model (DEM) using an airplane based LIDAR, the application of the HEC-RAS 2D to model the POR to provide daily stages for the base and with-pump condition, the determination of the areal extent of floods (frequency and duration) based on the new POR utilizing the LIDAR DEM, and finally obtaining new land-use/land-cover information using the NASS-2015 coverage. New information includes daily water elevations in 59 shallow groundwater wells and paired groundwater-surface water gages, 40 of which reside in the project area. Each of these seven topics will be covered in a sub-section below.

APPROACH

19. There is information available today that was not available in 2007. This information would result in significant changes to the 2007 Backwater Project Report and SEIS. The first major change would be an alteration of the period-of-record (POR). When analyzing the base or without project condition, it is advisable to use observed data. However, the observed data must meet several prerequisite conditions. One condition is that the POR should include at least 25 years of data, but 40 or more would be better. The second condition is that the POR should include the flood of record, which is 1973. The backwater levee was completed in 1978, so the minimum 25-year POR would be 1978 to2003. The 2007 re-evaluation study started in 2000 which would not meet the minimum 25-year POR and it did not contain the flood of record. Today the observed POR is 1978 to 2019, which is 42 years. Although this does not include the 1973 flood, there have been several significant floods since 2008. The 2007 report used a POR from 1943 to 1997. Many of the gages did not exist for the entire length of the 1943 to 1997 POR. A precipitation model was used to simulate run-off and the gage data was simulated with a routing model. The POR used in this study is 1978 to 2019. This represents the POR since the

completion of the Yazoo Backwater Levee, and therefore represents the actual base condition. All stage and discharge data used in this analysis of the without project hydrology are observed not modeled. However, the with-project condition must be modeled. This report contains the annual flood frequencies and stage durations for all gaging stations in the project area. The stage-frequency evaluation was done with HEC-SSP software package. The SSP General Frequency Model was used to calculate the annual flood frequencies based on stage data.

20. The second change is in the digital elevation model (DEM) used for the GIS analysis. The 2007 study used a 30-meter resolution DEM developed by the USGS. In 2009 the basin was flown with LIDAR. The study produced over 12,000 five-kilometer square tiles, with a horizontal resolution of 1 meter and an average vertical error of approximately six inches. The 12,000 tiles were merged and resampled to a ten-meter grid. The 10-meter DEM was then used to support a GIS flood inundation model. The LIDAR DEM made significant reductions in the areal extent of the flood frequency zones, and likely impacted the number of structures inundated by each flood event. The changes in the flood frequency elevations are discussed in greater detail later in this Appendix.

21. The Flood Event Simulation Model (FESM) was used to delineate the areal extent of flooding. FESM was used to map the extents of both flood frequency and flood duration. The FESM model is a GIS flood mapping tool. It requires three ArcMap coverages, which are: a point file providing gage elevations, a line file delineating the stream center line connecting the gage locations, and a DEM. Figure 2-1 shows examples of these three layers. FESM takes the gage elevations and interpolates the elevations along the stream center line. It then extends those elevations one grid cell at a time outward from the stream centerline (one cell on each side of the centerline is one iteration). If the water surface elevation is greater than the DEM elevation, the model marks the grid cell as flooded. It progresses step wise away from the channel until no additional grid cells are flooded. The model has two mechanisms that can be used to calibrate the flood extent. The first is that you can set a minimum flood depth. A minimum of 0.25 feet was used for most of the mapping for this project. The second tool is a lateral slope adjustment. Flood surfaces are generally not flat but decrease from upstream to downstream and from the stream center outward. The point file provides the flood elevations along the stream but does not account for the slope away from the channel centerline. The lateral slope can be adjusted three ways. First you can set a constant slope, which can be zero. Second, you can allow the model to calculate the lateral slope based on the water slope within each stream segment (the stream section between gage points). When using at calculated slope, you can specify a slope factor. The slope factor is a multiple of the calculated slope. In the Big Sunflower basin, the average slope is one half foot per mile which is roughly 0.0002 feet/foot. A slope factor of 2 would make the lateral slope 0.0004 feet/foot. With 30-foot grid, a river slope of 0.0002 and a slope factor of 2, the model will decrease the water surface by 0.012 feet/grid cell per iteration. Some caution needs to be exerted when employing calculated slopes. If the water surface rises as you move downstream, the lateral slope can have a rising surface as you move away from the channel (i.e. you can flood the world) The model does allow negative slope factors, which allows mapping of a flood on the falling leg of the hydrograph. The final method is through the use of a slope table. This is an additional polyline file with a slope field, where you can specify the slope for each reach. The flood modeling in this study used the second method, with a slope factor of two. Using a constant slope of zero, would always overestimate the flooded area. Using the

calculated slope with a factor of 2, generally gave the best results, without underestimating the flooded area.

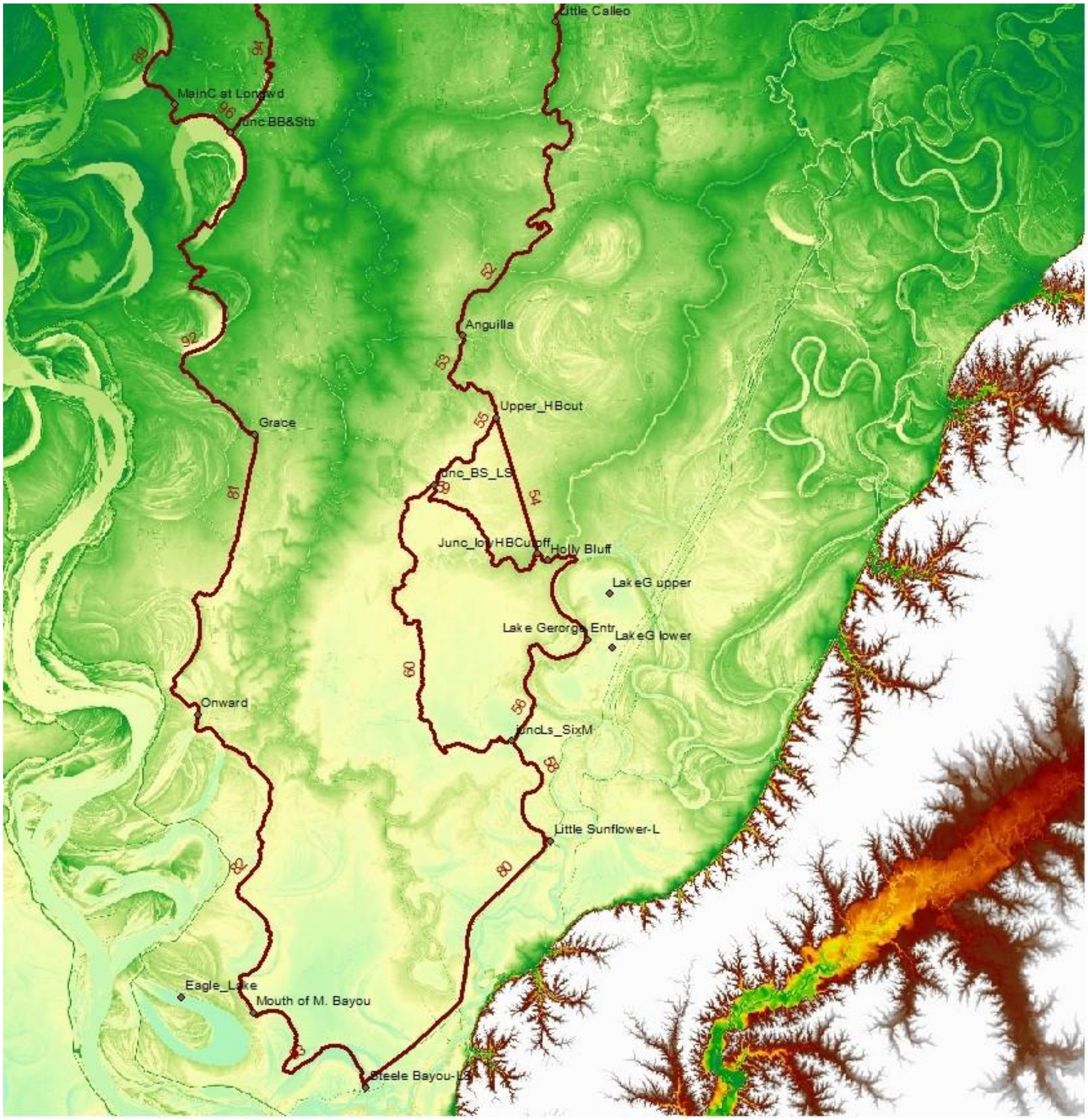

Figure 2-1. The Arc-Map data layers used in the FESM model.

22. Although the LIDAR DEM is much higher resolution than the older USGS DEM, there are some problems associated with its' use. The biggest problem is that LIDAR is reflected off of bridges, thus the raw DEM has the bridge decks. These need to be removed for flood waters to move along the channels. The contractor, which processed the DEM, did a good job of removing most of the primary and secondary road bridge decks, but they missed many of the smaller bridges or culverts. Additional processing was needed to remove the bridge decks in

Delta National Forest (DNF). FESM model calibration is accomplished by comparing an observed flood from a Landsat satellite image to a FESM simulation of that event. The greatest errors found when comparing a FESM flood to a satellite scene are due to bridge decks acting as dams. Figure 2-2 shows a FESM simulation where roads are acting as dams. To fix this, the roadbed needs to be eliminated. Figure 2-3 shows an example where the roadbed has been removed in three locations to allow flooding.

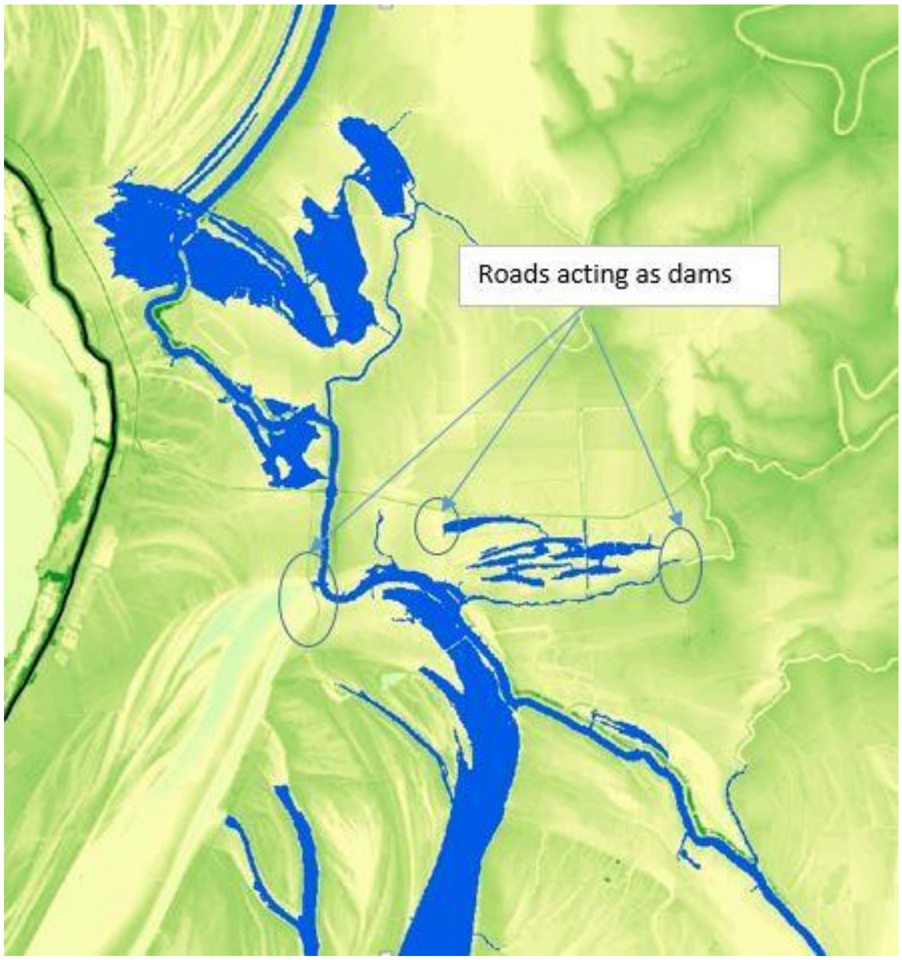

Figure 2-2. FESM simulation blocked by roads.

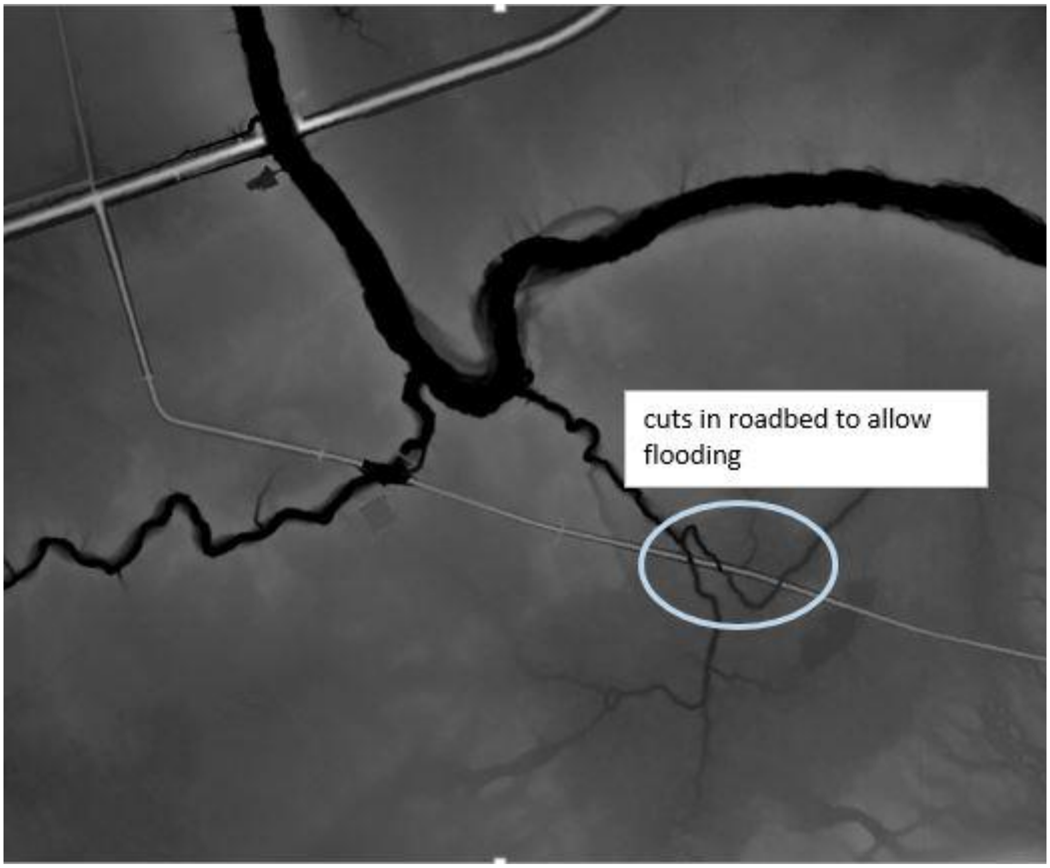

Figure 2-3. DEM with roads cut to allow flooding.

23. Two additional sources of new data are available. The first is surface ground-water elevations at 59 shallow ground-water wells in the Big Sunflower and Steele Bayou watersheds. Twenty-five of these wells were installed and maintained by MVK. The remainder were installed and maintained by ERDC. These wells recorded the surface ground water elevations in the top three feet of the soil horizon. The wells were sited based on flood frequency and duration based on the 1943-97 POR. Wells were placed in 1, 2, 5, 10, 25 and 50-year flood frequency zones, and the 7, 14, 21 and 28-day duration zones. Determination of the flood frequency and duration for the 1978 to 2019 POR, was not done until much later. Using the new POR, the wells fall in the 1, 2 and 5-year flood frequency zones and the 7, 14, 21 and 28-day duration zones. Figure 2-4 and Figure 2-5 show the MVK well positions relative the flood frequency and flood duration respectively. This data can be used to determine the degree of influence of precipitation to the water budget of wetlands in the project area. A site that remains saturated in the top thirty centimeters (cm) for 14 consecutive days meets the wetland hydrology criteria. Each well has from one to nine years of continuous depth measurements. The water depth and temperature in these wells was recorded every six hours (or hourly) with an Orpheus Mini depth transducer. The Statistical Analysis System (SAS) Univariate Procedure was used to calculate the daily average and daily maximum water surface elevation and daily average and maximum temperature for each well. SAS Univariate Procedure was also used to calculate the number of days of saturation in the top 30 cm of the well, this data was summarized by month, by year and by month and year. The daily maximum elevation was also analyzed with the WETSORT program to determine the median 7, 14, 21, 28, and 35-day durations for each well. The ground

elevation at each well was estimated with the one-meter resolution LIDAR data. The SAS Univariate Procedure was also used to calculate the number of days where the water surface was above the ground elevation at each well.

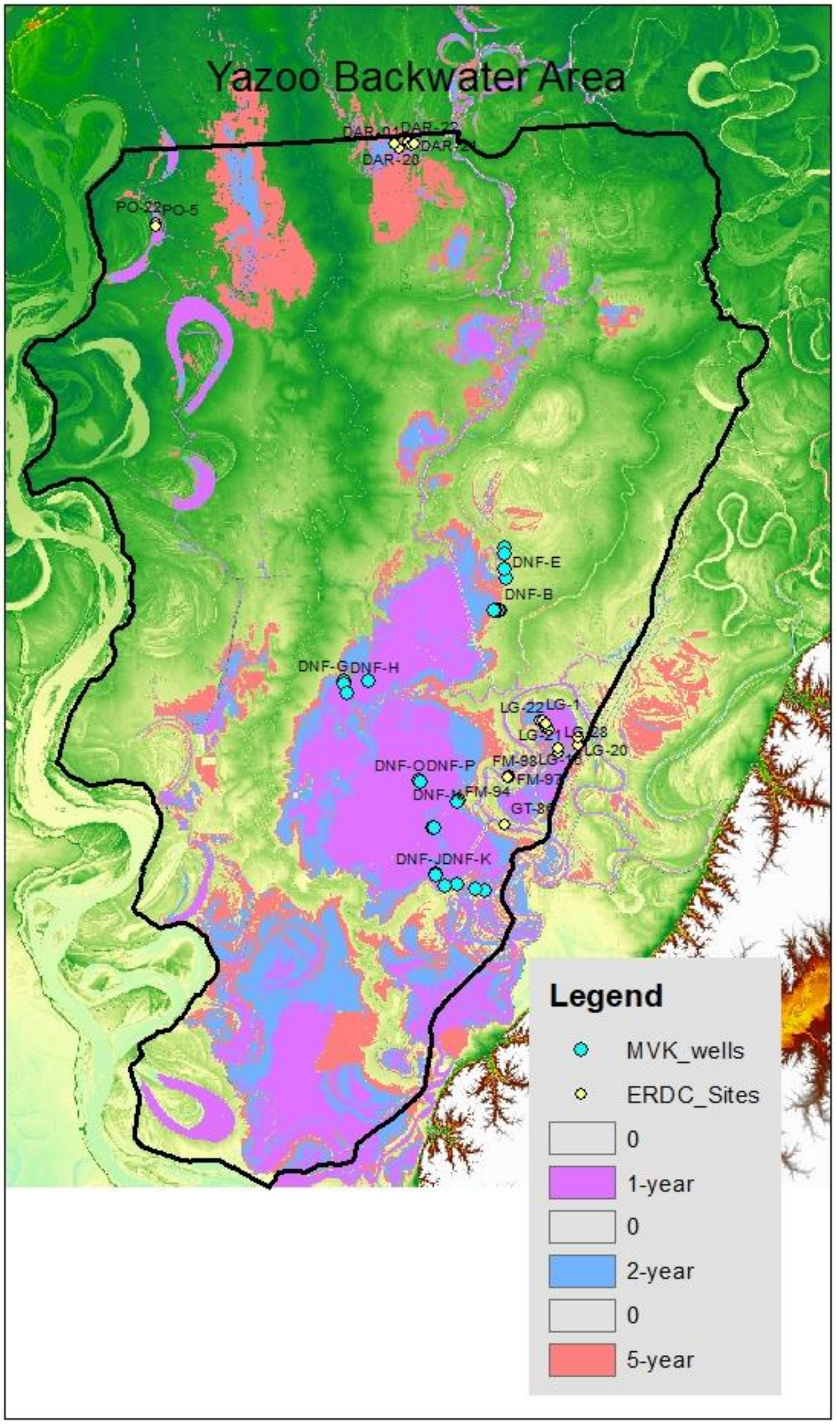

Figure 2-4. Shallow groundwater wells relative to flood frequency zones.

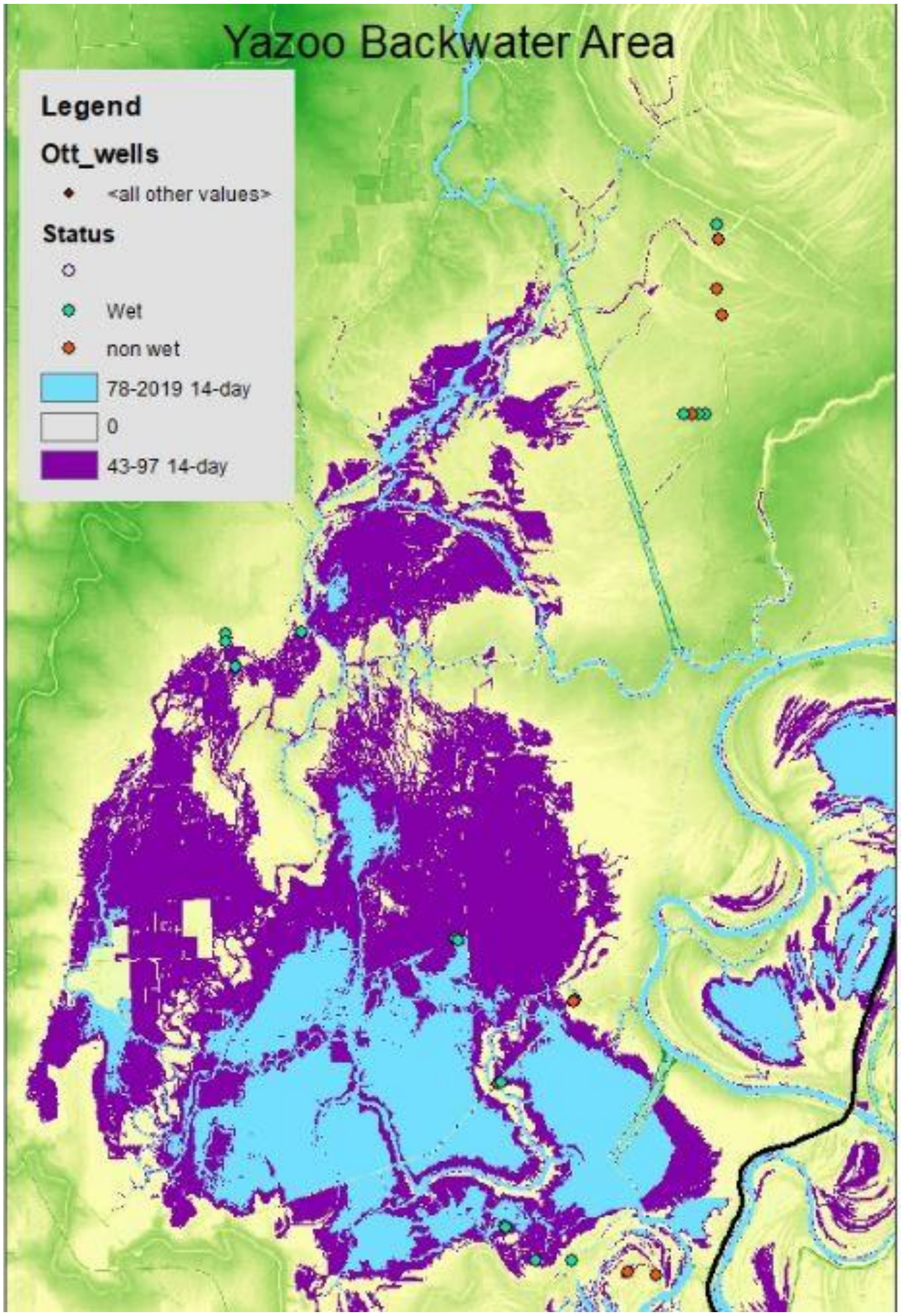

Figure 2-5. Shallow groundwater wells relative to flood duration zones.

24. A second source of new data is the availability of paired surface and ground-water gages. The USGS in conjunction with the Vicksburg District Corps of Engineers (MVK) collects hourly river stage and ground-water elevations at nine locations in the Yazoo Basin. Unlike the shallow ground-water wells, these wells extend into the alluvial aquifer. These data can be used to determine if the alluvial aquifer has any impact on wetlands in the study area (i.e., are they influenced by groundwater). Paired data were collected by these gages from 2010 to 2019 from the Big Sunflower River at Clarksdale, Sunflower and Anguilla; from Steele Bayou at Hopedale, and from Bogue Phalia at Leland. Paired data were collected from the Big Sunflower River at

Merigold, from the Quiver River at Doddsville, and from Steele Bayou at Glen Allen starting in 2014. The upper elevation of the alluvial aquifer is generally ten or more feet below the surface of the aquifer, which is significantly below the elevation in the shallow groundwater wells during periods of soil saturation.

BACKGROUND

25. The U.S. government operates flood control reservoirs all across the country. Three agencies are responsible for their operation: the U.S. Army Corps of Engineers, the Bureau of Reclamation, and the Tennessee Valley Authority. The flood control reservoirs fall into two basic categories dry dams and wet dams. Dry dams do not have a minimum, or base pool; while wet dams have a minimum pool. The Yazoo Study Area acts like a dry dam, as it only stores water during flood events. While the U.S. has with many lakes and reservoirs that can provide flood storage, many of the country's largest lakes have been modified to provide flood damage reduction. Lake Okeechobee in Florida is an example of a natural lake that has been modified by the addition of levees and flood control gates to provide downstream flood damage reduction. Where natural lakes do not exist the government has constructed large reservoirs to provide flood damage reduction. Many of these man-made reservoirs are among the largest lakes in the country (Lake Oahe, Lake Sakakawea, Toledo Bend and Lake Okeechobee). Wikipedia provides a list of the 100 largest lakes and reservoirs in the U.S. Both Grenada (90) and Sardis (98) Lakes in Mississippi are on that list. If the Yazoo Study Area was treated as a lake or reservoir, it would rank as the 23rd largest when the Steele Bayou landside gage is at elevation 87 feet (NGVD 29) (the pump-on elevation). In 2019, the Steele Bayou landside gage reached 98.2 feet (North American Vertical Datum [NAVD 88]), and the Yazoo Study Area would have jumped to 9th on the list of largest water bodies. The only lakes larger than the Yazoo Study Area lake, would be the five Great Lakes, Great Salt Lake (Utah), Lake-of the Woods (Minnesota and Canada), and Iliamna Lake (Alaska), which are all natural lakes. The Yazoo Study Area lake would be larger than all of the man-made reservoirs in the U.S. at that time. When the Yazoo Study Area is at 87 feet (NGVD 29) on the Steele Bayou landside gage, the area flooded is a great as the sum of the four Yazoo Basin flood control reservoirs when they are at their maximum capacity. This capacity was achieved 19 times in the 23 years that have elapsed since 1997. As another indication of the scale of flooding in the basin, the 2019 flood covered an area equal to two-thirds of the area of the State of Rhode Island.

DESCRIPTION OF YAZOO STUDY AREA

26. The Mississippi River Mainline Levees are designed to protect the alluvial valley from extreme flood events by confining flow to the leveed floodway, except where it enters the natural backwater areas or is diverted intentionally into floodway areas. When major floods occur and the carrying capacity of the Mississippi River leveed channel is threatened, additional conveyance through the Birds Point-New Madrid Floodway and relief outlets through the Atchafalaya Basin Floodway, Morganza Floodway, and Bonnet Carre Floodways are utilized as well as the storage capacity of flat lowlands at the junctions of tributaries with the Mississippi River. These tributary areas are commonly referred to as backwater areas. The Yazoo River tributary area is commonly known as the Yazoo Backwater Area, or the Yazoo Study Area. The Yazoo Backwater levees were built to protect a major portion of the Mississippi Delta from major Mississippi River floods and are primarily designed to overtop prior to the MR&T Project

Design Flood (PDF) peak such that storage is made available in order to reduce the level of the PDF, thus resulting in a lesser levee grade along the mainline levees.

DRAINAGE AREAS

27. The Yazoo Study Area has a drainage area comprised of the Little Sunflower River, Big Sunflower River, Deer Creek, and Steele Bayou Basins as shown in Figure 2-6. These streams have a total drainage area of 4,093 square miles of the alluvial valley of the Mississippi River commonly called the Mississippi Delta. The area extends from the confluence of Steele Bayou with the Yazoo River north to the vicinity of Clarksdale, Mississippi, and has an average width of approximately 30 miles. The Mississippi Delta alluvial plain is generally flat with slopes averaging 0.3 to 0.9 feet per mile. Drainage areas of the four basins can be seen in Table 2-1.

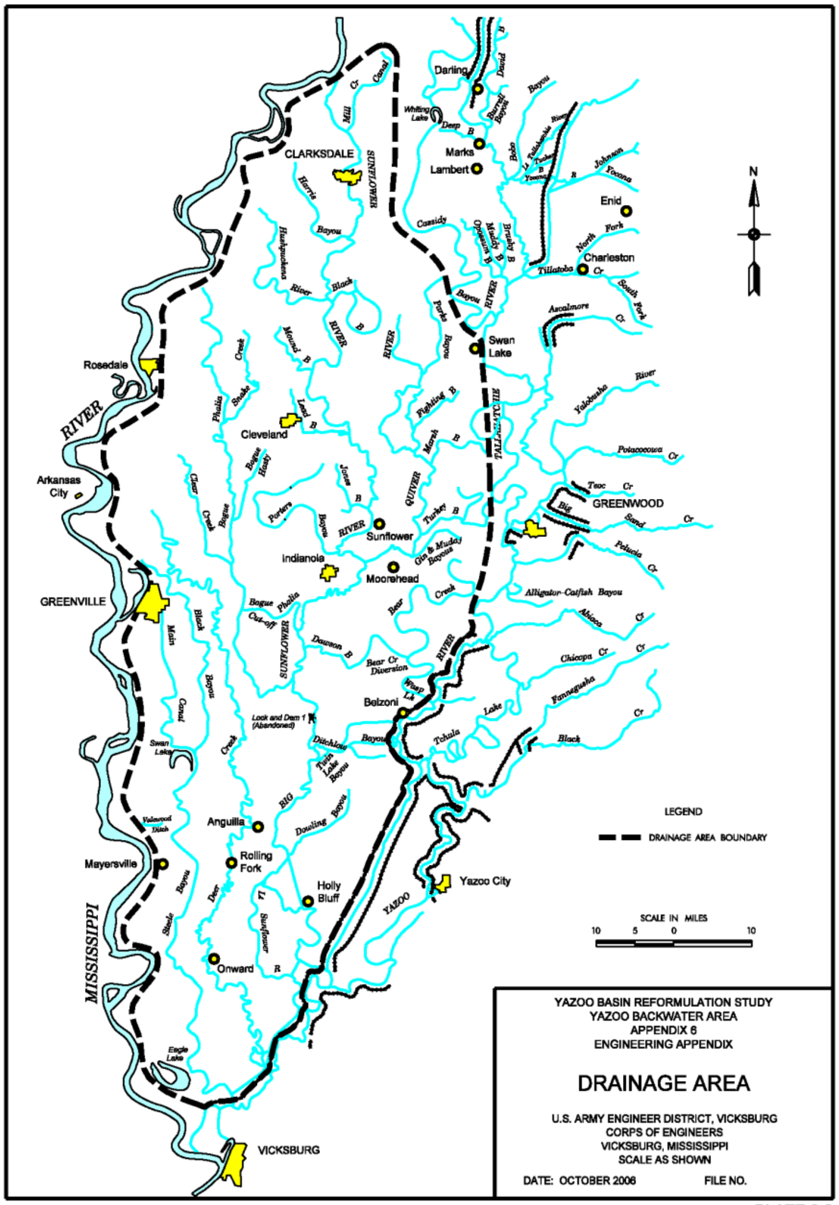

Figure 2-6. The drainage areas within the Yazoo River Basin.

Table 2-1. Yazoo Area Drainage Basin Area

Stream	**Drainage Area (sq mi)**
Big Sunflower River	2,832
Little Sunflower River	309
Deer Creek	200
Steele Bayou	752
Total	4,093

CLIMATE

28. The climate of the Yazoo Study Area is primarily humid, subtropical with abundant precipitation. The summers are long and hot; the winters are short and mild. According to the 2017 Climatological Data Annual Summary for Mississippi, the average annual temperature for the Lower Mississippi Delta was about 66.5 degrees Fahrenheit in 2017. Additionally, during 2017, the average monthly temperatures for the Lower Mississippi Delta ranged from 46.9 degrees Fahrenheit in December to 82.2 degrees Fahrenheit in July (NCEI 2017). During 2018, the Lower Mississippi Delta experienced an average annual temperature of 64.3 degrees Fahrenheit, with average monthly temperatures ranging from 39.1 degrees Fahrenheit in January to 81.7 degrees Fahrenheit in July (NCEI 2018). The average annual temperature for the Lower Mississippi Delta during 2019 was 64.9 degrees Fahrenheit. Monthly average temperatures during 2019 range from 45.0 degrees Fahrenheit in January to 83.0 degrees Fahrenheit in September (NCEI 2019). Temperature extremes ranged from about 10 degrees Fahrenheit to 100 degrees Fahrenheit for 2017 and 2018 (NCEI 2017, NCEI 2018). Temperature extremes during 2019 ranged from 20 degrees Fahrenheit to 100 degrees Fahrenheit (NCEI 2019).

PRECIPITATION

29. According to the 2017 Climatological Data Annual Summary for Mississippi, the annual rainfall over the Lower Mississippi Delta was approximately 53.9 inches. During 2017, normal monthly rainfall for the Lower Mississippi Delta varied from 6.4 inches in April to 1.5 inches in October (NCEI 2017). In 2018, the Lower Mississippi Delta had an annual rainfall of 68.2 inches, with a normal monthly rainfall ranging from 2.3 inches in October to 13.3 inches in February (NCEI 2018). In 2019, the Lower Mississippi Delta had an annual rainfall of 77.9 inches, with a normal monthly rainfall ranging from 0.7 inches in September to 13.9 inches in February (NCEI 2019). The Lower Mississippi Delta generally receives more rainfall during winter and spring months than summer or fall months due to the intrusion and retreat of polar air across the region that creates frontal boundaries and widespread and persistent rainfall. Snowfall occurs about once a year with an average of approximately two inches.

CLIMATE CHANGE

30. According to the Fourth National Climate Assessment, the southeastern United States has experienced an uneven trend in observed warming since the mid-20th century (Carter et al. 2018). Similarly, Mississippi has not experienced an overall warming trend since 1900 and instead has only experienced a near or slightly above average near-surface air temperature since the 1990s (Runkle et al. 2017). The observed and projected temperature change for Mississippi from 1900 through 2100 is shown in Figure 2-7. Unlike maximum daily temperatures, the average daily minimum temperature has increased for the southeastern United States (Carter et al. 2018). Additionally, Mississippi has experienced an above average number of warm nights, with a minimum temperature of at least 75 degrees Fahrenheit, for the last nine years (Runkle et al. 2017). Figure 2-8 shows the number of warm nights per year from 1900 through 2016 and the percent change in warm nights from 1950 through 2016 for the southeastern United States. From Figure 2-8, it is evident the southeast has experienced more frequent warm nights, and the majority of Mississippi has experienced a positive percent change in warm nights. Furthermore,

climate model simulations for future conditions project increases in temperatures for lower and higher scenarios (Carter et al. 2018).

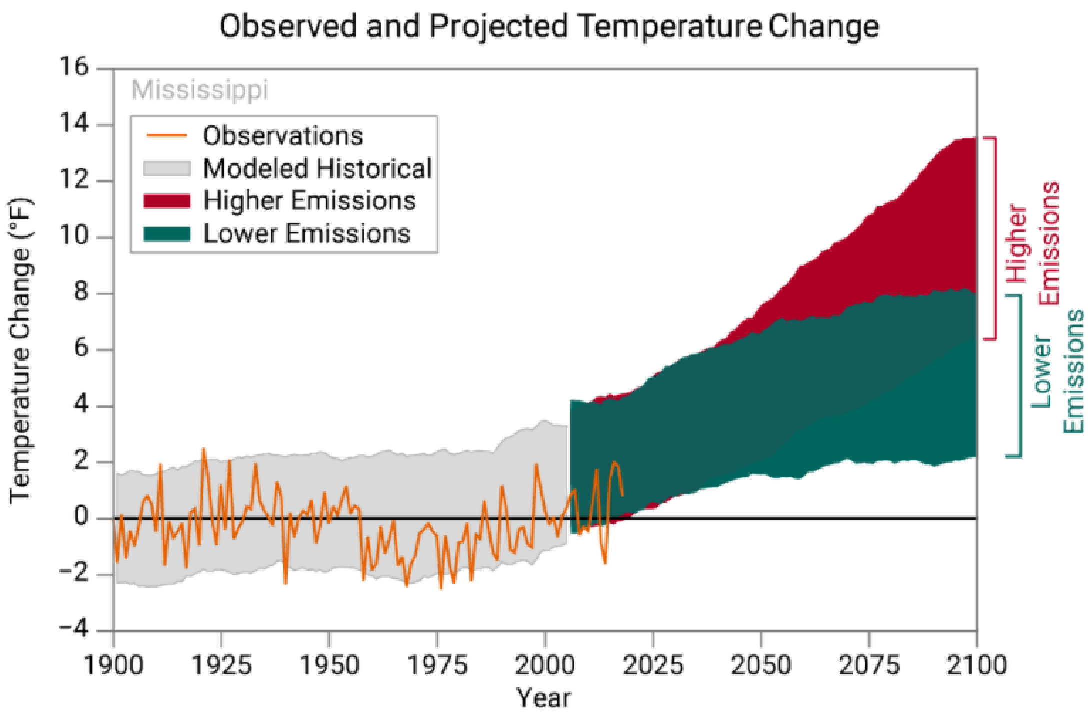

Figure 2-7. The observed and projected temperature change for Mississippi from 1990 through 2100 under both high and low emission climate projections. This figure was obtained from Runkle et al. 2017.

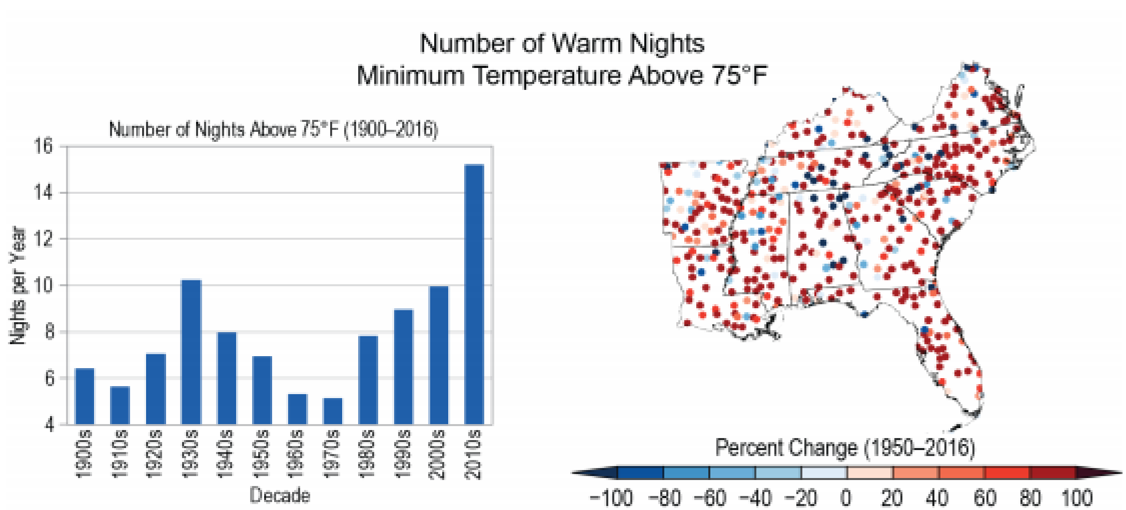

Figure 2-8. The number of warm nights above 75 degrees Fahrenheit and the percent change in the number of warm nights for the Southeastern United States. This figure was obtained from Carter et al. 2018.

31. In addition to increasing average daily minimum temperatures, the annual precipitation in Mississippi has been above average since the 1970s (Runkle et al. 2017). More specifically,

Mississippi's Climate Division 4, which encompasses the Lower Mississippi Delta, has experienced a positive trend for annual precipitation equal to 0.61 inches per decade from 1895 through 2019 (Figure 2-9). As another indicator in the change in annual precipitation, prior to 1955 there were only four years where the sum annual precipitation exceeded 65 inches, since 1955 there have been 14 years where the sum annual precipitation exceeded 65 inches. Additionally, the number of days with extreme precipitation events, that produce above three inches of precipitation, has been increasing for the southeastern United States, with the State of Mississippi and the Lower Mississippi Delta experiencing a positive percent change in extreme precipitation events since 1950 (Figure 2-10). Currently, climate projects indicate the number of extreme rainfall events will become more frequent and intense in the future (Runkle et al. 2017, Carter et al. 2018, and Easterling et al. 2017). In addition, the northern United States, is projected to receive more precipitation in the winter and spring months (Figure 2-11). Climate projections do not indicate the southeastern United States having as a dramatic increase in winter and spring precipitation when compared to the northern United States. However, the above normal precipitation projected for the northern United States, during the Lower Mississippi River Basin's wet season, will increase the potential for flooding along the Mississippi River and consequently within the Mississippi Delta.

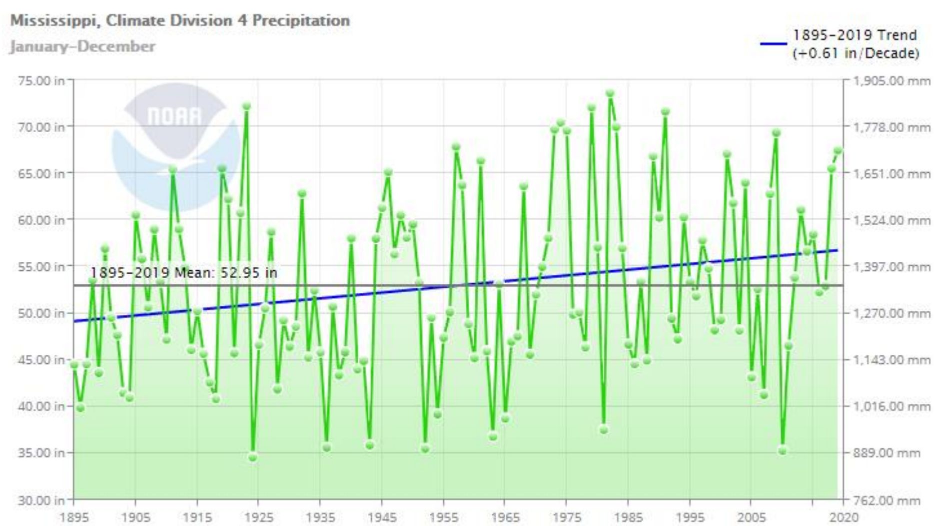

Figure 2-9. The annual precipitation for Mississippi's Climate Division 4 from 1895 through 2019 (NCEI 2020).

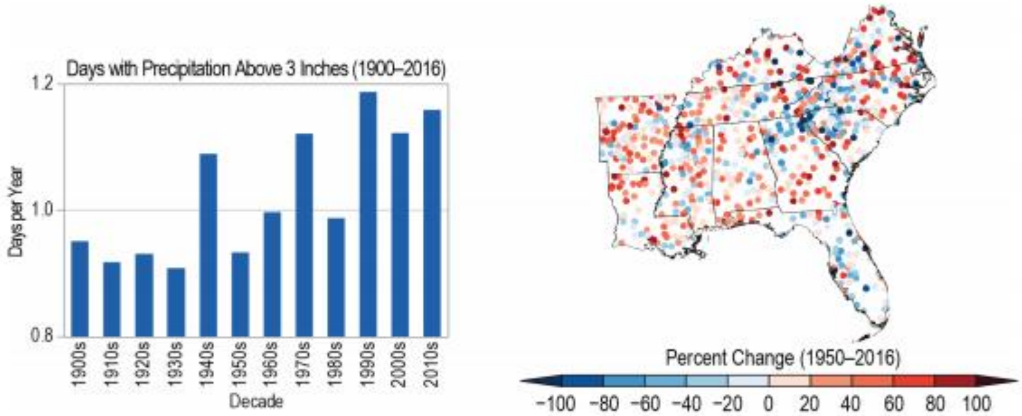

Figure 2-10. The number of days with heavy precipitation events and the percent change in heavy precipitation events for the Southeastern United States. This figure was obtained from Carter et al. 2018.

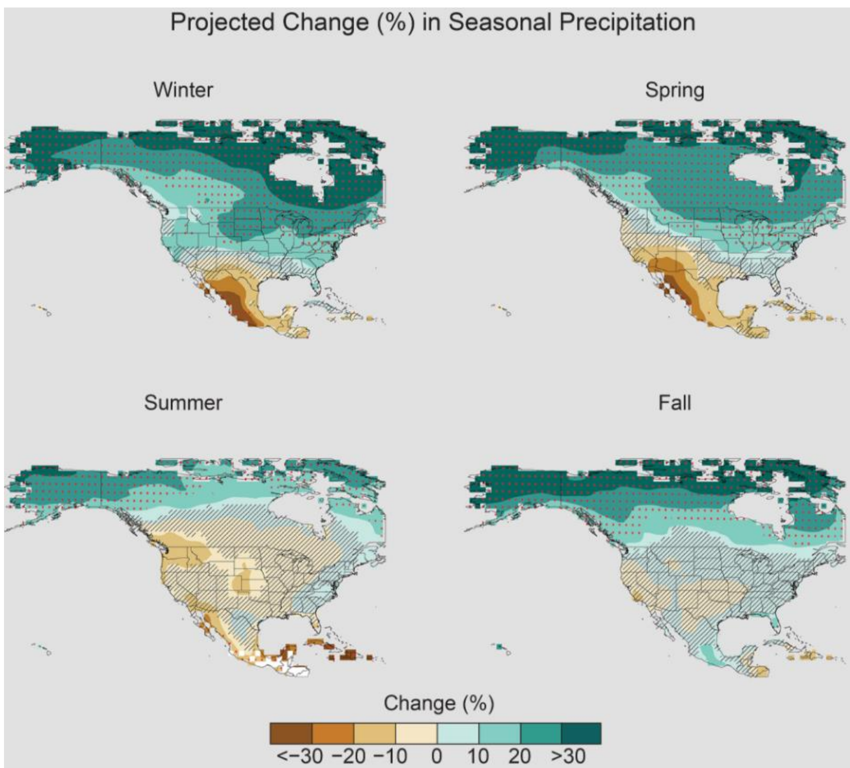

Figure 2-11. The projected change in total seasonal precipitation from CMIP5 simulations for 2070 through 2099. The projected changes are weighted multimodel means and are expressed as the percent change relative to the 1976-2005 average. Stippling indicates changes are determined to be large compared to natural variations. Hatching indicates changes are determined to be small compared to natural variations. This figure was obtained from Easterling et al. 2017.

32. As climate projections indicate, the southeastern United States will experience warmer temperatures, more frequent heavy precipitation events, and increased susceptibility to flooding during winter and spring months. Thus, it is vital regions, such as the Mississippi Delta, are

proactive and implement effective water management and flood control measures to prevent the destruction of homes, businesses, and diverse ecosystems within the region.

33. For civil works projects, it is important to comply with Engineering and Construction Bulletin (ECB) 2018-14 to determine if climate change impacts inland hydrology for such projects. ECB 2018-14 requires an initial scoping that identifies relevant climate factors and assesses the need for quantitative hydrology and sea level change assessments. While stages on the Mississippi River have been experiencing an increasing trend over time, it is unclear how much is attributed to climate change indicators, such as ice melt or increased precipitation. Furthermore, the Yazoo Basin has a minimum ground surface elevation above 75 feet, and sea level rise will not likely impact the basin. Thus, a quantitative climate change assessment was not performed for this study.

INFILTRATION AND RUNOFF

34. When precipitation falls, some is stored as infiltration and some leaves as runoff. The runoff coefficient is the percentage of precipitation that leaves. Runoff coefficients vary from 10 percent in the summer months to 70 percent in the spring and winter months, depending on antecedent conditions, rainfall distribution, and rainfall intensity. Observed data on the Big Sunflower River at Sunflower, Mississippi, show that annual runoffs vary from about six to 41 inches and average about 24.5 inches over the drainage area. The runoff coefficients are average values that reflect conditions in the basin. Seasonal variations in runoff coefficients are shown by the monthly-generalized values in Table 2-2.

Table 2-2. Average Monthly Percent Runoff

Month	**Runoff Coefficients (%)**
January	60
February	60
March	70
April	70
May	60
June	40
July	25
August	10
September	10
October	25
November	25
December	60

FLOODING SINCE 1979

35. The Yazoo Basin experiences headwater floods, backwater floods, or both simultaneously. Generally, whenever the basin receives more than 0.5 inches of precipitation, there will be some run-off. This run-off will cause the basin's rivers to rise. When they rise enough, water will start to fill off-channel storage areas. At this point, the event is classified as a flood. For most gages, this flooding initiates for events greater than the 1.25-year frequency event, but flooding may not begin until the 5-year event is achieved. These events are called headwater floods.

Another aspect of headwater floods is that there is typically more than one foot of slope between gages. There are six gages that were in operation for the entire 42 years of the POR, and another six with partial records. Of the six with partial records, only two are within the 100-year floodplain. Backwater floods occur when a downstream river experiences higher stages than the tributary. When this occurs, the water surface on the tributary rises to the elevation of the downstream river. Backwater floods can affect large areas and extend many miles upstream. During the 2011 Mississippi River flood, the Yazoo River backed up all the way to Belzoni, which is a distance of 116 river miles upstream of the confluence of the Yazoo River with the Mississippi River in Vicksburg. A true backwater flood will have a flat or nearly flat surface. A backwater flood in the Yazoo Study Area is defined by two conditions. First, the water surface at the Steele Bayou landside gage is above 80 feet (NGVD 29), and second, the water surface elevation for the Steele Bayou riverside gage is higher than the landside gage. This means the structures gates are closed. At 80 feet (NGVD 29) on the Steele Bayou landside gage, off-channel storage areas start to fill. The backwater flood persists until the gates are open and the water surface has returned to 80 feet (NGVD 29). A backwater flood is seldom caused by a single precipitation event. During the course of a backwater flood there is generally several precipitation events, some or all may induce some headwater flooding. All these events contribute to the total volume of water stored within the backwater area. Figure 2-12 provides the hydrographs from several gages for the first few months of 1994, and it identifies several headwater flood events and a backwater event. The gages at Holly Bluff, Anguilla, and Little Callao reside on the Big Sunflower River. The many precipitation events that cause headwater flooding will not be affected by the pump station. These flood pulses will continue to occur after the project is completed.

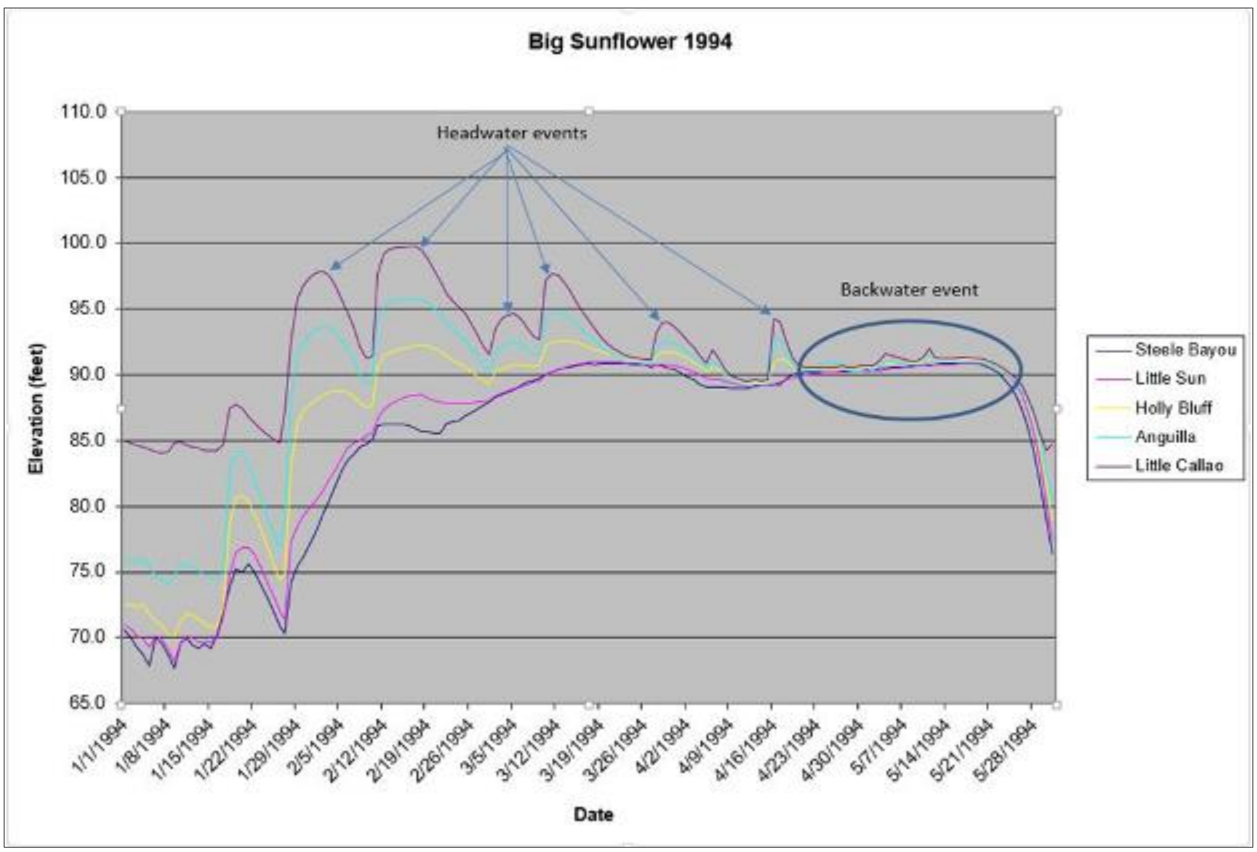

Figure 2-12. 1994 hydrograph for several Yazoo Study Area gages.

36. As previously stated, the Yazoo Basin experiences a backwater-driven flood when the riverside of the Steele Bayou flood control structure exceeds the landside and when the landside is above 80.0 feet (NGVD 29). When these conditions are met, the Steele Bayou flood control structure gates are closed, and the Yazoo Backwater begins to experience flooding since flood waters are unable to drain from the region. The following paragraphs describe backwater-driven flood events from 1978 through 2019 and provides graphics that illustrate when these backwater conditions are met. The new period-of-record encompasses 1978 through 2019.

MAJOR BACKWATER FLOOD EVENTS

FLOOD OF 1979

37. The flood of 1979 occurred after the Yazoo Backwater levee was completed and began as the Mississippi River started to rise early in 1979. By 01 March, due to a combination of rainfall in the Yazoo Study Area and high Mississippi River stages, Steele Bayou began to rise above elevation 80 feet (NGVD 29). On 04 March, as water reached an elevation of 82.5 feet (NGVD 29) in the Yazoo Study Area, the Steele Bayou gates were closed to prevent the Mississippi and Yazoo Rivers from flowing into the Yazoo Study Area. The Little Sunflower River structure was closed on 05 March as water reached 85.05 feet (NGVD 29). Water in the Yazoo Study Area continued to rise throughout March. However, from 08 April through 14 April, the Steele Bayou gates were momentarily opened as the Mississippi River at Vicksburg briefly fell from 90.0 feet (NGVD 29) on 24 March to 88.3 feet (NGVD 29) on 03 April and Steele Bayou riverside fell below Steele Bayou landside.

38. After this brief recession of water, both the river and landsides of the Backwater levees began to experience an increase in water elevations, resulting in the closure of the Steele Bayou gates on 14 April. Steele Bayou riverside and Little Sunflower riverside then reached peak elevations of 97.2 and 97.6 feet (NGVD 29) on 28 April. Despite the large amount of rainfall in the Yazoo Study Area, Little Sunflower landside did not reach its peak of 96.6 feet (NGVD 29) until 05 May. The Mississippi and Yazoo Rivers, which had begun their fall several days before, fell low enough for the floodgates to be opened at Steele Bayou on 04 May at elevation 96.3 feet (NGVD 29) and Little Sunflower River on 05 May at elevation 96.6 feet (NGVD 29). The peak elevations in the Yazoo Study Area, during this backwater-driven flood event, were the annual peak elevations during 1979. This decline continued until water fell below elevation 80.0 feet (NGVD 29) in the Steele Bayou area on 14 June and the Little Sunflower area on 15 June 1979 ending a flood which lasted 104 days and flooded a maximum of 350,400 acres.

39. Without the Yazoo Backwater levees and structures, approximately 400,000 acres would have been flooded. Many homes in the Eagle Lake area were threatened with major flooding as water levels were within inches of the natural ridge protecting the area adjacent to the Muddy Bayou structure. Emergency efforts to raise the ridge by USACE were successful during this event; however, lake water levels were raised to elevation 90.0 feet (NGVD 29), with flow through the Muddy Bayou structure, in preparations to lessen catastrophic damage, which would have occurred had Steele Bayou stages risen another inch or two. Because the Yazoo Backwater

exceeded an elevation of 87.0 feet (NGVD 29), the proposed pumps would have been turned on to alleviate the high water within the Yazoo Study Area.

40. In Figure 2-13, the top graph illustrates the Yazoo Backwater elevations for the gages at Steele Bayou landside, Little Sunflower landside, Holly Bluff (Big Sunflower River), Anguilla (Big Sunflower River), and Little Callao (Big Sunflower River) during the 1979 Yazoo Backwater flood. The bottom graph depicts the difference in elevation between Steele Bayou landside and riverside during the 1979 Yazoo Backwater flood. When Steele Bayou landside is lower than Steele Bayou riverside, i.e., the difference in elevation is negative, and Steele Bayou landside is above 80.0 feet (NGVD 29), the gates of the Steele Bayou water control structure are closed. The closure of the Steele Bayou gates keeps high water from draining from the Yazoo Study Area. The Yazoo Backwater elevations and Steele Bayou landside and riverside elevation difference graphics are provided for each following historical Yazoo Backwater flood event.

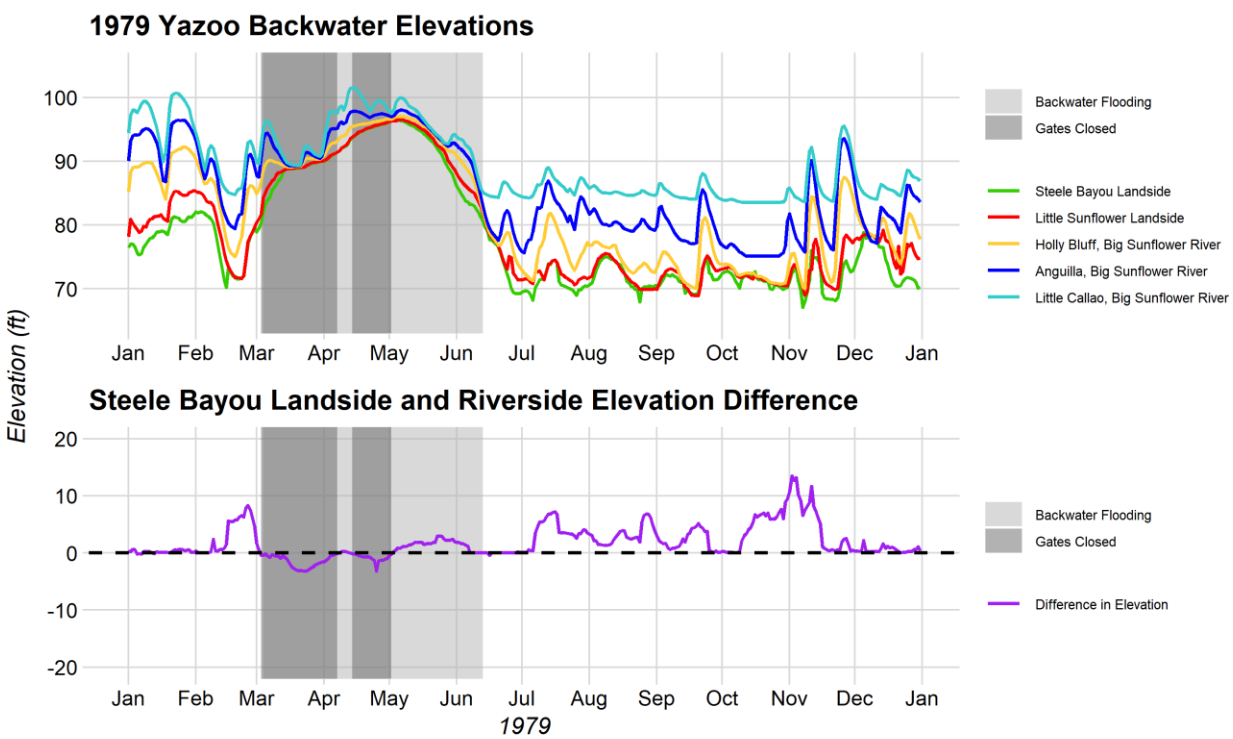

Figure 2-13. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 1979 Yazoo Backwater flood.

FLOOD OF 1983

41. Headwater flooding in the Yazoo Study Area began in December 1982 and peaked at 92.0 feet (NGVD 29) on 11 January 1983 before falling below an elevation of 80.0 feet (NGVD 29) on 19 February 1983 (Figure 2-14). During March, the Yazoo Study Area experienced another headwater flood, but during April, stages on the Mississippi River began to increase after three storms, occurring from late April and throughout May, produced rainfall totals up to 16 inches in the lower Ohio and Mississippi River Basins. The excessive rainfall resulted in the Mississippi River beginning to experience dramatic increases in elevation during April and

resulted in the closure of the Steele Bayou gates on 19 April. On 27 May, the Mississippi River at Vicksburg peaked at 95.5 feet (NGVD 29). On 28 May, the Steele Bayou riverside peaked at 98.5 feet (NGVD 29) and on 09 June, the Steele Bayou landside peaked at 95.8 feet (NGVD 29). After the Yazoo Study Area crested, the gates at Steele Bayou were opened on 11 June, and the Yazoo Study Area flood waters receded below an elevation of 80 feet (NGVD 29) on 30 June 1983. Overall, the Yazoo Study Area experienced backwater-induced flooding for 73 days from 19 April until 30 June during 1983. Because the Yazoo Study Area exceeded 87.0 feet (NGVD 29) the proposed pumps would have been turned on during this flood event.

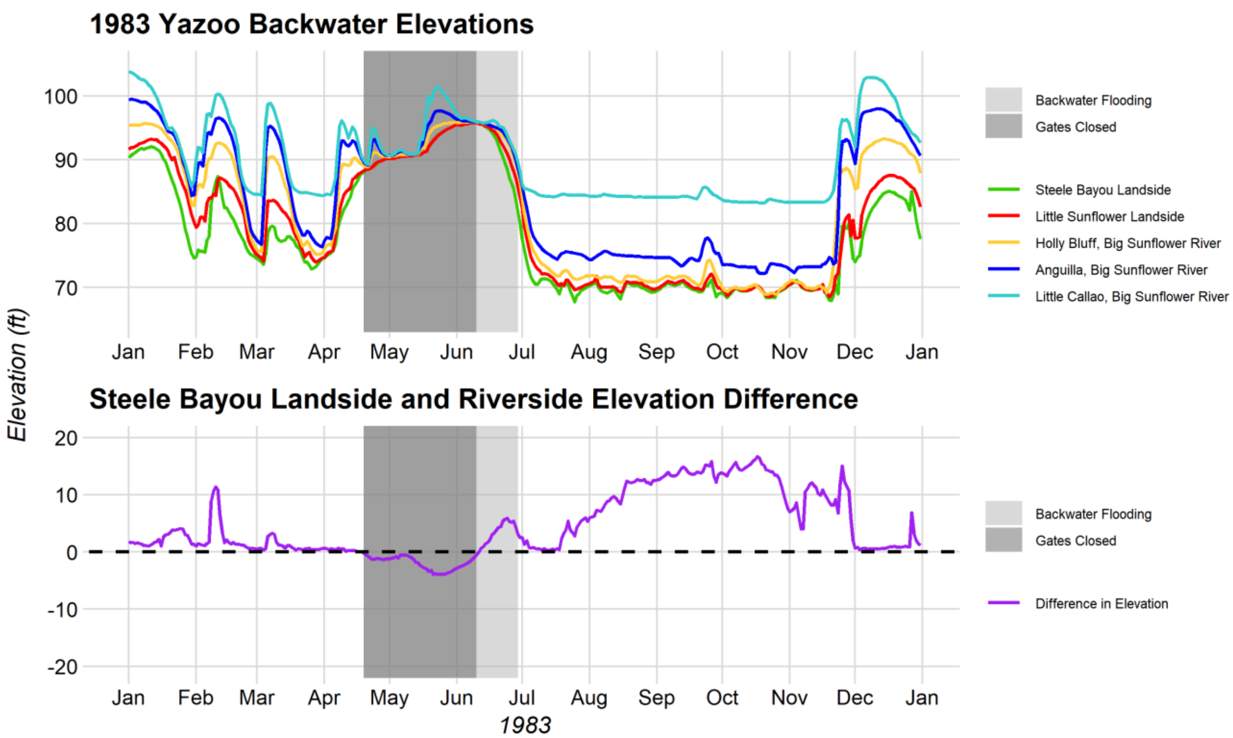

Figure 2-14. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 1983 Yazoo Backwater flood.

FLOOD OF 1984

42. The 1984 Yazoo Study Area flood began on 27 March when the gates at Steele Bayou were forced to close due to a rising Mississippi River and Steele Bayou riverside (Figure 2-15). As the Mississippi River at Vicksburg began to experience increasing stages, water backed up into the Yazoo Study Area. The Mississippi River at Vicksburg peaked on 26 May at 92.0 feet (NGVD 29). Then the Steele Bayou riverside crested at 94.5 feet (NGVD 29) on 27 May, and the Steele Bayou landside crested at 92.0 feet (NGVD 29) on 29 May. The flood receded below an elevation of 80.0 feet (NGVD 29) on 15 June. The Yazoo Study Area experienced backwater-induced flooding for 81 days from 27 March to 15 June during 1984. Additionally, because the Yazoo Study Area exceeded 87.0 feet (NGVD 29) the proposed pumps would have been turned on during this flood event.

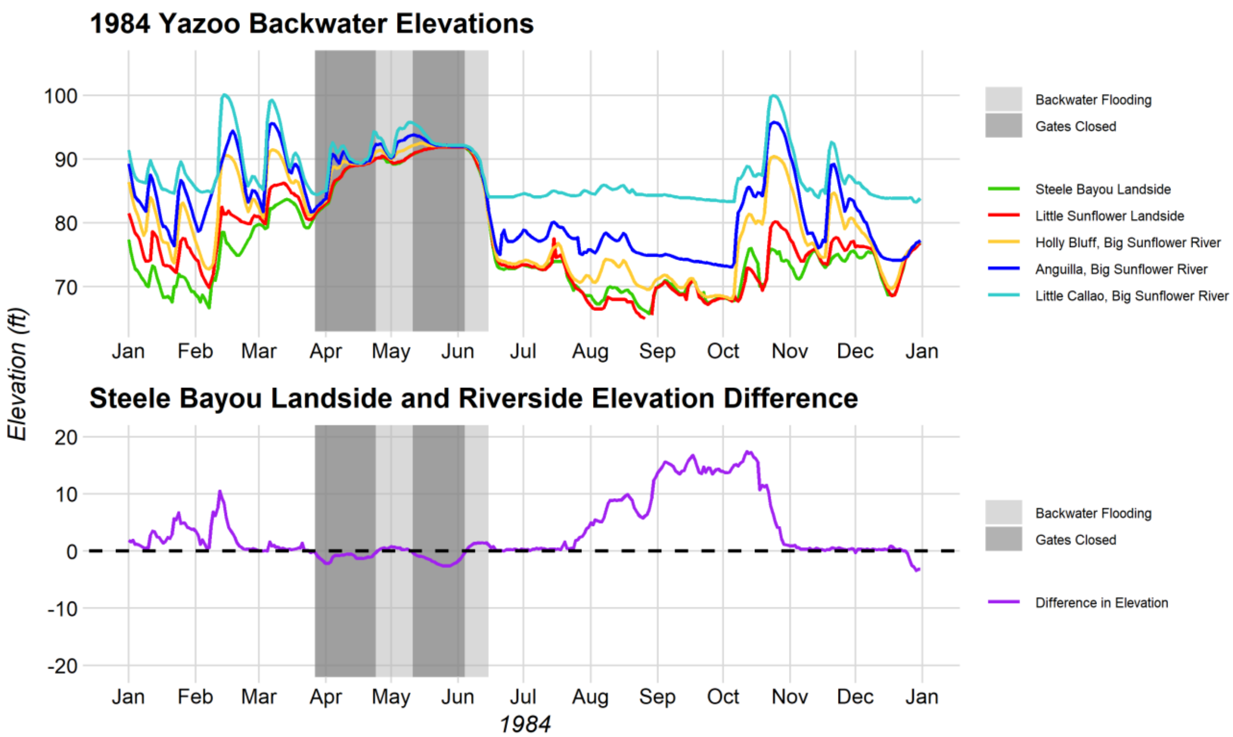

Figure 2-15. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 1984 Yazoo Backwater flood.

FLOOD OF 1991

43. During January of 1991, the Yazoo Study Area experienced backwater-induced flooding that resulted in the closure of the Steele Bayou gates (Figure 2-16). The Mississippi River at Vicksburg began to rise on 20 December 1990 and crested at 90.6 feet (NGVD 29) on 20 January 1991. Due to the increasing stages on the Mississippi River, the Steele Bayou riverside began to increase and surpassed the landside elevation, resulting in the closure of the Steele Bayou gates on 07 January and remained closed until 27 January. The Steele Bayou riverside peaked at 91.7 feet (NGVD 29) on 28 January and the Steele Bayou landside crested at 93.1 on 22 January. Because the Steele Bayou landside surpassed an elevation of 87.0 feet (NGVD 29) during this backwater-induced flood event, the proposed pumps would have been turned on.

44. From April through June, the Yazoo Study Area was flooded by a headwater flood due to tremendous amounts of rainfall in the Upper Yazoo Area (Figure 2-16). The flooding in the Yazoo Area peaked at elevation 92.4 feet (NGVD 29) on 06 May. Because this flood event was a headwater flood, the Steele Bayou riverside elevation reached a peak of 90.8 feet (NGVD 29) on 04 May, roughly 1.5 feet lower than the landside elevation. The Steele Bayou and Little Sunflower River structure gates only briefly closed at the beginning of this flood event as the Steele Bayou riverside momentarily exceeded the Steele Bayou landside.

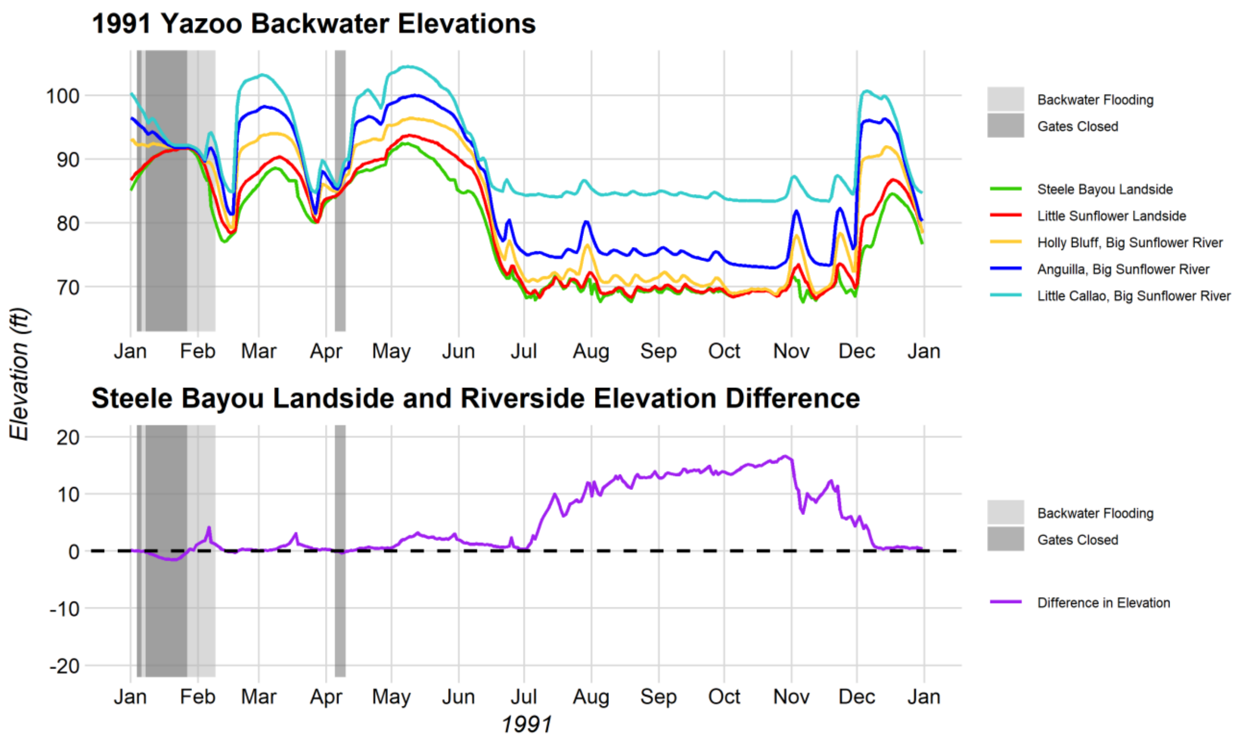

Figure 2-16. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 1991 Yazoo Backwater flood.

FLOOD OF 1993

45. The flood of 1993 primarily affected the Upper Mississippi River and its tributaries. High antecedent soil moisture followed by persistent, heavy rainfall from April through September produced extensive flooding in the Upper Mississippi Basin. The effect on the Lower Mississippi River was passed without major flooding. The flood of 1993 demonstrated that during high Upper Mississippi River discharges, flooding on the Upper Mississippi River alone would not produce a major flood event on the Lower Mississippi River. However, the Yazoo Study Area still experienced backwater-induced flooding as a result of the major flooding in the Upper Mississippi Basin. On 10 March, the gates at Steele Bayou were closed as the Steele Bayou riverside exceeded the Steele Bayou landside (Figure 2-17). The gates at Steele Bayou were briefly able to open from 30 March to 07 April before closing again. The Mississippi River at Vicksburg peaked at 89.9 feet (NGVD 29) on 18 May. Then, both the Steele Bayou landside and riverside reached an elevation of 91.5 feet (NGVD 29) on 19 May. The flood receded below elevation of 80 feet (NGVD 29) on 07 June. The Mississippi River at Vicksburg rose again on 16 July, due to the Upper Mississippi River flooding, and reached an elevation of 85.2 feet (NGVD 29) on 12 August. The Steele Bayou gates were closed from 23 July to 10 August, and the Steele Bayou riverside and landside gages both crested at 86.5 feet (NGVD 29) on 12 August. The flood receded below 80.0 feet (NGVD 29) on 02 September (Figure 2-17). Overall, the Yazoo Study Area was flooded for 130 days in 1993. The proposed pumps would have been turned on during the flood event in May since high water elevations in the Yazoo Study Area exceeded 87.0 feet (NGVD 29). However, the proposed pumps would not have been turned on for the August flood event since water elevations did not exceed 87.0 feet (NGVD 29).

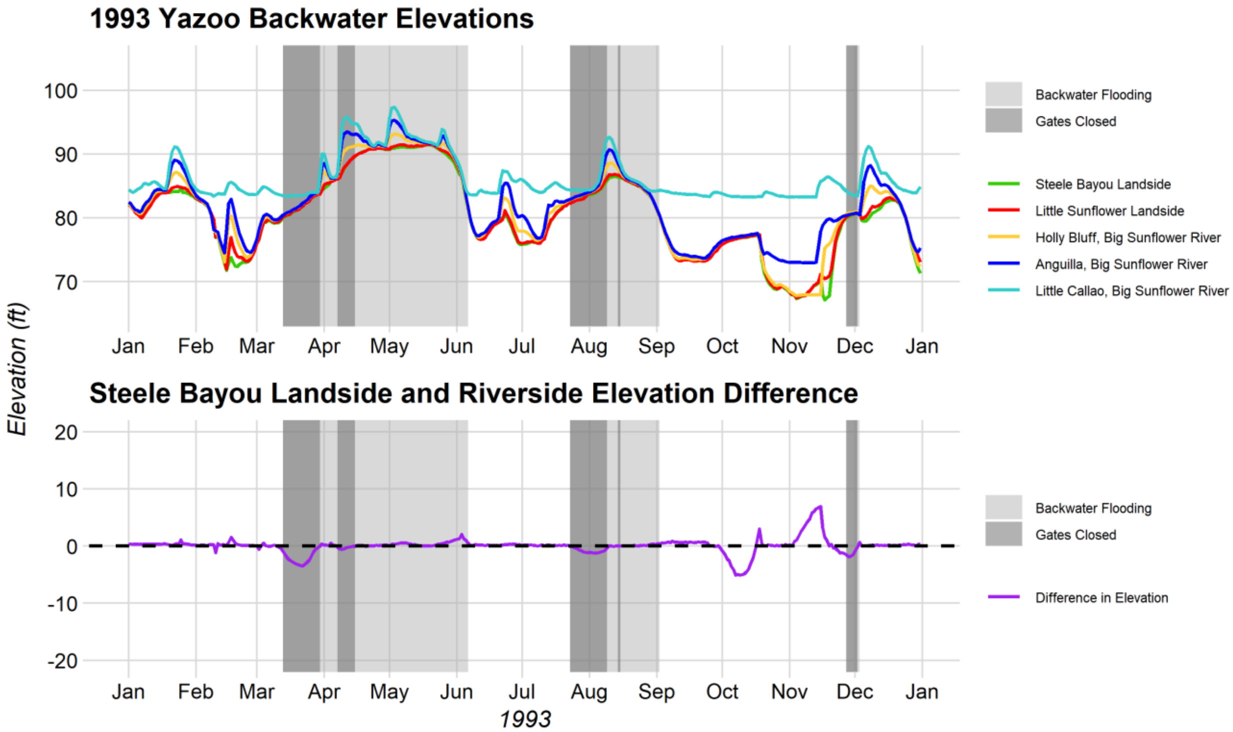

Figure 2-17. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 1993 Yazoo Backwater flood.

FLOOD OF 1997

46. The flood of 1997 began with the Mississippi River reaching the highest flood levels experienced at Arkansas City, Arkansas, and Natchez, Mississippi, since 1973 and the highest at Greenville and Vicksburg, Mississippi, since 1983.The 1997 Mississippi River flood was the fourth highest of record at Natchez and Cairo, following close behind 1927, 1937, and 1973.The Mississippi River at Vicksburg began to experience significant increases in stage in early March (Figure 2-18). On 09 March, the gates at Steele Bayou were closed as the riverside exceeded the landside elevation. The Mississippi River at Vicksburg crested at 95.3 feet (NGVD 29) on 23 March. The Steele Bayou riverside peaked at 98.2 feet (NGVD 29) on 24 March, and the Steele Bayou landside peaked at an elevation of 93.3 feet (NGVD 29) on 08 April. The Steele Bayou gates remained closed until 12 April. The Yazoo Study Area did not recede below 80.0 feet (NGVD 29) until 19 May. The Yazoo Study Area experienced another brief backwater-induced flood from 08 June through 08 July and peaked at 85.0 feet (NGVD 29) on 28 June. Because the Yazoo Study Area experienced high water above an elevation of 87.0 feet (NGVD 29) in March and April, the proposed pumps would have been turned on during this flood event. The proposed pumps would not have been turned on during the minor flood event in June as high water elevations did not exceed 87.0 feet (NGVD 29). Overall, the Yazoo Study Area was flooded for 101 days in 1997.

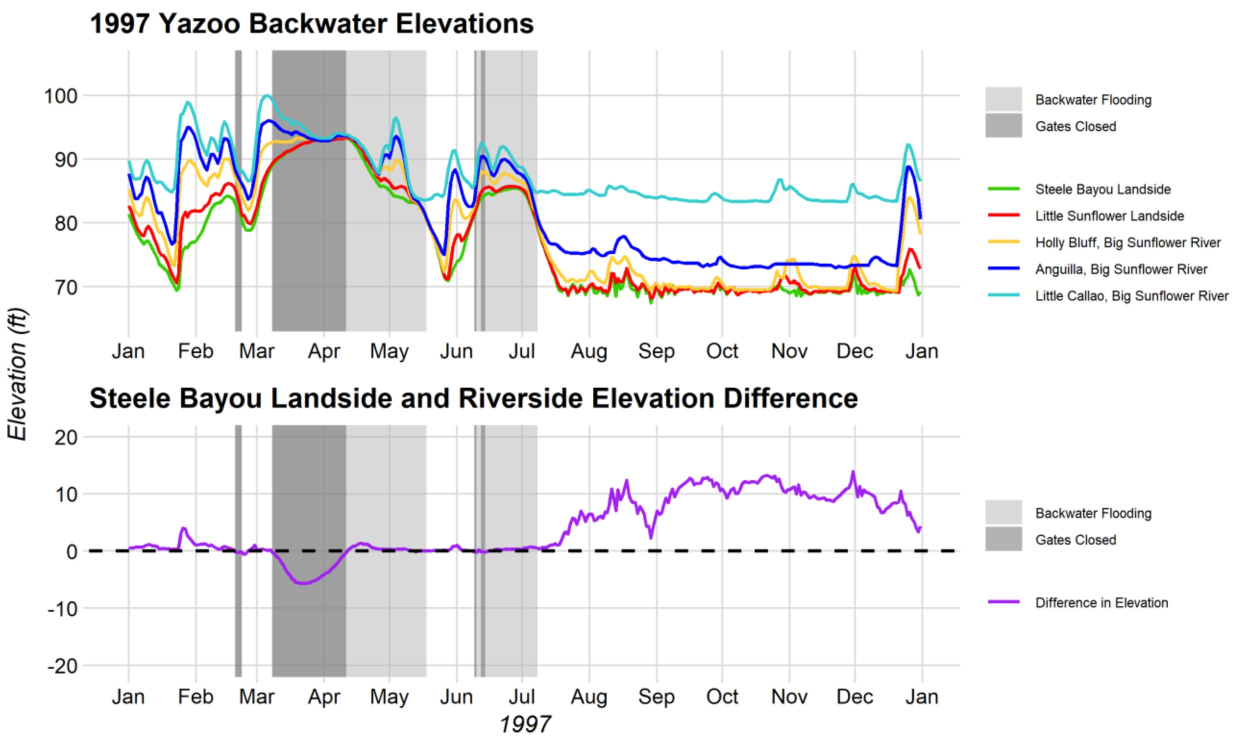

Figure 2-18. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 1997 Yazoo Backwater flood

FLOOD OF 1998

47. The 1998 Yazoo Backwater flood began on 29 March due to a rising Mississippi River. The increases in elevation on the Mississippi River resulted in Steele Bayou riverside exceeding the landside elevation, and the landside elevation surpassing 80.0 feet (NGVD 29). Consequently, the gates were closed on 29 March, which is depicted in Figure 2-19. The Steele Bayou gates were closed from 29 March through 11 April, 25 April through 25 May, and 29 June through 09 July. The second closure of the Steele Bayou gates corresponded to the peak of the 1998 flood event, when the Steele Bayou landside crested on 11 May at 88.3 feet (NGVD 29). Around this time, more upstream river gages within the Yazoo Backwater (Little Callao, Anguilla, and Holly Bluff on the Big Sunflower River) began to equalize with the downstream gages (Little Sunflower landside and Steele Bayou landside). The Mississippi River at Vicksburg and the riverside elevation of Steele Bayou peaked shortly after at 89.8 feet (NGVD 29) and 91.6 feet (NGVD 29), respectively, on 14 May. Although the Steele Bayou gates were opened after the peak of the Yazoo Backwater flood, from 26 May through 28 June, the elevation of the Yazoo Backwater remained above an elevation of 80.0 feet (NGVD 29), prolonging the backwater flood until 05 June when the elevation fell below 80.0 feet (NGVD 29). Similarly, the third gate closure from 29 June through 09 July, resulted in the Yazoo Backwater flooding again, with high water elevations remaining above 80.0 feet (NGVD 29) until 18 July. Overall, the Yazoo Backwater was flooded for 89 days from 29 March through 18 July during 1998, and the highest Yazoo Backwater elevation for 1998 was associated with the backwater flood. In addition, this backwater flood event would have required the proposed pumps to be turned on since the Yazoo Backwater elevation exceeded 87.0 feet (NGVD 29).

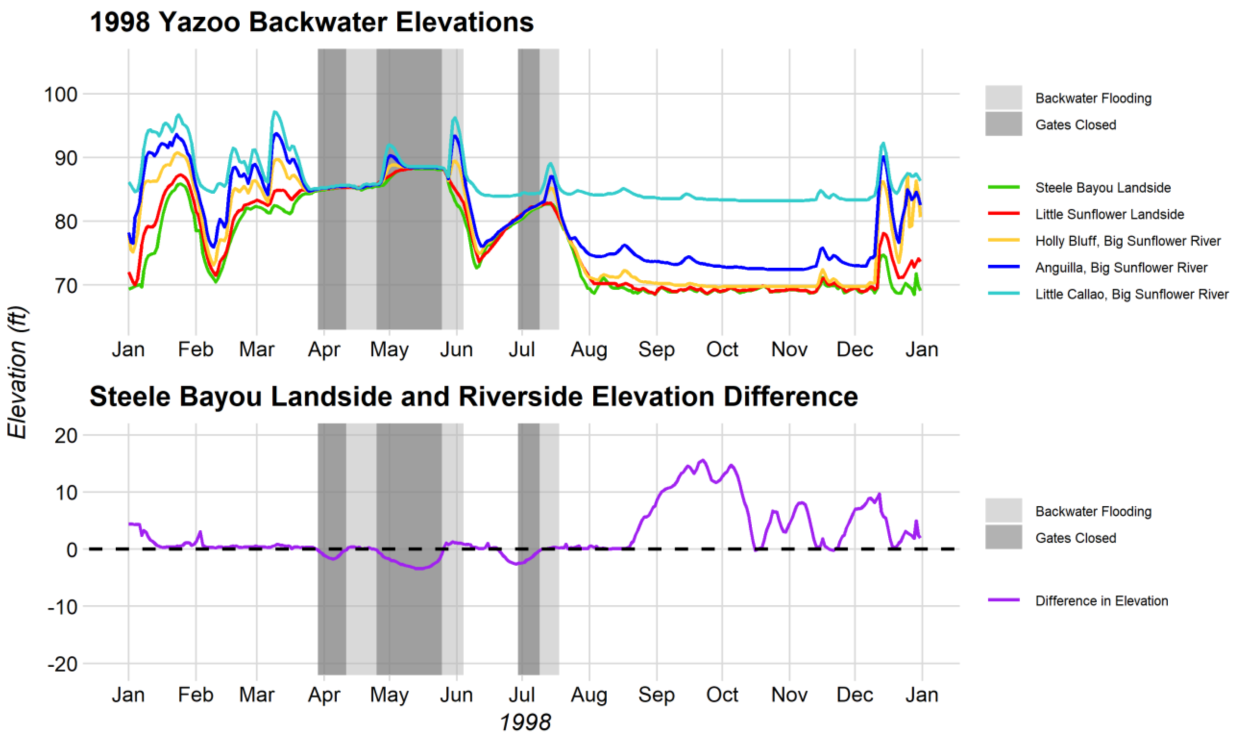

Figure 2-19. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 1998 Yazoo Backwater flood.

FLOOD OF 1999

48. During 1999, the Yazoo Study Area experienced numerous heavy rainfall events from January through April that resulted in the Yazoo Backwater having prolonged headwater-driven flooding. The highest elevation the Yazoo Backwater experienced during 1999 was 90.3 feet, which occurred on 15 February due to the headwater flooding. Then, on 02 May, the Yazoo Backwater began to experience backwater-driven flooding due to a rising Mississippi River. The Steele Bayou gates were closed from 02 May through 06 May (Figure 2-20). The Steele Bayou riverside elevation peaked at 85.0 feet (NGVD 29) on 13 May, and the Steele Bayou landside elevation peaked at 85.2 feet (NGVD 29) on 14 May. The Mississippi River at Vicksburg peaked at 83.5 feet (NGVD 29) on 19 May. Although the Steele Bayou gates were only closed from 02 May through 06 May, the Yazoo Backwater continued to experience backwater-driven flood conditions until 27 May when the Yazoo Backwater was able to recede below an elevation of 80.0 feet (NGVD 29). Overall, the Yazoo Backwater was flooded for 26 days during the 1999 backwater-driven flood event. Additionally, the backwater-driven flood event would be considered minor since the Yazoo Backwater elevation did not exceed 87.0 feet (NGVD 29), and the proposed pumps would not have been turned on.

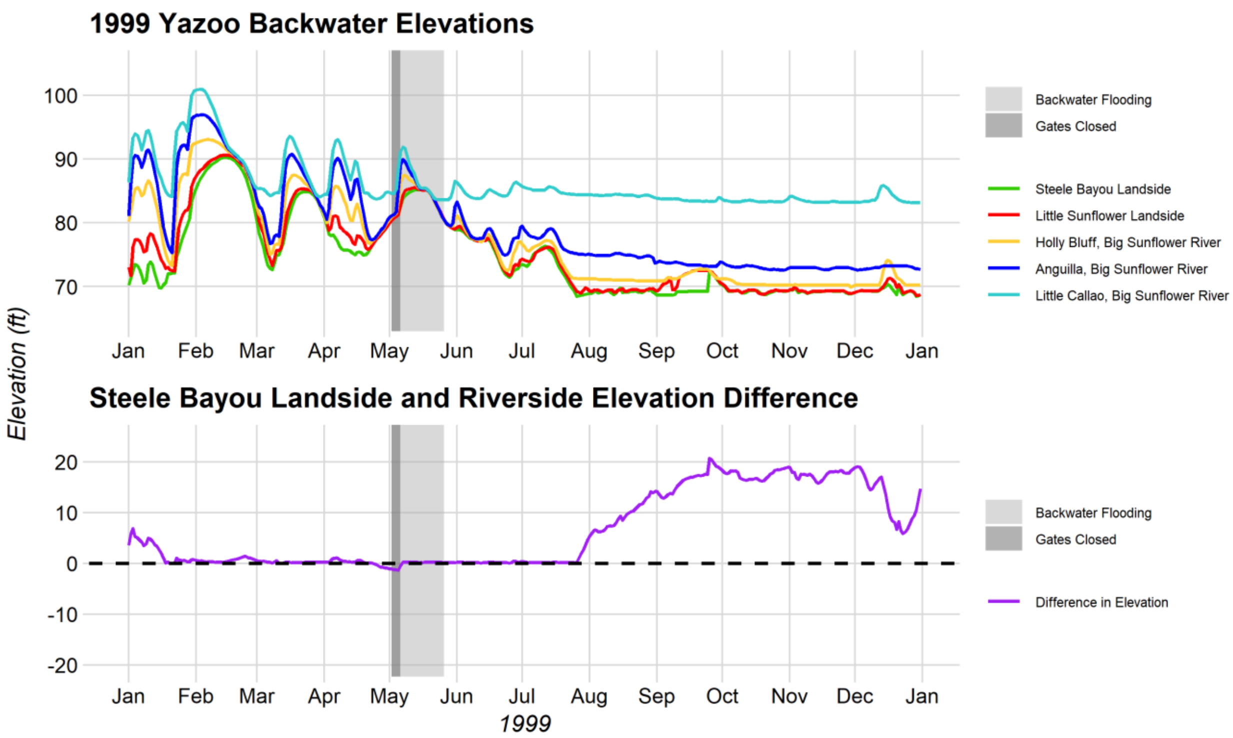

Figure 2-20. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 1999 Yazoo Backwater flood.

FLOOD OF 2002

49. During 2002, the Yazoo Backwater experienced numerous heavy rainfall events from January through April that resulted in headwater flooding in the Yazoo Basin. The Yazoo Backwater experienced an annual crest at 90.0 feet (NGVD 29) on 13 April during the headwater-driven flooding. Then, on 04 May, the Yazoo Backwater began to experience backwater-driven flooding due to a rising Mississippi River. Both the Mississippi River at Vicksburg and the Steele Bayou riverside peaked at 91.6 and 93.7 feet (NGVD 29), respectively, on 03 June. Thus, the Steele Bayou gates were closed briefly from 04 May through 06 May to mitigate backwater flow into the Yazoo Study Area (Figure 2-21). The gates were then opened from 07 May through 14 May, before closing from 15 May through 11 June. The Steele Bayou landsidecrested 12 June at 88.0 feet (NGVD 29), during the second closure of the Steele Bayou gates, and elevations in the Yazoo Backwater began to equalize. Although the Steele Bayou gates were reopened on 12 June, the flood waters within the Yazoo Backwater did not recede below 80.0 feet (NGVD 29) until 21 June. The Yazoo Backwater was flooded for a total of 49 days during the backwater-driven flood event of 2002. In addition, the backwater-driven flood event would have resulted in the proposed pumps being turned on since the Yazoo Backwater exceeded an elevation of 87.0 feet (NGVD 29).

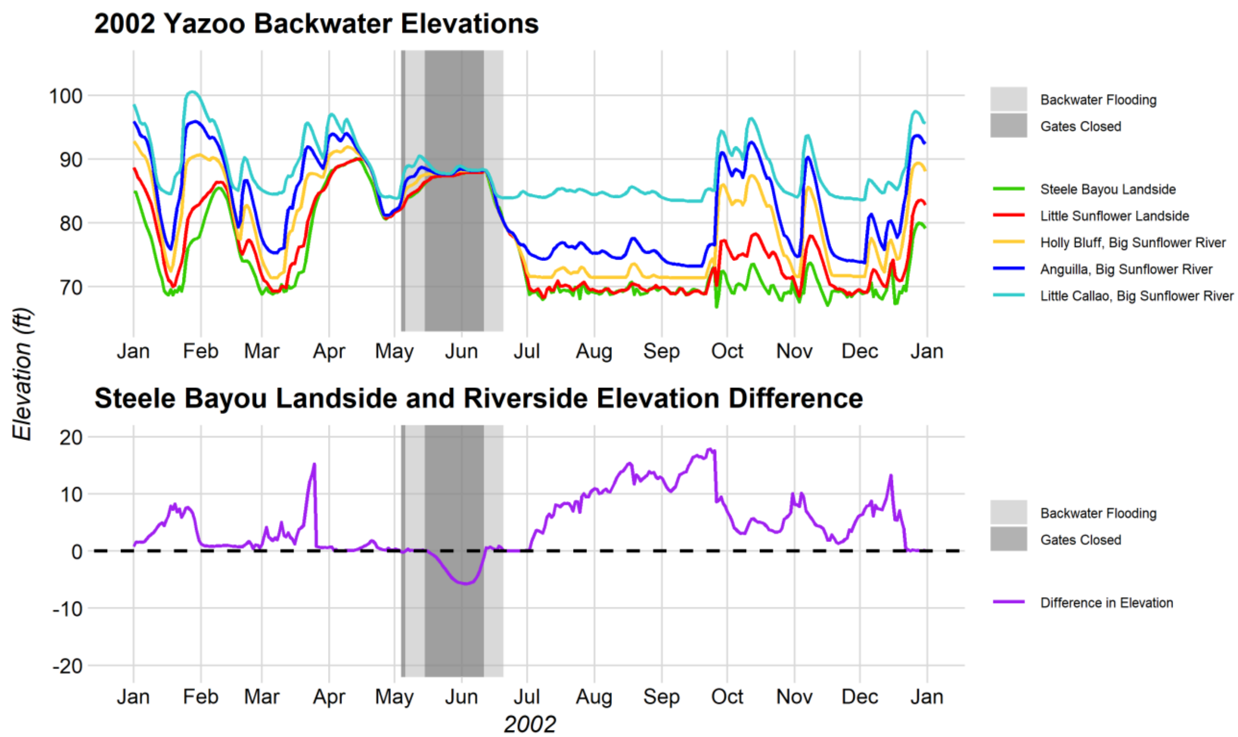

Figure 2-21. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2002 Yazoo Backwater flood.

FLOOD OF 2003

50. The 2003 Yazoo Backwater flood began on 19 May when the Yazoo Backwater elevation exceeded 80.0 feet (NGVD 29) and Steele Bayou landside had a lower elevation than the riverside (Figure 2-22). To reduce backwater flow into the Yazoo Study Area, the Steele Bayou gates were closed from 19 May through 04 June. The Steele Bayou riverside elevation peaked at 91.0 feet (NGVD 29), on 29 May, and the Mississippi River at Vicksburg crested at 89.2 feet (NGVD 29) on 30 May. The Steele Bayou landside peaked on 05 June at 88.3 feet (NGVD 29), and river gages within the Yazoo Backwater began to equalize as flood waters reached their maximum depth. The Yazoo Backwater flood waters were able to recede below an elevation of 80.0 feet (NGVD 29) on 10 June following the decline of elevations on the Mississippi River. The Yazoo Study Area experienced flood conditions for 23 days during 2003, and the annual peak elevation for the Yazoo Study Area occurred during this backwater flood. This flood event would have resulted in the proposed pumps being turned on since the Yazoo Backwater exceeded an elevation of 87.0 feet (NGVD 29).

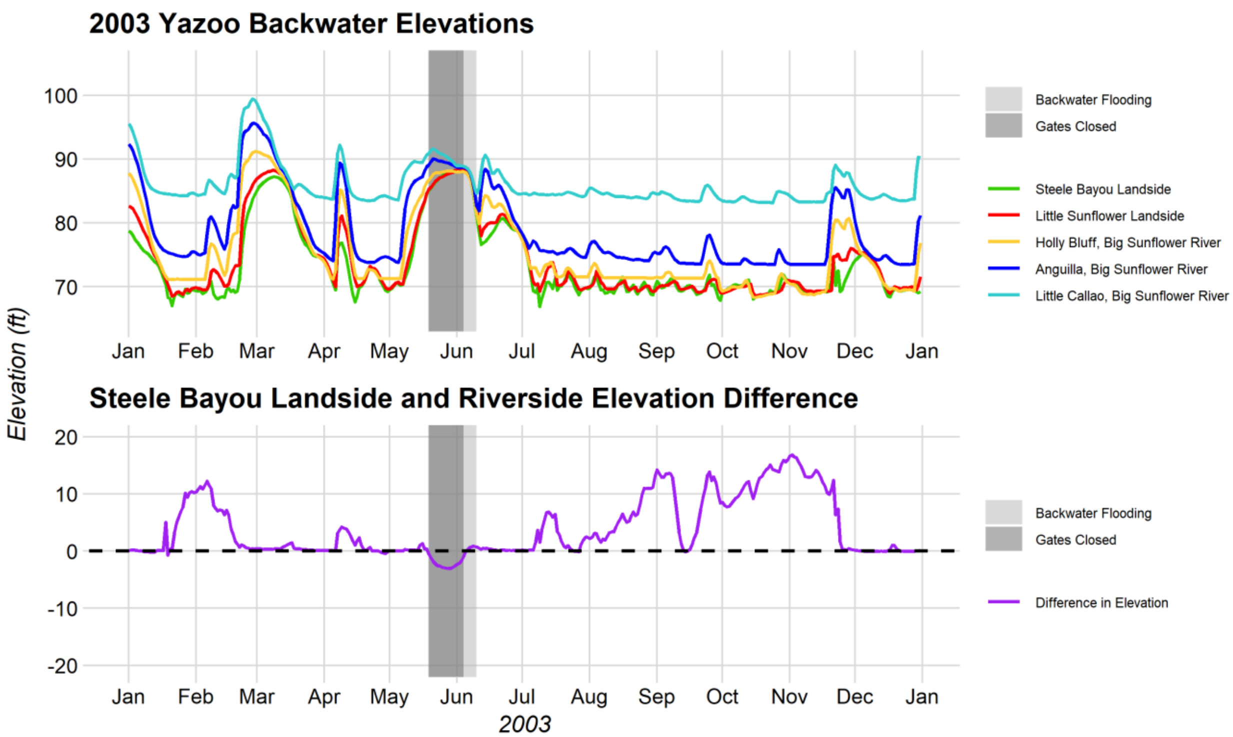

Figure 2-22. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2003 Yazoo Backwater flood.

FLOOD OF 2004

51. During 2004, the Yazoo Backwater experienced headwater flooding from February through March and again from November through December as storm events deposited copious amounts of rainfall across the Yazoo Basin. In fact the highest annual elevation the Steele Bayou landside reached was 87.6 feet (NGVD 29) on 20 December, during the second headwater flood. In addition to the headwater floods, the Yazoo Backwater experienced two backwater-driven floods during 2004. The Yazoo Backwater briefly flooded from 15 March through 27 March due to rising elevations on the Mississippi River. The Steele Bayou gates were closed from 15 March through 24 March as a result (Figure 2-23). During this brief backwater-driven flood event, the Mississippi River at Vicksburg crested at 83.6 feet (NGVD 29) on 21 March, the Steele Bayou riverside elevation peaked at 84.9 feet (NGVD 29) on March 21, and the Steele Bayou landside peaked on 25 March at 83.2 feet (NGVD 29). The Steele Bayou gates were then opened on 25 March, and the Yazoo Backwater receded below the elevation of 80.0 feet (NGVD 29) on 27 March. The Yazoo Backwater was flooded for a total of 13 days during March. The Yazoo Backwater briefly flooded again for 36 days due to backwater conditions from 06 June through 11 July, when the Yazoo Backwater fell below an elevation of 80.0 feet (NGVD 29). The Steele Bayou gates were closed from 06 June until 18 June. The Steele Bayou landside peaked at 84.7 feet (NGVD 29) on 02 July, and the Steele Bayou riverside elevation peaked at 84.0 feet (NGVD 29) on 15 June. Figure 2-23 illustrates the Yazoo Backwater elevations from the most downstream station at Steele Bayou landside to the most upstream station at Little Callao (Big Sunflower River). The cresting of the Yazoo Backwater occurred on 25 March and 02 July. Around the time of both crests, Steele Bayou, Little Sunflower, Holly Bluff (Big Sunflower

River) and Anguilla (Big Sunflower River) elevations converged. In contrast to other major flood events, not all Yazoo Backwater stations equalized in the 2004 flood. The Yazoo Backwater did not exceed 87.0 feet (NGVD 29) during either of these backwater-driven flood events. Therefore, the proposed pumps would not have been turned on.

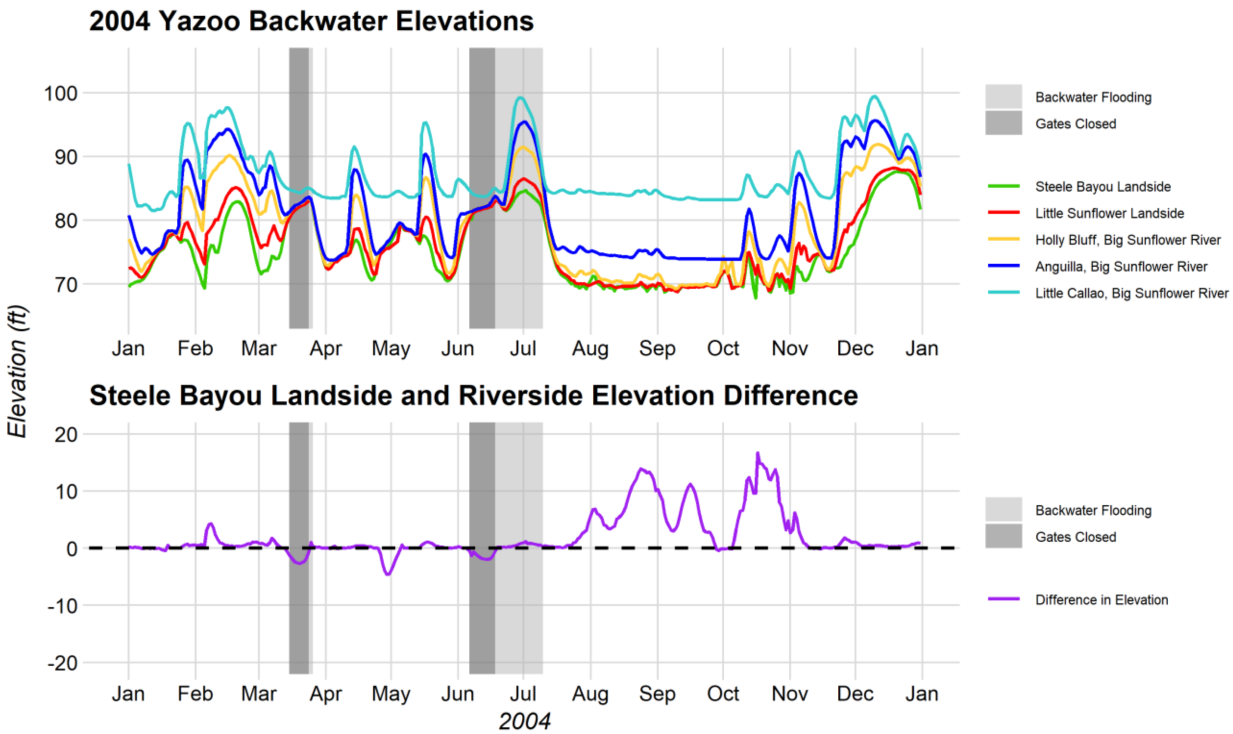

Figure 2-23. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2004 Yazoo Backwater flood.

FLOOD OF 2005

52. The 2005 Yazoo Backwater flood began on 14 January due to a rising Mississippi River. The Mississippi River at Vicksburg crested at 90.7 feet (NGVD 29) on 30 January. The Steele Bayou gates were closed from 15 January through 06 February (Figure 2-24). The Steele Bayou riverside elevation peaked at 92.8 feet (NGVD 29) on 29 January, and roughly a week later, the Steele Bayou landside peaked at 90.0 feet (NGVD 29) on 07 February. The Yazoo Backwater was flooded for 57 days before the flood waters receded below an elevation of 80.0 feet (NGVD 29) on 11 March. Figure 2-24 illustrates the Yazoo Backwater elevations from the most downstream station at Steele Bayou landside to the most upstream station at Little Callao (Big Sunflower River). When the Yazoo Backwater reached a maximum in high water, the elevations at the upstream river gages began to equalize with the elevations at the downstream river gages. Because the Yazoo Backwater surpassed 87.0 feet (NGVD 29), the proposed pumps would have been turned on during this flood event.

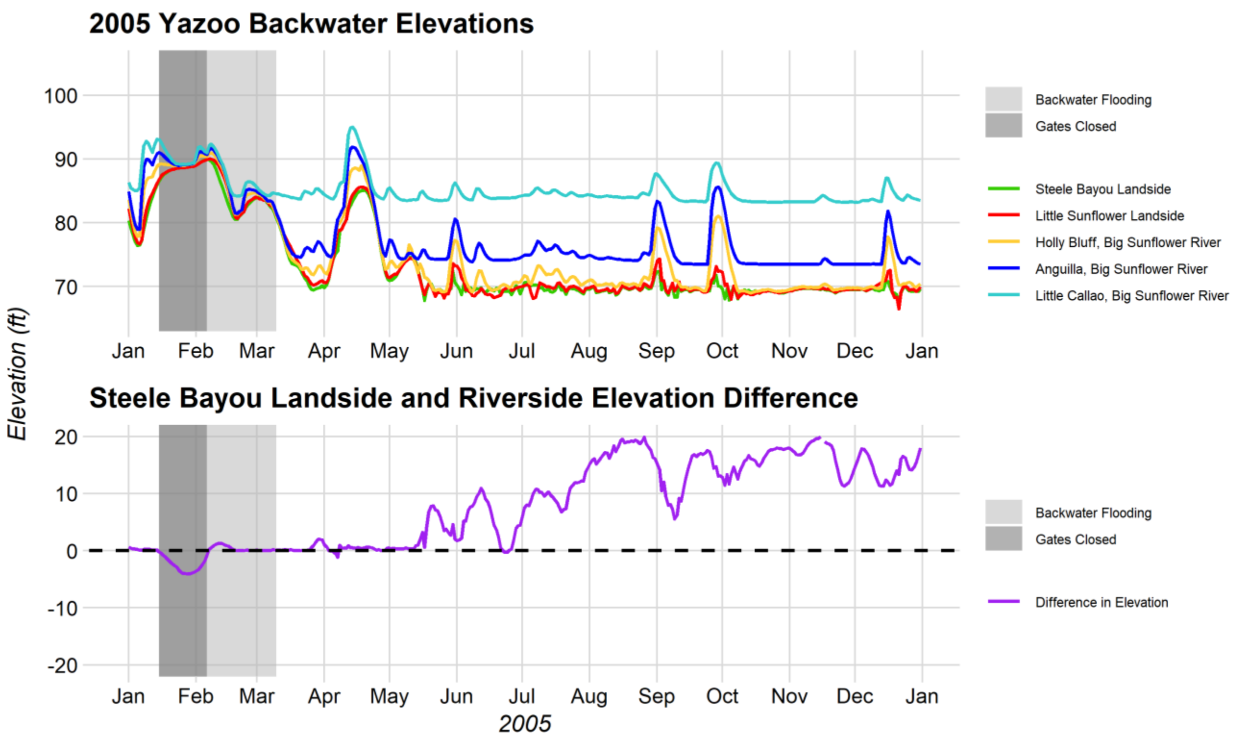

Figure 2-24. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2005 Yazoo Backwater flood.

FLOOD OF 2007

53. During 2007, the Yazoo Backwater experienced headwater flooding during the beginning of January from heavy rainfall. The headwater flooding amplified water levels in the Yazoo Study Area; and when the Mississippi River began to rise, the Yazoo Study Area began to flood from backwater conditions. The backwater flooding began 19 January, and the Steele Bayou gates were closed to prevent backwater flow into the Yazoo Backwater from 19 January through 21 January (Figure 2-25). The Mississippi River at Vicksburg crested at 83.9 feet on 26 January. Consequently, the riverside elevation of the Steele Bayou crested at 85.4 feet (NGVD 29), on 26 January. The Steele Bayou landside elevation peaked at 85.4 feet (NGVD 29), on 25 January, which is the annual peak elevation the Yazoo Backwater experienced during 2007. The elevation of the Yazoo Backwater then fell below 80.0 feet (NGVD 29) on 07 February. The Yazoo Backwater was flooded for 20 days during 2007. Because the Yazoo Backwater did not exceed 87.0 feet (NGVD 29), the proposed pumps would not have been turned on during this flood event.

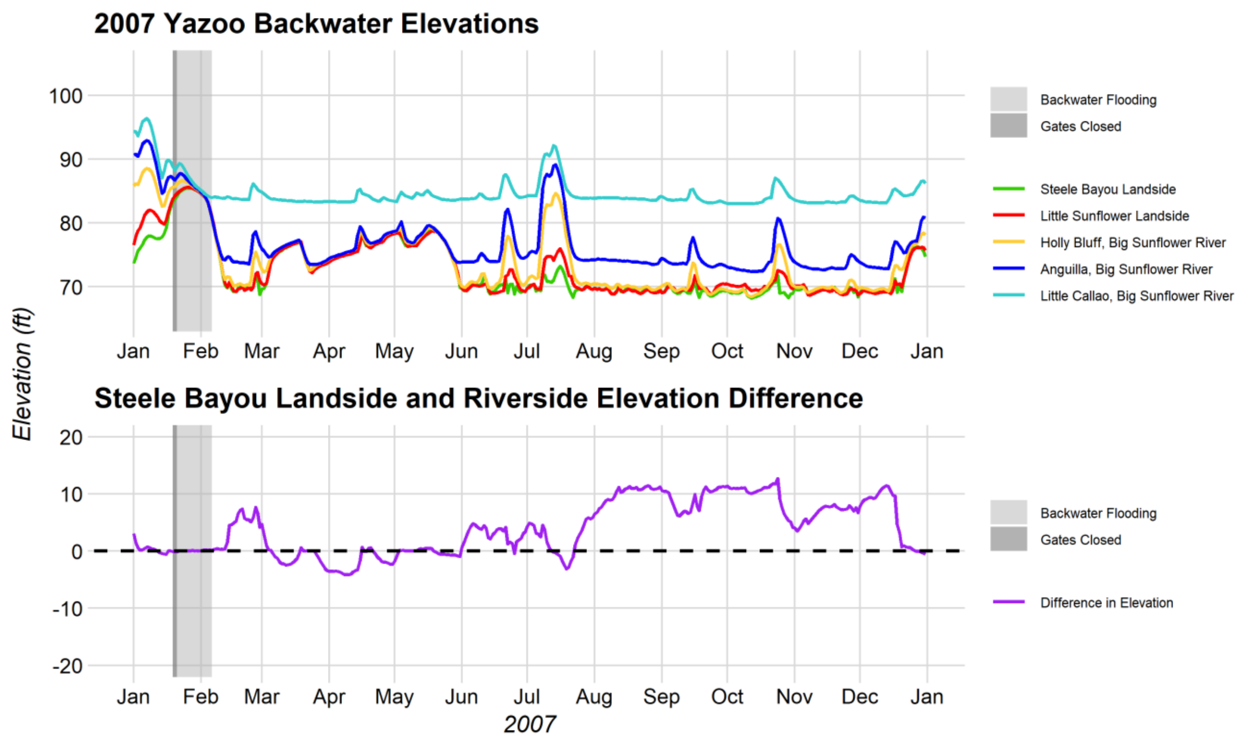

Figure 2-25. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2007 Yazoo Backwater flood.

FLOOD OF 2008

54. The 2008 Yazoo Backwater flood began 18 February due to a rising Mississippi River. After above normal rainfall across the Mississippi River Valley during February and March, the Mississippi River at Vicksburg started rising and crested at 97.1 feet (NGVD 29) on 20 April. The Steele Bayou gates were closed from 18 February through 04 March when the riverside exceeded the landside and when the landside surpassed 80.0 feet (NGVD 29) (Figure 2-26). The gates were then briefly opened before being closed again from 06 March through 09 June and 11 June through 22 July. During the second closure of the gates, the riverside elevation of the Steele Bayou crested at 100.0 feet (NGVD 29) on 23 April, and the landside peaked at 92.2 feet (NGVD 29) on 08 May. The Steele Bayou landside elevation of 92.2 feet (NGVD 29) was the annual peak elevation for the Yazoo Study Area during 2008. After the crest on the Mississippi River, elevations began to fall, allowing the high water within the Yazoo Backwater to recede below an elevation of 80.0 feet (NGVD 29) on 22 July. Overall, the Yazoo Backwater experienced high water above 80.0 feet (NGVD 29) for 156 days during 2008. Figure 2-26 illustrates the Yazoo Backwater elevations from the most downstream station at Steele Bayou landside to the most upstream station at Little Callao (Big Sunflower River). The Yazoo Backwater peaked on 08 May as flood waters reached their maximum level, resulting in the elevation at all river gages within the Yazoo Backwater equalizing soon after. The Yazoo Backwater elevation exceeded 87.0 feet (NGVD 29). Therefore, the proposed pumps would have been turned on during this flood.

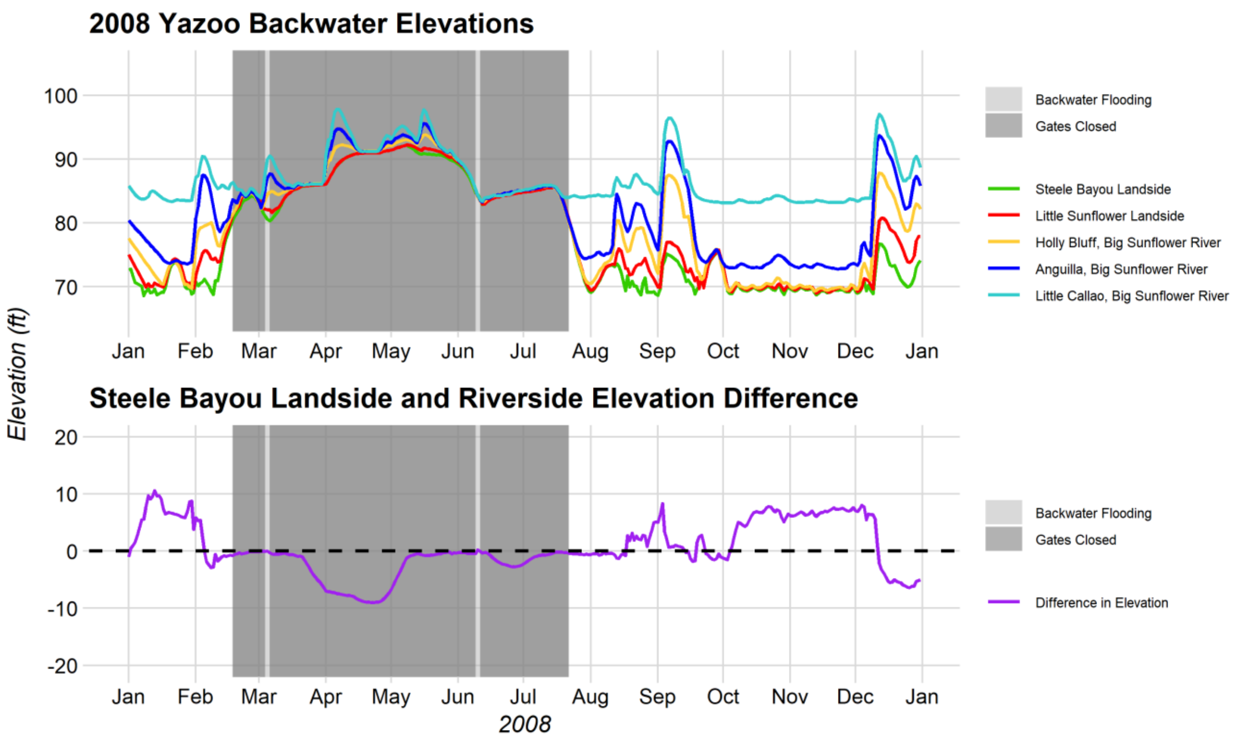

Figure 2-26. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2008 Yazoo Backwater flood.

FLOOD OF 2009

55. During 2009, the Yazoo Backwater experienced numerous flood events due to a rising Mississippi River. The first flood event occurred briefly from 07 January through 12 January after localized heavy rainfall occurred across the Lower Mississippi Valley during December 2008. The Steele Bayou gates were closed from 07 January through 11 January (Figure 2-27). The Mississippi River at Vicksburg peaked at 79.4 feet (NGVD 29) on 08 January, and the Steele Bayou riverside elevation crested at 85.7 feet (NGVD 29) on 09 January. Due to the increasing elevations on the Mississippi River, the Steele Bayou landside peaked at 81.8 feet (NGVD 29) on 09 January, before falling below 80.0 feet (NGVD 29) on 12 January. Because the Yazoo Backwater elevation exceeded 87.0 feet (NGVD 29), the proposed pumps would not have been turned on during this flood event.

56. The second flood event began in March when the Mississippi River at Vicksburg once again began to experience increasing elevations. The Steele Bayou gates were closed 28 March through 30 March and again from 04 April through 01 July (Figure 2-27). The elevation of the flood waters within the Yazoo Backwater exceeded 80.0 feet (NGVD 29) on 28 March. Then the Mississippi River at Vicksburg crested at 93.8 feet (NGVD 29) on 28 May. Similarly, the Steele Bayou riverside elevation peaked at 96.6 feet (NGVD 29) on 28 May. As a result of the increasing backwater, the Yazoo Backwater peaked on 04 June at 93.7 feet (NGVD 29), which was the annual peak elevation for the Yazoo Study Area during 2009. The second flood event receded below an elevation of 80.0 feet (NGVD 29) on 01 July. Because the Yazoo Backwater

elevation exceeded 87.0 feet (NGVD 29), the proposed pumps would have been turned on during this flood event.

57. The third Yazoo Backwater flood began 15 October, which consisted of backwater fluctuating above and below 80.0 feet (NGVD 29) throughout the remainder of the year (Figure 2-27). The downstream United States received anywhere from 200 to more than 300 percent of normal precipitation during October. Specifically, Mississippi received almost 10 inches of rainfall, making it the second wettest October from 1895 through 2009. The influx of copious rainfall led to high water conditions on the Mississippi River and within the Yazoo Study Area. The Yazoo Backwater elevation experienced significant fluctuations resulting from the opening and closing of the Steele Bayou gates in an attempt to release flood waters. The Steele Bayou gates were closed eight times during this flood event with the periods from 16 October through 28 October and 31 October through 12 November being the longest consecutive periods the gates were closed. During the flood event, on 12 November, the Mississippi River at Vicksburg, the Steele Bayou riverside, and the Steele Bayou landside crested at 86.3, 88.2, and 88.1 feet (NGVD 29), respectively. The third flood event receded below an elevation of 80.0 feet (NGVD 29) on 29 November. The Yazoo Backwater was above an elevation of 80.0 feet (NGVD 29) for 148 days during 2009. Because the Yazoo Backwater elevation exceeded 87.0 feet (NGVD 29), the proposed pumps would have been turned on during this flood event.

58. Figure 2-27 illustrates the Yazoo Backwater elevations from the most downstream river gage at Steele Bayou landside to the most upstream river gage at Little Callao (Big Sunflower River). The cresting of the Yazoo Backwater is indicated by the majority of the gages equalizing around 09 January, 04 June, and 12 November.

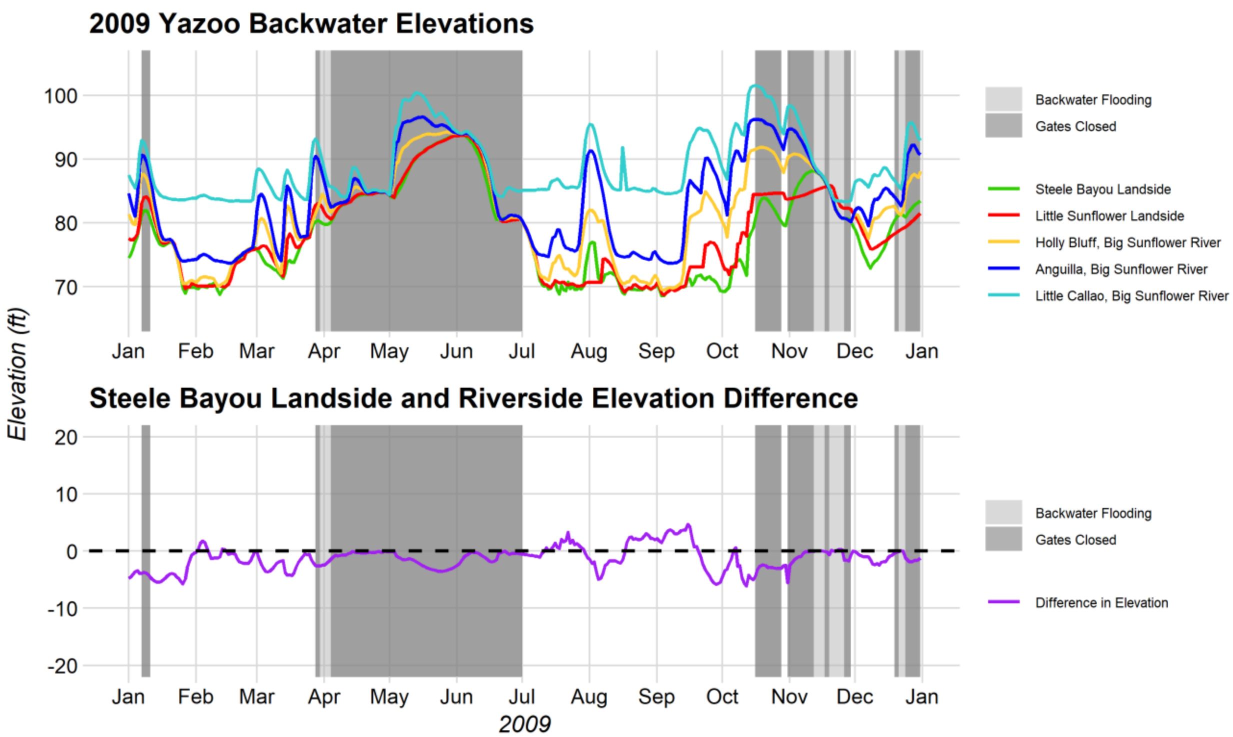

Figure 2-27. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2009 Yazoo Backwater flood.

FLOOD OF 2010

59. The 2010 Yazoo Backwater flood event began as a continuation of the third 2009 Yazoo Backwater flood. The Yazoo Backwater continued to fluctuate above and below 80.0 feet (NGVD 29) from January through July. These fluctuations were driven by heavy rainfall events and backwater flow into the Yazoo Backwater, resulting in the opening and closing of the Steele Bayou flood control structure. The Steele Bayou gates were closed seven times during 2010, with 15 February through 01 March being the longest, consecutive closure (Figure 2-28). The Steele Bayou landside elevation exceeded 80.0 feet (NGVD 29) on 18 December 2009 and remained above 80.0 feet (NGVD 29) through January 2010. The Steele Bayou landside then peaked on 06 January at 85.6 feet (NGVD 29) and the riverside elevation peaked at 85.6 feet (NGVD 29) on 05 January. Because the Yazoo Backwater elevation did not exceed 87.0 feet (NGVD 29), the proposed pumps would not have been turned on during this flood event. The flood receded below elevation of 80.0 feet (NGVD 29) on 12 January.

60. A second Yazoo Backwater flood event then began on 28 January. The Steele Bayou landside crested at 89.8 feet (NGVD 29) on 13 February and the riverside elevation peaked at 89.9 feet (NGVD 29) on 13 February. The Yazoo Backwater elevation of 89.8 feet (NGVD 29) was the annual peak elevation the Yazoo Study Area experienced during 2010. Because the Yazoo Backwater elevation exceeded 87.0 feet (NGVD 29), the proposed pumps would have been turned on during this flood event. The flood receded below an elevation of 80.0 feet (NGVD 29) on 01 March. Figure 2-28 illustrates the Yazoo Backwater elevations from the most downstream river gage at Steele Bayou landside to the most upstream gage at Little Callao. The

Yazoo Backwater peaked on 13 February and flood waters reached their maximum level resulting in the elevation of all stations equalizing soon after.

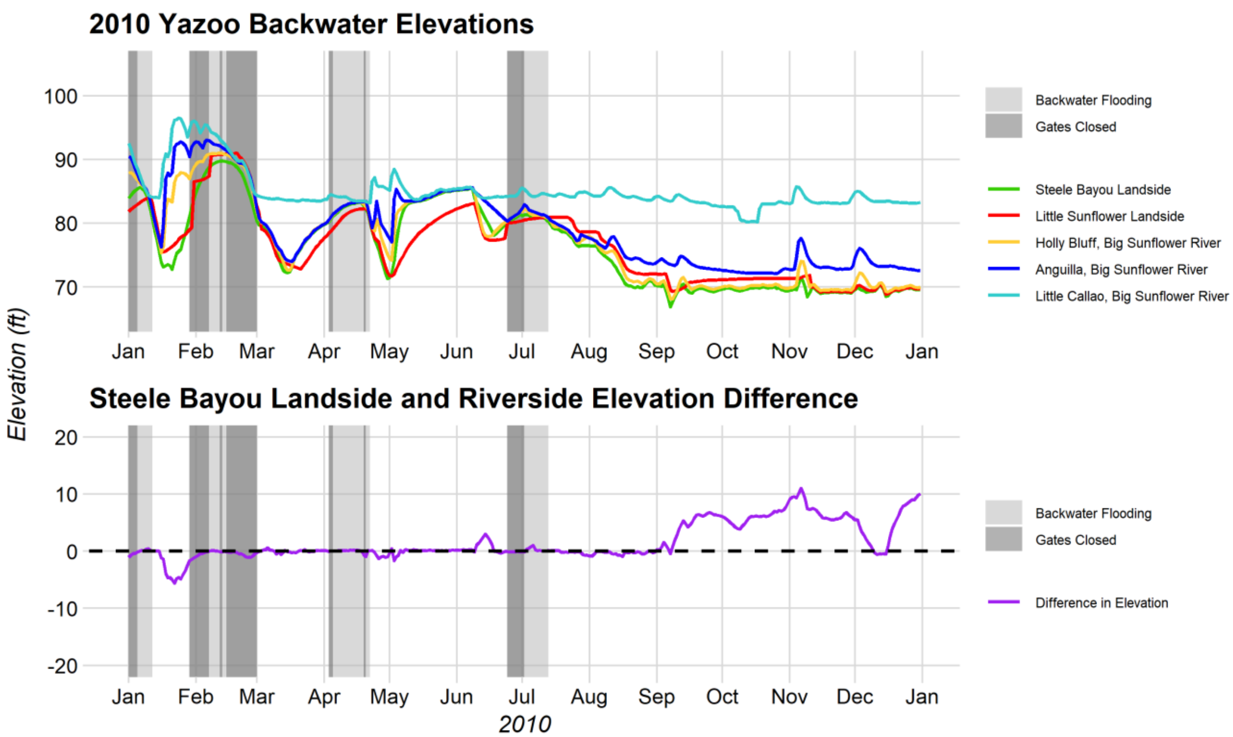

Figure 2-28. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2010 Yazoo Backwater flood.

FLOOD OF 2011

61. The 2011 Yazoo Backwater flood began on 10 March due to a rising Mississippi River. The Mississippi River began to swell due to two major storm systems that deposited record levels of rainfall over the Mississippi River Valley. Thus, the Mississippi River at Vicksburg peaked at 103.4 feet (NGVD 29) on 18 May. The Steele Bayou landside peaked on 29 May at 90.0 feet, (NGVD 29) and the riverside elevation peaked 106.2 feet (NGVD 29) on 19 May. The Steele Bayou landside elevation of 90.0 feet (NGVD 29) was the annual peak elevation for the Yazoo Backwater during 2011. The flood receded below an elevation of 80.0 feet (NGVD 29) on 19 July. During this flood event, the Steele Bayou gates were closed from 10 March through 20 April and 22 April through 19 July (Figure 2-29). The Yazoo Backwater was flooded for a total of 132 days during 2011. Figure 2-29 also illustrates the Yazoo Backwater elevations from the most downstream river gage at Steele Bayou landside to the most upstream river gage at Little Callao (Big Sunflower River). The Yazoo Backwater peaked on 29 May, and flood waters reached their maximum level resulting in all stations equalizing. Because the Yazoo Backwater elevation exceeded 87.0 feet (NGVD 29), the proposed pumps would have been turned on during this flood event.

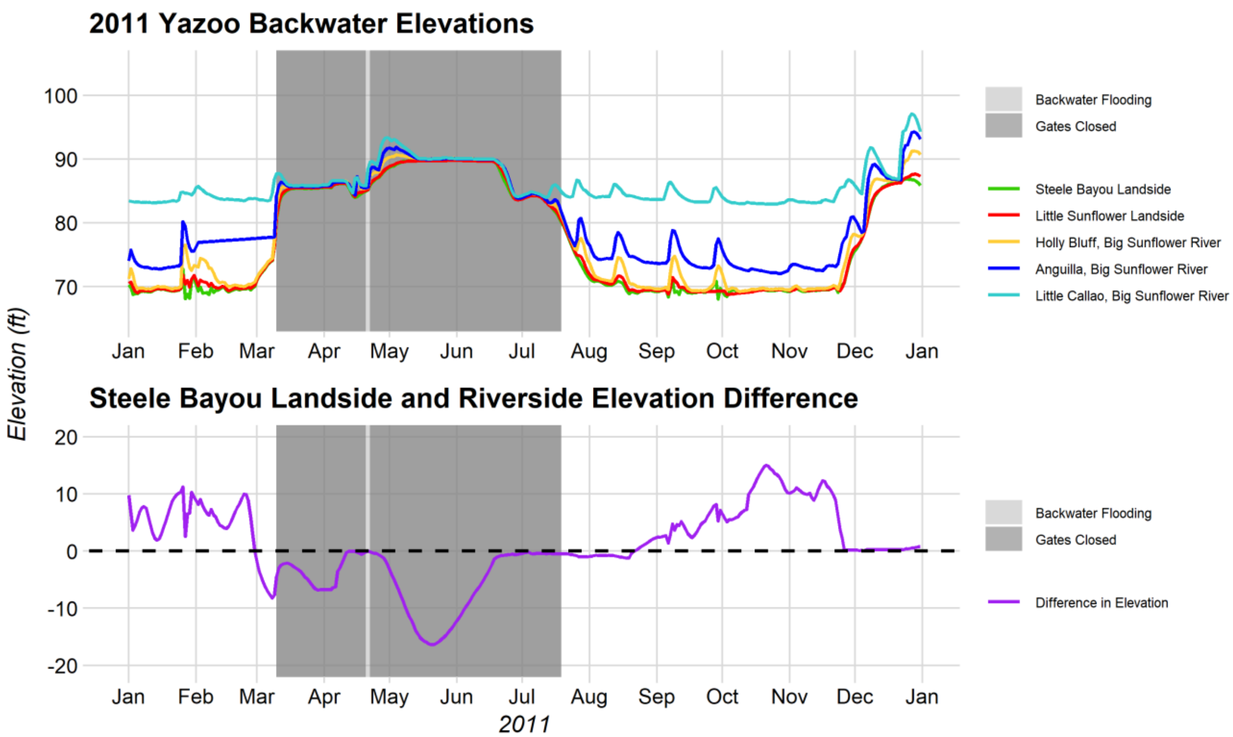

Figure 2-29. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2011 Yazoo Backwater flood.

FLOOD OF 2013

62. The 2013 Yazoo Backwater flood began on 29 April due to a rising Mississippi River. Heavy rainfall events resulted in the Lower Mississippi Valley receiving more than eight inches above normal monthly precipitation in January and 0.5 to three inches above normal precipitation in February. Because heavy rainfall events occurred earlier in the year, the Mississippi River at Vicksburg began to experience rises in elevations during March. Then above normal rainfall in the Upper Mississippi Valley during April and May further amplified river flow along the Mississippi River downstream. Consequently, the Mississippi River at Vicksburg peaked at 90.5 feet (NGVD 29) on 24 May. The high water on the Mississippi River prompted flood waters to enter the Yazoo Study Area. As a result, the Steele Bayou gates were closed 29 April through 28 May, 12 June through 22 June, and 15 July through 23 July (Figure 2-30). The Steele Bayou riverside elevation peaked during the first gate closure at 92.3 feet (NGVD 29) on 22 May, and the Steele Bayou landside peaked on 29 May at 90.9 feet (NGVD 29). The Yazoo Backwater elevation of 90.9 feet (NGVD 29) was the maximum elevation the Yazoo Study Area experienced during 2013. Flood waters within the Yazoo Backwater finally receded below an elevation of 80.0 feet (NGVD 29) on 26 July. Figure 2-30 illustrates the Yazoo Backwater elevations equalizing soon after the Yazoo Backwater reached a maximum in high water elevation. Overall, the Yazoo Backwater was above 80.0 feet (NGVD 29) for 79 days during the 2013 flood event. Because the Yazoo Backwater elevation exceeded 87.0 feet (NGVD 29), the proposed pumps would have been turned on during this flood event.

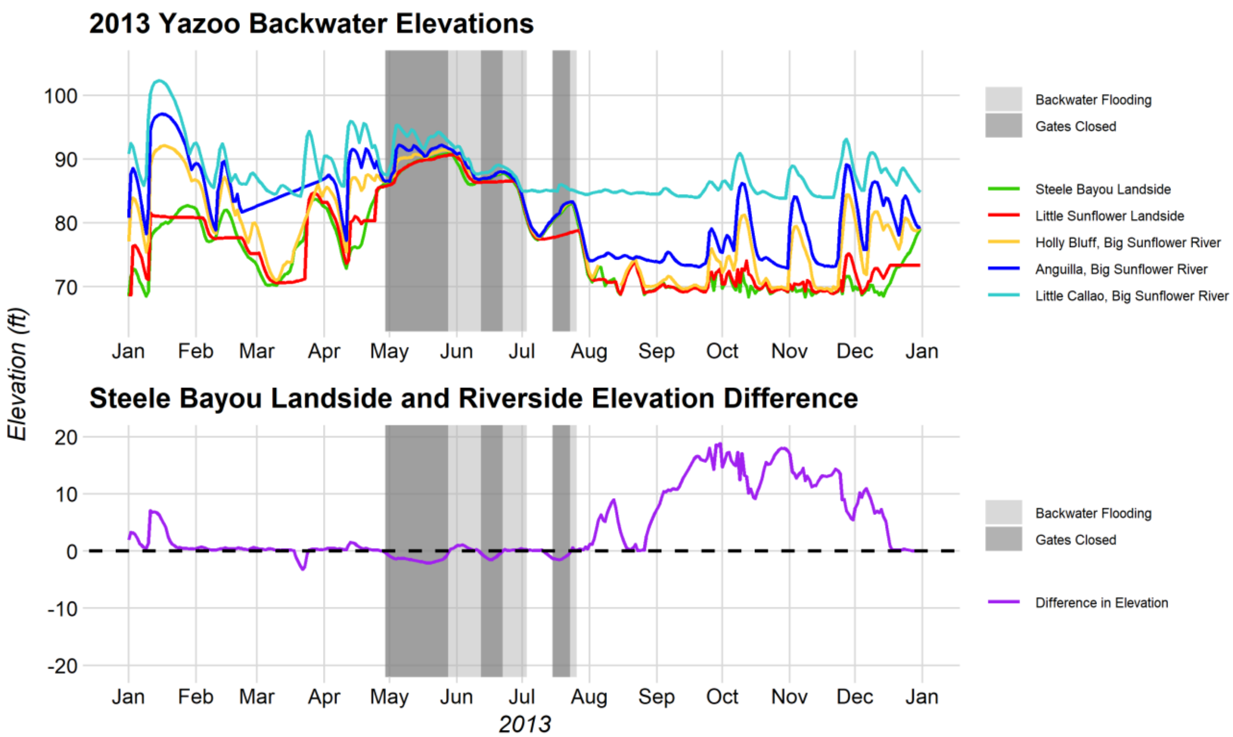

Figure 2-30. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2013 Yazoo Backwater flood.

FLOOD OF 2014

63. The Yazoo Backwater also experienced flood conditions during 2014. The Steele Bayou gates were closed twice, from 04 January through 07 January and from 10 May through 12 May, due to a rising Mississippi River (Figure 2-31). In between these backwater-driven flood events, the Yazoo Study Area experienced an annual peak elevation of 86.8 feet (NGVD 29) on 20 April due to a headwater-driven flood event. The backwater-driven flood event that occurred during May was more significant than the backwater-driven flood event that occurred during January. The Mississippi River at Vicksburg peaked on 21 April at 84.7 feet (NGVD 29) after receiving more than eight inches above monthly normal precipitation. The Steele Bayou riverside elevation peaked at 81.2 feet (NGVD 29) on 12 May, and the Steele Bayou landside elevation peaked at 81.2 feet (NGVD 29) on 13 May. The Yazoo Backwater was flooded for 23 days before the flood receded below an elevation of 80.0 feet (NGVD 29) on 02 June. Because the Yazoo Backwater elevation did not exceed 87.0 feet (NGVD 29), the proposed pumps would not have been turned on during this flood event.

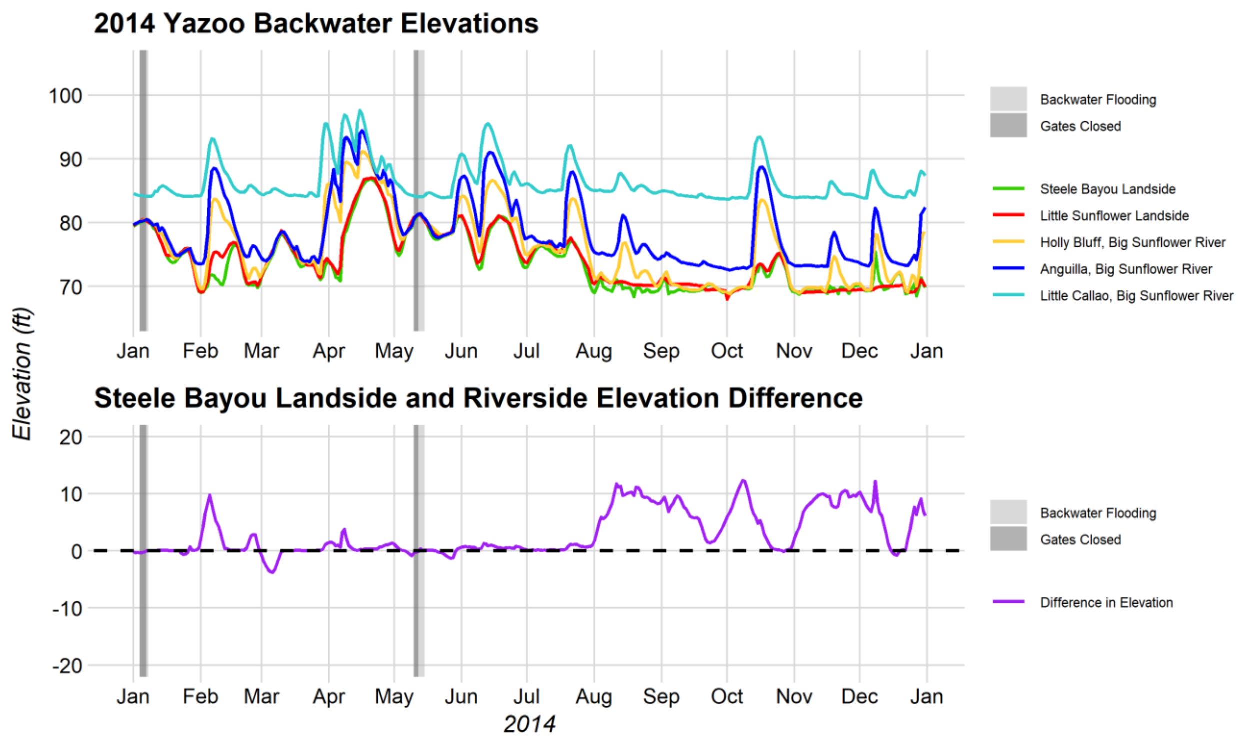

Figure 2-31. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2014 Yazoo Backwater flood.

FLOOD OF 2015

64. During 2015, the Yazoo Backwater began to experience increases in elevations due to headwater flooding at the beginning of March. Then, the Yazoo Backwater began to experience backwater-driven flooding on 22 March due to a rising Mississippi River. The Upper Mississippi Valley received above normal precipitation for March, April, June and July, which consequently increased elevations on the Mississippi River downstream. The Mississippi River at Vicksburg began to experience dramatic increases in elevations in March and remained elevated before cresting at 92.2 feet (NGVD 29) on 29 July. The Steele Bayou landside peaked on 04 April at 90.6 feet (NGVD 29), which was the maximum elevation for 2015. Because the Yazoo Backwater elevation exceeded 87.0 feet (NGVD 29), the proposed pumps would have been turned on during this flood event. The Steele Bayou riverside elevation peaked at 94.1 feet (NGVD 29) on 27 July. Because the Mississippi River at Vicksburg crested later than the Yazoo Backwater, the Yazoo Backwater was unable to drain and experienced prolonged flooding. In addition, the Steele Bayou gates were closed five times, with 23 June through 07 August being the longest, consecutive closure (Figure 2-32). Flood conditions existed within the Yazoo Backwater for 145 days before high water receded below an elevation of 80.0 feet (NGVD 29) on 13 August. Figure 2-32 also illustrates the elevations at all Yazoo Backwater river gages equalizing as flood waters peaked.

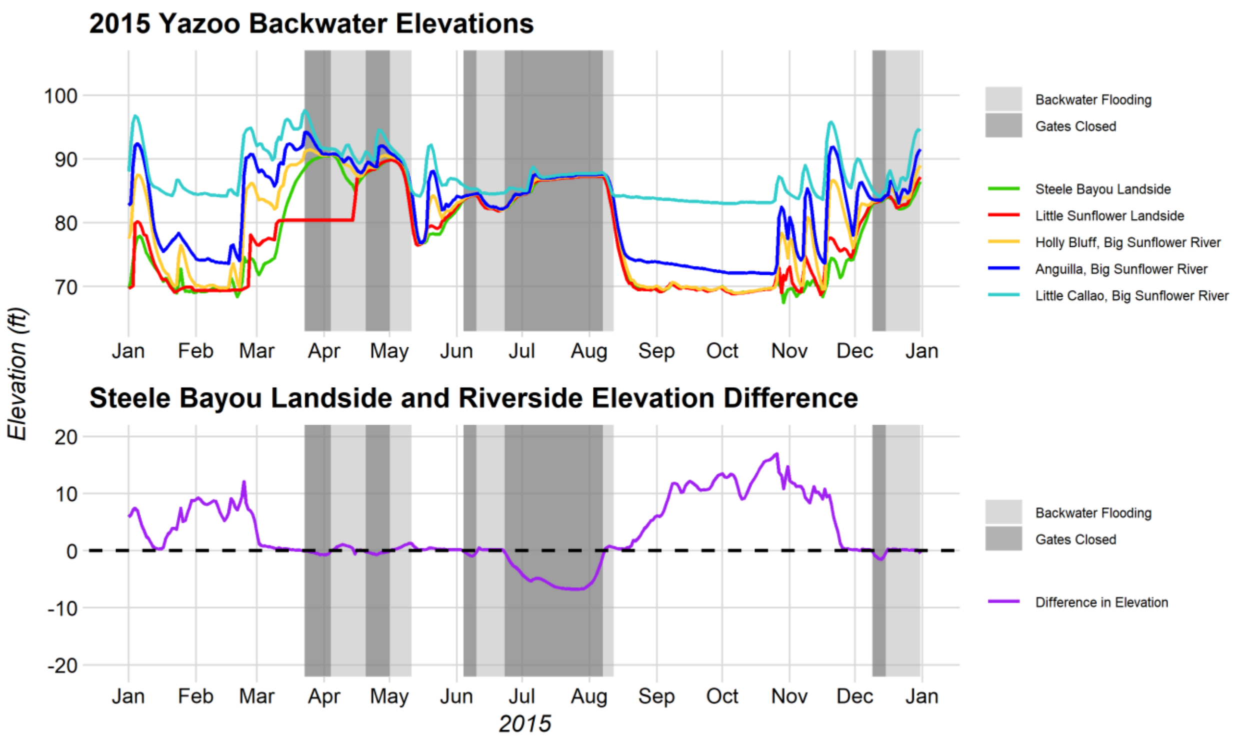

Figure 2-32. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2015 Yazoo Backwater flood.

FLOOD OF 2016

65. The 2016 Yazoo Backwater flood began on 05 December 2015 due to a rising Mississippi River. After an abnormally wet fall and significant rainfall events in December 2015, the Mississippi River at Vicksburg was roughly 86.3 feet (NGVD 29) on 01 January before increasing 10 feet to 96.4 feet (NGVD 29) on 16 January. Similarly, the Steele Bayou riverside also crested on 16 January at 99.3 feet (NGVD 29). The Steele Bayou landside did experience a minor crest at 91.4 feet (NGVD 29) on 29 January. However, the major crest occurred on 21 March at 92.0 feet (NGVD 29) due to a secondary rise in elevation of the Steele Bayou riverside and the Mississippi River at Vicksburg. This crest was the highest elevation the Yazoo Backwater experienced during 2016 and was associated with the backwater flood conditions, but was further amplified from significant headwater-driven flooding, which occurred concurrent to the backwater-driven flooding. Because the Yazoo Backwater elevation exceeded 87.0 feet (NGVD 29), the proposed pumps would have been turned on during this flood event. Flood waters receded below elevation of 80.0 feet (NGVD 29) on 09 June, 202 days after the start of flood conditions within the Yazoo Backwater. During this flood event, the Steele Bayou gates were closed five times, with 01 January through 28 January being the longest, consecutive closure (Figure 2-33). Figure 2-33 also illustrates the Yazoo Backwater elevations equalizing as flood waters reached their maximum level.

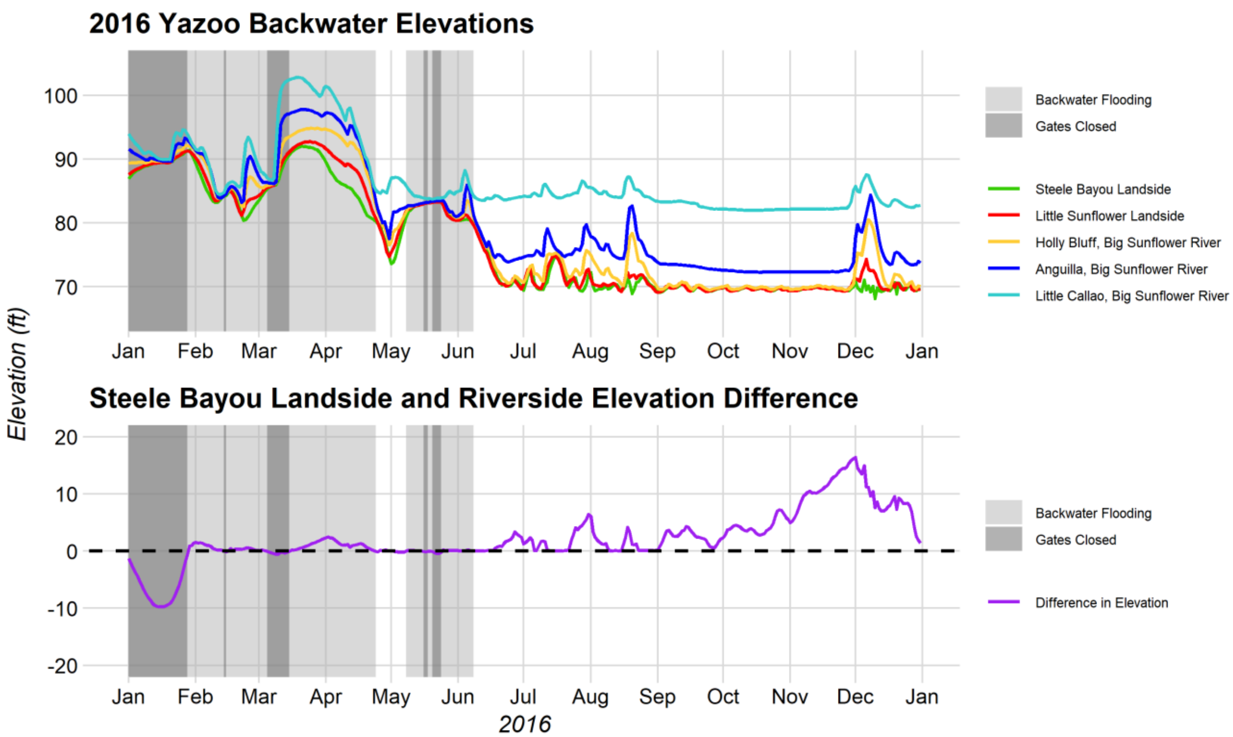

Figure 2-33. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2016 Yazoo Backwater flood.

FLOOD OF 2017

66. The 2017 Yazoo Backwater flood began on 02 May due to a rising Mississippi River. The Mid-Mississippi Valley received more than eight inches above normal monthly precipitation during April 2017. As a result of the abundant rainfall upstream, the Mississippi River at Vicksburg peaked on 26 May at 94.6 feet (NGVD 29). The Steele Bayou riverside elevation crested at 97.1 feet (NGVD 29) on 27 May, and the Steele Bayou landside elevation peaked on 16 June at 88.5 feet (NGVD 29), which was the maximum annual elevation for the Yazoo Backwater during 2017. Because the Yazoo Backwater elevation exceeded 87.0 feet (NGVD 29), the proposed pumps would have been turned on during this flood event. The Yazoo Backwater experienced flood conditions for 53 days, before the high water elevation fell below 80.0 feet (NGVD 29) on 23 June. In addition, the Steele Bayou gates were closed from 02 May through 15 June, during this event (Figure 2-34). Figure 2-34 illustrates the Yazoo Backwater elevations from the most downstream station at Steele Bayou landside to the most upstream station at Little Callao (Big Sunflower River). The Steele Bayou landside structure crested on 16 June when the Yazoo Backwater reached a maximum in high water elevation. Around this time, more upstream river gages were also experiencing rises in elevation and began to equalize with the downstream river gages.

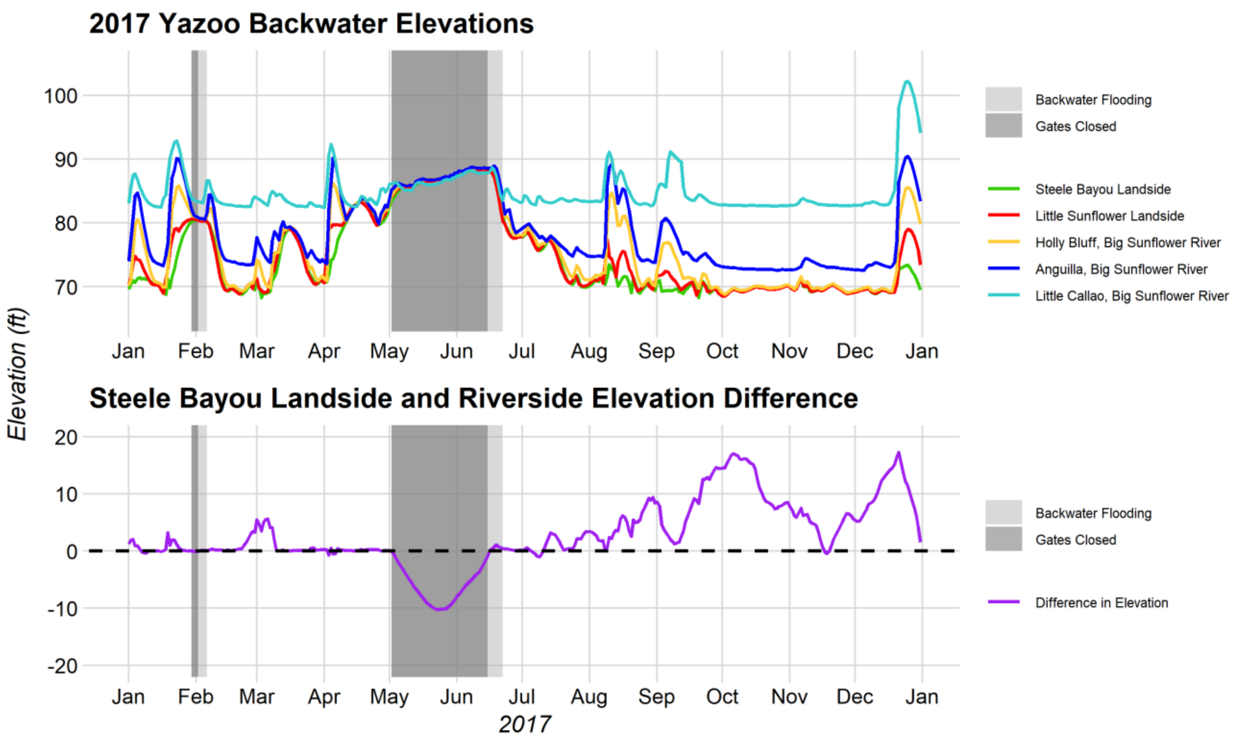

Figure 2-34. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2017 Yazoo Backwater flood.

FLOOD OF 2018

67. During 2018, the Yazoo Basin received significant rainfall and headwater flooding began during February as the Lower Mississippi Valley received more than eight inches above normal monthly precipitation during February. The Steele Bayou landside reached a maximum annual elevation of 95.1 feet (NGVD 29) on 25 March due to the headwater-driven flooding. The above normal rainfall also resulted in elevations on the Mississippi River at Vicksburg increasing during February, which initiated backwater-driven flooding. As a result of the backwater-driven flooding, the gates at the Steele Bayou control structure were closed 01 March through 25 March (Figure 2-35). The Mississippi River at Vicksburg peaked at 96.1 feet (NGVD 29) on 15 March. As a result, the Steele Bayou riverside elevation peaked at 99.0 feet (NGVD 29) on 16 March, and the Steele Bayou landside elevation peaked at 95.1 feet (NGVD 29) on 25 March. Because the Steele Bayou elevation exceeded 87.0 feet (NGVD 29), the proposed pumps would have been turned on during this backwater-driven flood event. The Yazoo Backwater was above an elevation of 80.0 feet (NGVD 29) for a total of 81 days during 2018 before receding on 20 May. In addition, two minor backwater events occurred during November and December, forcing the Steele Bayou gates closed for four days (Figure 2-35).

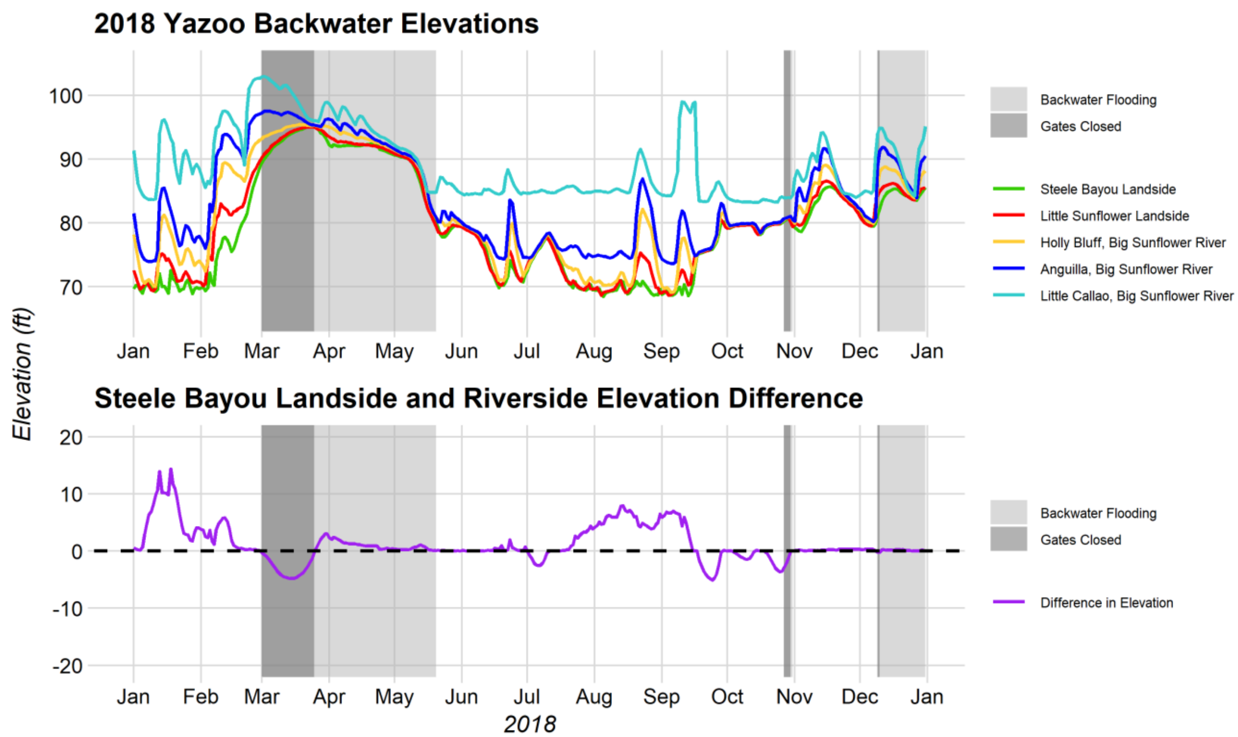

Figure 2-35. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2018 Yazoo Backwater flood.

FLOOD OF 2019

68. The 2019 Yazoo Backwater flood began in the fall of 2018 due to an abnormally wet season. Frequent rain events from January through July, resulted in persistent, increased elevations on the Mississippi River. Additionally, an extended closure of the Steele Bayou gates further amplified flood conditions. Steele Bayou was closed five times during 2019, with 15 February through 01 April being the longest, consecutive closure (Figure 2-36). In addition, the steady spring rainfalls occurred subsequent to an abnormally wet winter season, which further amplified the above normal stream flow during the spring months. Flood conditions within the Yazoo Backwater began 09 January, when the Steele Bayou riverside exceeded the Steele Bayou landside, and the Steele Bayou landside was above an elevation of 80.0 feet (North American Vertical Datum of 1988 [NAVD 88]). The last week in February, multiple storm systems propagated across the Lower Mississippi Valley and deposited more than 10 inches of rainfall across the region. As a result, the Mississippi River at Vicksburg increased from an elevation of 90.3 feet (NAVD 88) on 20 February to a peak elevation of 97.6 feet (NAVD 88) on 13 March. The increased elevations on the Mississippi River resulted in water backing up to the Steele Bayou Control Structure. Thus, the Steele Bayou riverside peaked on 12 March at 100.0 feet (NAVD 88). The Steele Bayou landside experienced a minor crest on 31 March at 97.2 feet (NAVD 88). After the significant rainfall in the last week of February, elevations on the Mississippi River at Vicksburg and the Steele Bayou riverside started to fall. However, the Steele Bayou flood control structure gates remained closed throughout March, preventing the Yazoo Backwater to drain.

69. On 01 April, the control structure was opened, allowing the Yazoo Backwater to drain slightly. However, multiple heavy rainfall events throughout May produced increases in elevation on the Mississippi River at Vicksburg and the Steele Bayou riverside, forcing the Steele Bayou gates closed. This second closure resulted in the Steele Bayou landside experiencing its primary crest at 98.2 feet (NAVD 88) on 23 May. This crest was the maximum elevation the Yazoo Backwater obtained during 2019. After the crest within the Yazoo Backwater, the Steele Bayou gates were opened, but were closed on 07 June to prevent backflow into the Yazoo Backwater. The closure of the control structure kept the Steele Bayou landside at an elevation around 97.0 feet (NAVD 88), for May, June, and most of July. It was not until the third week in July when the Yazoo Backwater began to experience significant declines in elevation.

70. From 1973 through 2018, the Steele Bayou landside elevation exceeded 95.0 feet (NAVD 88) for 124 days, with the longest duration above 95.0 feet (NAVD 88) being 68 days from 09 April 1973 through 15 June 1973. During 2019, the Yazoo Backwater was above an elevation of 80.0 feet (NAVD 88) from 09 January to 16 August, or 219 days, and was above 95.0 feet (NAVD 88) for 145 days from 05 March through 27 July. The duration of high water, above 95.0 feet (NAVD 88), during 2019 was more than twice the longest duration of high water that occurred in 1973. Because the Yazoo Backwater elevation exceeded 87.0 feet (NAVD 88) during the 2019 flood event, the proposed pumps would have been turned on. Figure 2-36 illustrates the Yazoo Backwater elevations from the most downstream station at Steele Bayou landside to the most upstream station at Little Callao (Big Sunflower River). In contrast to other major flood events, all of the Yazoo Backwater gages converged in the 2019 flood and remained equalized for the majority of the flood event due to the extreme, prolonged high water conditions.

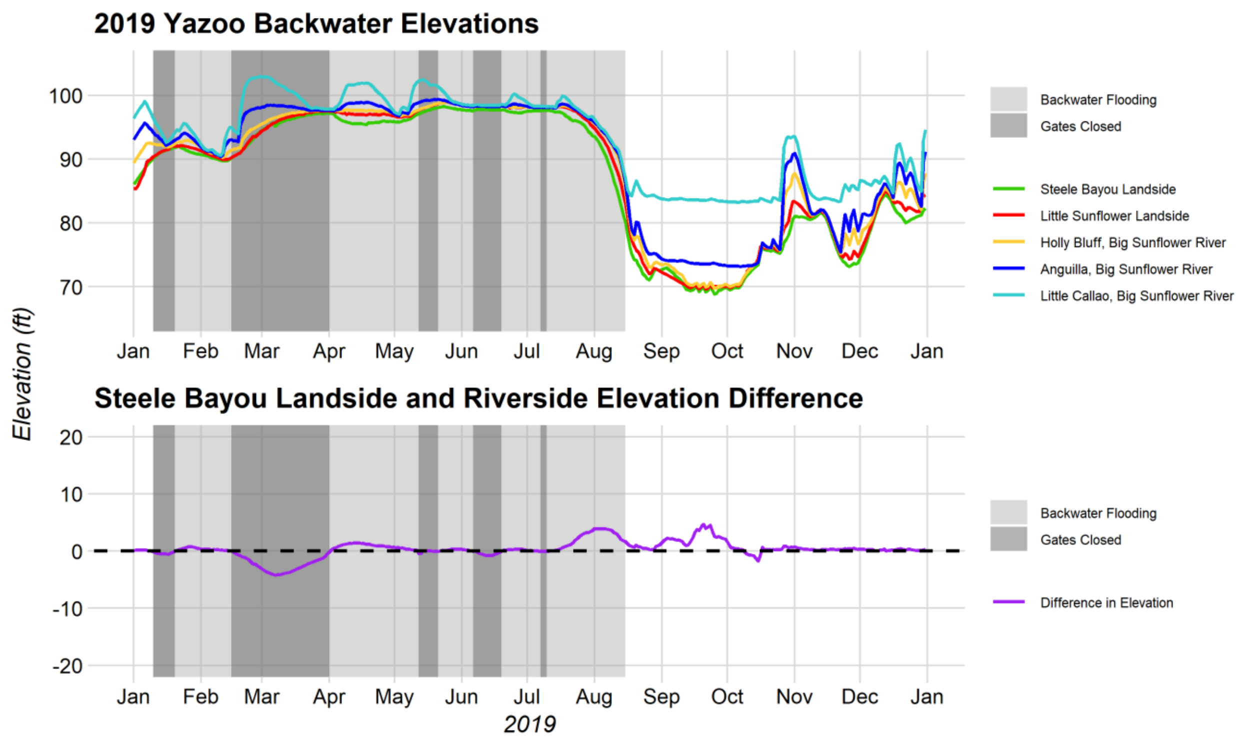

Figure 2-36. The Yazoo Backwater elevations and the Steele Bayou landside and riverside elevation differences for the 2019 Yazoo Backwater flood.

FLOOD CONTROL

PROJECT FEATURES

71. Completed flood control projects in the Yazoo Backwater Area, or the Yazoo Study Area, are shown in Figure 2-37. These features include the following:

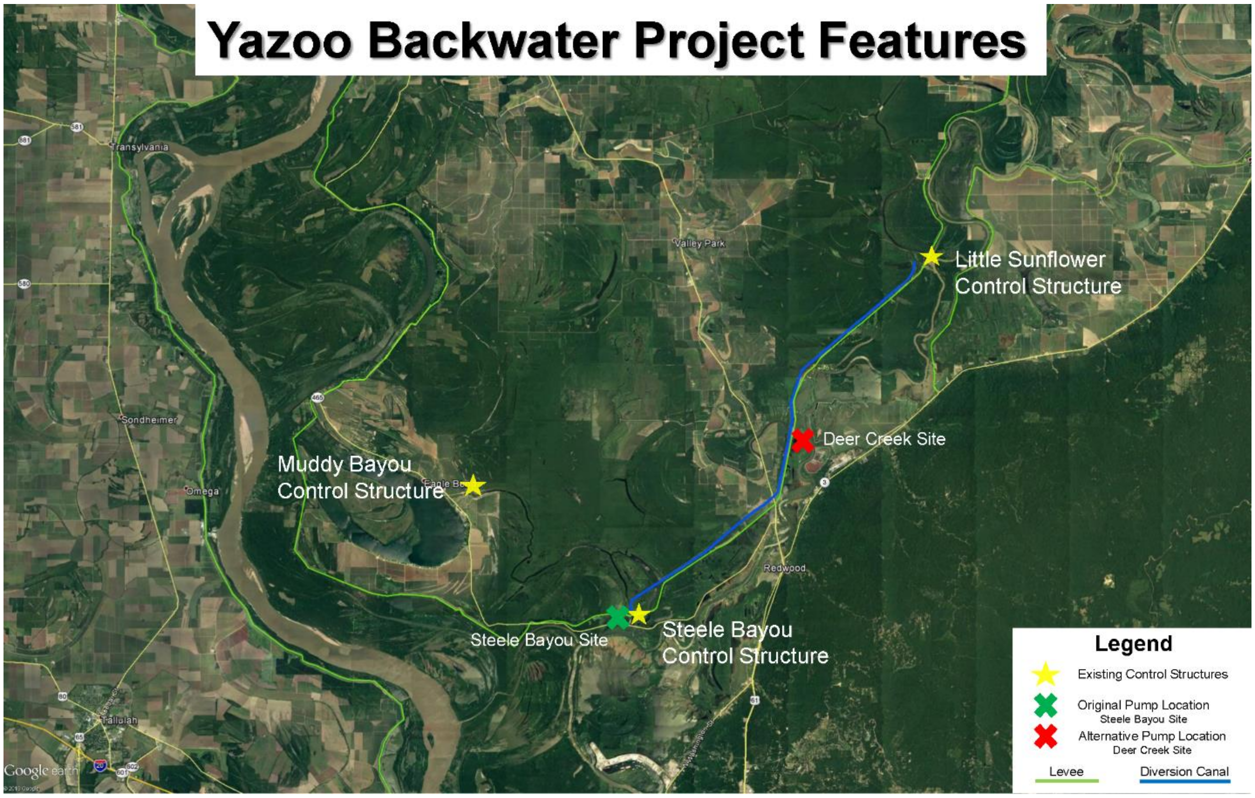

Figure 2-37. The flood control projects in the Yazoo Backwater Area.

72. Yazoo Backwater Levee connects to the end of the east bank Mississippi River levee just north of Vicksburg and extends north eastward to the downstream end of the west bank Will M. Whittington Lower Auxiliary Channel Levee. The Yazoo Backwater levee has a net levee grade of elevation 107.0 feet (NGVD 29). The Yazoo Backwater levee is considered an overtopping section to the mainline levee of the Mississippi River, except for 1,000 feet on each side of the Steele Bayou and Little Sunflower structures. These 30.5 miles of overtopping levee ensure that in case of the MR&T Project Design Flood (PDF), the storage in the Yazoo Study Area will be utilized to reduce the risk of overtopping the main stem levee.

73. Steele Bayou structure is located 3,200 feet upstream of the confluence of Steele Bayou and the Yazoo River. The structure consists of four vertical lift gates 30 by 22.5 feet, concrete-paved approach channel, and a stilling basin. The Steele Bayou ponding area is connected by a 200-foot bottom width channel to the Little Sunflower ponding area. Construction of the Steele Bayou structure was begun on 22 July 1965 and completed 17 January 1969.

74. Two connecting channels play a vital part in the operation of the proposed plan. One is a 200 foot bottom width channel between the Big and Little Sunflower Rivers. The Little Sunflower River is enlarged between this connecting channel and the Little Sunflower Structure. The other connecting channel is a 200-foot bottom width channel between the Little Sunflower River and Steele Bayou, which also intercepts Deer Creek flow. The purpose of the channel connecting the Sunflower ponding area with the lower and larger Steele Bayou ponding area is to make the most efficient and economical use of the pumping capacity.

75. Little Sunflower structure is located opposite Yazoo River River Mile 32.6, approximately 21 miles northeast of Vicksburg. The structure consists of two vertical lift gates 25.0 by 22.5 feet, concrete-paved approach channel, and a stilling basin. Construction of the structure was completed 28 July 1975.

76. Muddy Bayou control structure is located 13 miles northwest of Vicksburg in the Yazoo Study Area on Muddy Bayou a tributary of Steele Bayou approximately 1,300 feet from its mouth at RM 11.4 of Steele Bayou. The control structure consists of two 20 by 12 foot vertical lift gates the Muddy Bayou Channel (a cutoff dam adjacent to the structure) and an access road from Mississippi Highway 465. The control structure was completed 18 August 1977, controls all water flowing in or out of Eagle Lake through Muddy Bayou, provides flood protection to the Eagle Lake area during periods of moderately high stages (elevation 95.0 feet [NGVD 29]) on Steele Bayou, and provides the means of regulating pool stages in Eagle Lake.

EXISTING PROJECT OPERATION

77. The primary purpose of the Yazoo Backwater Project is to provide flood protection from the Mississippi and Yazoo Rivers to areas in the Lower Mississippi Delta. During periods of high water stages on the Mississippi and Yazoo Rivers, the Steele Bayou and Little Sunflower Structures are closed, necessitating storage of interior drainage within the ponding areas. The interior areas will pond up until the riverside tailwater subsides and the interior water can be released through the floodgates.

78. The Steele Bayou Structure is the principal structure for the Yazoo Backwater Project. Anytime the stage on the landside of the Steele Bayou and Little Sunflower Structures is higher than the riverside and above 70 feet (NGVD 29) the gates are opened. With a rising river, the interior ponding areas are allowed to rise to an elevation of 75.0 feet (NGVD 29). The structures are closed when the river elevation is higher than the interior ponding levels.

79. The Steele Bayou Structure is operated to control minimum water levels in the Steele Bayou and Little Sunflower ponding areas. The present criterion calls for holding minimum water levels in the ponding areas between 68.5 and 70.0 feet (NGVD 29).

80. The interior ponding areas are primarily agricultural and forested lands. Several developed areas exist in the Yazoo Study Area. Although the interior area is protected from the high stages of the Mississippi and Yazoo Rivers, it is subject to flooding resulting from inflow into the ponding areas from Steele Bayou, Deer Creek, Little Sunflower River, and Big Sunflower River.

INTERIOR HYDROLOGIC AND HYDRAULIC ANALYSES

HYDROLOGIC MODEL SETUP

DATA COMPILATION

81. This section describes the data collected and reviewed for this modeling effort, which includes geographic and climatic information, field observations, and previous reports for the Yazoo Study Area.

Streamflow Data

82. The two main sources of stream data used within this modeling effort were from the USGS National Water Information System (NWIS)[1] and the Mississippi Valley Division (MVD) Corps Water Management System (CWMS) database[2]. All data was downloaded as daily average discharges and used to calibrate the HEC-HMS model. The stream gages, identified as inputs or used to calibrate the HEC-HMS model, are listed in Table 2-3.

Table 2-3. Streamflow Gages

ID	Gage Description	Type	Latitude	Longitude
Anguilla*	Sunflower @ Anguilla	Flow***	32° 58’ 19” N	90° 46’ 40” W
Doddsville*	Quiver @ Doddsville	Flow***	33° 38' 25" N	90° 24' 5" W
Grace*	Steele Bayou @ Grace	Flow***	32° 55' 3" N	90° 57' 45" W
Leland**	Bogue Phalia @ Leland	Flow	33° 23' 48" N	90° 50' 51" W
Sunflower**	Sunflower @ Sunflower	Flow	33° 32' 50" N	90° 32' 35" W
Swan Lake**	Tallahatchie @ Swan Lake	Flow	33° 51' 35" N	90° 16' 35" W

*These gages were used as computation points for calibration
**These gages were model inputs
***These flows are based on rating curves at the gage locations

Precipitation Data

83. Precipitation data was collected from gaging stations and gridded precipitation data files. The gaging stations are owned and operated by the National Centers for Environmental Information (NCEI) National Climatic Data Center (NCDC) for the National Oceanic and Atmospheric Administration (NOAA)[3]. The precipitation gages were then used as input for the HEC’s GageInterp program. GageInterp can be used to estimate spatially distributed values of precipitation, temperature, or other parameters. The program reads values from a HEC-DSS file and interpolates between and around those points, at the center of cells in a grid. The program then writes the resulting grids to new records in one or more DSS files. In order for the program to run, the user specifies the input gages as locations given by longitude, latitude, optional elevation, and DSS path names from which the values at the gages will be read, and also specifies the type and extent of the grid to be used. The user can select an interpolation method from several options, and interpolated values may be adjusted by specifying a bias grid, or by using a lapse computation on temperature measurements, based on a user-supplied elevation grid (USACE 2016). For the precipitation data, a Standard Hydrologic Grid (SHG) with a 2,000 meter cell size was chosen. The Inverse distance squared (ID2W) interpolation method was utilized along with a 100,000 meter range. The range sets a maximum distance between the cell center and gage contributing to cell precipitation estimate.

84. Due to the given NCDC precipitation gages having data until the middle of 2013, a Stage IV precipitation grid was used from January 2013 through December 2019. This Stage IV grid is produced by the University Corporation for Atmospheric Research (UCAR)[4]. Table 2-4

[1] https://waterdata.usgs.gov/nwis
[2] https://www.mvk-wc.usace.army.mil/watercontrol.html
[3] https://www.ncdc.noaa.gov/cdo-web/
[4] https://data.eol.ucar.edu/dataset/21.093

identifies the precipitation stations and Figure 2-38 locates the precipitation stations within the Yazoo River watershed.

Table 2-4. Precipitation Gages

State/County	Gage Description	Latitude	Longitude
MS Desoto	Arkabutla Dam*	34° 45’ 0” N	90° 8’ 0” W
MS Marshall	Byhalia*	34° 52’ 0” N	89° 41’ 0” W
MS Coahoma	Clarksdale	34° 12’ 0” N	90° 34’ 0” W
MS Bolivar	Cleveland	33° 51’ 46” N	90° 6’ 12” W
AR Desha	Dumas*	33° 53’ 19” N	91° 31’ 54” W
LA West Carroll	Epps*	32° 36’ 14” N	91° 28’ 40” W
MS Leflore	Greenwood*	33° 31’ 0” N	90° 10’ 0” W
MS Carroll	Greenwood AP*	33° 30’ 0” N	90° 5’ 0” W
MS Grenada	Grenada Dam*	33° 48’ 0” N	89° 46’ 0” W
MS Rankin	Jackson Int. AP*	32° 18’ 52” N	90° 4’ 43” W
MS Holmes	Lexington*	33° 7’ 0” N	90° 3’ 0” W
AR Drew	Monticello*	33° 38’ 3” N	91° 45’ 17” W
MS Marshall	Mount Pleasant*	34° 54’ 20” N	89° 33’ 43” W
MS Lafayette	Oxford*	34° 23’ 0” N	89° 32’ 0” W
AR Jefferson	Pine Bluff*	34° 15’ 0” N	92° 0’ 0” W
MS Sharkey	Rolling Fork	32° 55’ 0” N	90° 52’ 0” W
MS Panola	Sardis Dam*	34° 24’ 0” N	89° 47’ 25” W
MS Washington	Stoneville	33° 25’ 0” N	90° 55’ 0” W
AR Arkansas	Stuttgart*	34° 29’ 0” N	91° 32’ 0” W
LA Madison	Tallulah*	32° 20’ 53” N	91° 1’ 48” W
MS Warren	Vicksburg*	32° 23’ 0” N	90° 52’ 0” W
MS Yazoo	Yazoo City*	32° 51’ 0” N	90° 26’ 0” W

*These gages are outside the Yazoo Study Area boundary but are used in the precipitation grid

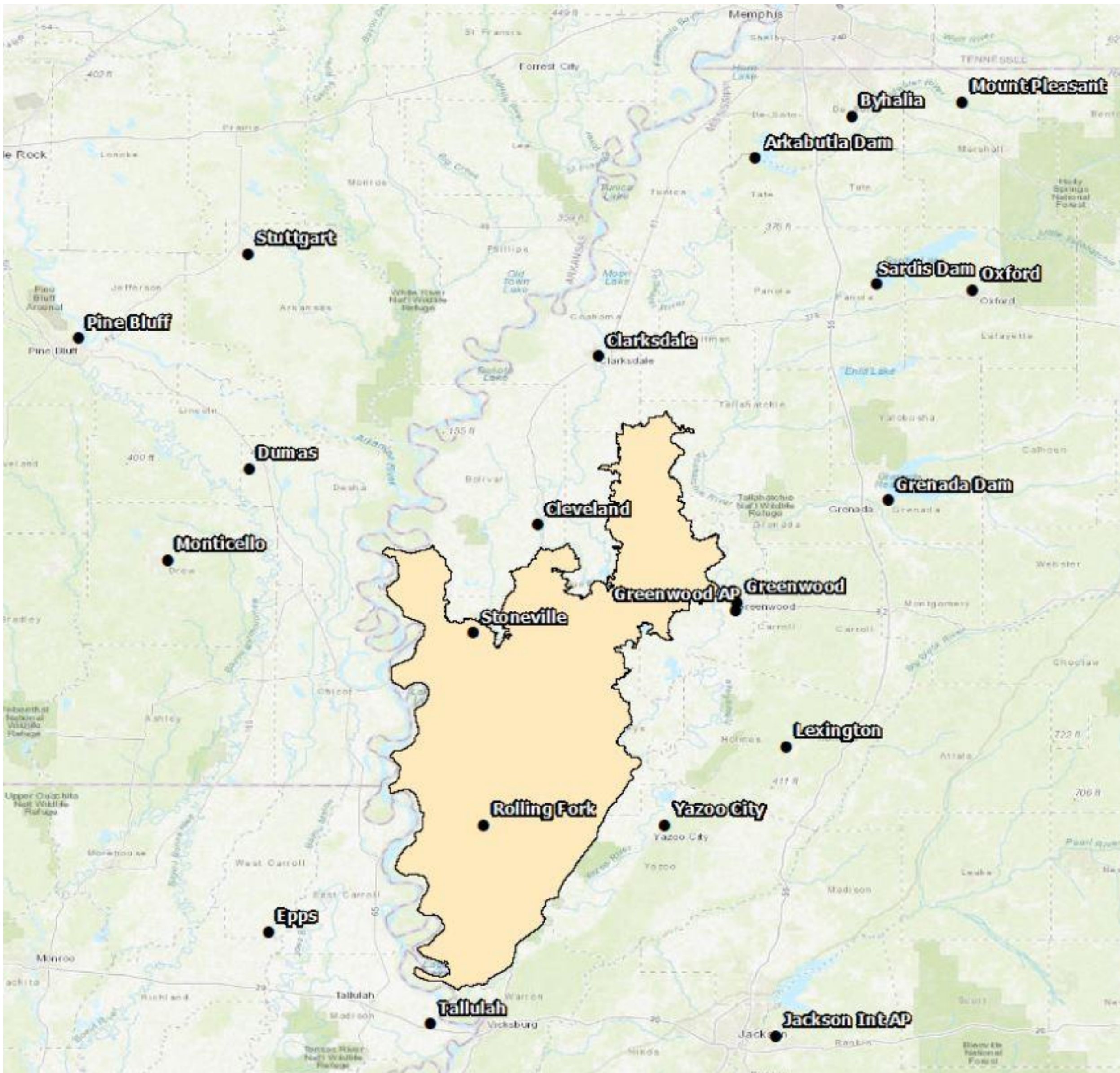

Figure 2-38. The precipitation gages within the Yazoo River watershed.

Temperature Data

85. Temperature data that was used within this modeling effort was also generated from the HEC GageInterp program. The period-of-record was retrieved from the NOAA Climate Data Online (CDO)[5]. The maximum and minimum temperature were used to calculate the average temperature, and then the average temperature HEC-DSS file was used in GageInterp to generate a spatially interpolated gridset. Within the GageInpterp program, the temperature grid was a SHG with a 2,000-meter cell size. The inverse distance (IDW) interpolation method was chosen with an unlimited range of temperature gage influence.

[5] https://www.ncdc.noaa.gov/cdo-web/

SOFTWARE AND DOCUMENTATION

86. Table 2-5 provides a summary of the computer programs and versions used in development of the HEC-HMS model.

Table 2-5. Computer Programs Utilized

Program	Version	Capability	Developer
ArcGIS	10.4.1	Geographical Information System	ESRI
HEC-DSSVue	3.0	Plot, tabulate, edit, and manipulate data in HEC-DSS files	HEC
HEC-HMS	4.4.1	Rainfall-runoff simulation	HEC
HEC GageInterp	1.6	Create a sequence of HEC-DSS grids from time-series measurements	HEC

HEC-HMS MODEL DEVELOPMENT

87. To develop a continuous simulation model that computed volumetric flow rates necessary for use in the Yazoo Study Area over a 42-year period, a hydrologic model was needed. HEC-HMS 4.4.1 was used to convert precipitation and temperature into runoff. The following sections detail model-specific processes that were used to create and calibrate the HEC-HMS model.

Status of the Vicksburg District's Existing HEC-HMS Model(s)

88. The USACE Vicksburg District had a completed HEC-HMS model for the Yazoo River watershed, which includes the Yazoo Study Area. This model was used as a basis for the new Yazoo Study Area HEC-HMS model. The original Yazoo River watershed covered a total area of 13,480 square miles and consisted of 110 subbasins. The model domain was reduced to only 2,687 square miles and thirteen subbasins for this study. The Yazoo River CWMS and Yazoo Study Area are shown in Figure 2-39.

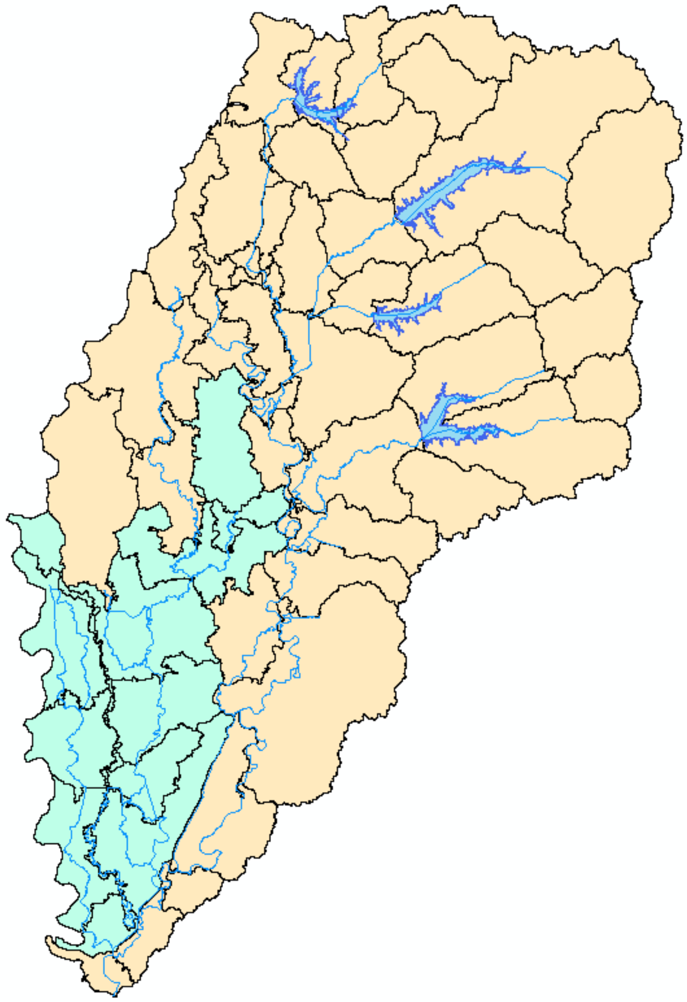

Figure 2-39. Yazoo River CWMS and Yazoo Study Area Comparison

<u>Yazoo Study Area</u>

89. A list of subbasins and their sizes can be found in Table 2-6.

Table 2-6. Subbasin Summary

Subbasin Name	Area (sq. mi.)
SF Doddsville Loc	258
SF QuiverSunflower	81
SF BigSunatQuiver	302
SF LittleCalleo	379
SF Anguilla Loc	268
DB DeerCreekN	113
SF HollyBluff	150
SF LittleSunflower	331
DB DeerCreekS	28
SB Longwood	259
SB SteeleGrace	224
SB MuddyBayou	212
SB SteeleMouth	82

Precipitation

90. A gridded precipitation file was initially used to estimate rainfall in the HEC-HMS model. Once the initial 42-year simulation was run, the output HEC-DSS file included hourly precipitation that was associated with each subbasin. In order to cut down on run times, the hourly precipitation from the gridded precipitation run was converted to specified hyetographs at each subbasin. These hyetographs were linked to their respective precipitation gages from the output of the gridded precipitation run.

Evapotranspiration

91. A modified, gridded version of the Hamon method was used initially to estimate potential evapotranspiration (ET) losses using the previously mentioned daily average temperature gridset and a coefficient (Harwell 2012). The output from the gridded Hamon method consisted of HEC-DSS files that had the average temperature associated with each subbasin. Later, the Hamon method was utilized by linking temperature gages for each subbasin to the HEC-DSS file that had the gridded Hamon output. The Hamon method was used to simulate evapotranspiration (ET) losses throughout the model. Within the Hamon method, ET losses are directly proportional to the daily average temperature and related to the location of interest and time of year (Hamon 1961). The Hamon coefficient for the Yazoo Study Area was set as the default of 0.0065.

Infiltration

92. Infiltration computations were executed using the Deficit and Constant Loss method. Initial estimates of initial deficit, maximum deficit, and constant loss rate were based upon surficial soil texture estimates done in the Yazoo River CWMS model. These textures were acquired from the NRCS gSSURGO soil coverage. These values were later set to a similar range for consistency across all subbasins for the 42-year period.

Unit Hydrograph Transform

93. The modified Clark (ModClark) unit hydrograph transform was used to route excess precipitation to the subbasin outlet within each subbasin. This linear, quasi-distributed transform method uses a set of grid cells to represent travel times within a subbasin to the outlet point. As such, it explicitly accounts for variations in travel time from all areas within a subbasin through the use of a time travel index for each grid cell. As previously stated, these grid cells were laid out using the Standard Hydrologic Grid (SHG) system with a 2,000-meter by 2,000-meter resolution and then placed over the modeling domain using tools available through HEC.

94. The Yazoo Study Area HEC-HMS model stayed fairly consistent with the original estimates from the Yazoo River CWMS model. These initial estimates were conducted using the TR55 method in HEC- GeoHMS and the Travel Time Tool (TTT) in ArcGIS.

95. Much like in the Yazoo River CWMS model, the time of concentration (Tc) and storage coefficient (R) values were adjusted as necessary to calibrate at stream gages.

Baseflow

96. The Linear Reservoir method was used to transform water which was infiltrated into interflow and baseflow and add these components to any direct runoff generated within each subbasin. For this modeling effort, the storage and movement of infiltrated water was simulated using two layers. The layers are considered “linear” due to the fact that the outflow at each time step of the simulation is a linear function of the average storage during the time step. Due to the use of the Deficit and Constant Loss method, the volume of infiltrated water was evenly divided between the two layers. The resultant outflow from both layers was combined to compute the total baseflow for each subbasin. Finally, within this method, only infiltrated water is available, which allows for mass to be conserved. This was essential due to the long simulation windows used during model calibration.

97. The two baseflow layers were conceptualized to differentiate between short and long baseflow responses; the upper layer was parameterized to respond faster than the lower layer. Initial parameter estimates of a storage coefficient for both layers were based upon the previously mentioned unit hydrograph transform parameters. Initially, the groundwater “one” storage coefficient was set to two times the subbasin ModClark storage coefficient and the groundwater “two” coefficient was set to one hundred times the groundwater “one” storage coefficient. This was done in an effort to preserve the expected physical relationships between subbasin size, slope, land use, and geology (amongst other factors) when estimating the movement of water as baseflow. Lastly, the number of reservoirs was initially set to one in both layers. These values were adjusted during the calibration phase to calibrate at stream gages.

Streamflow Routing

98. The routing methods used in the Yazoo River CWMS model were also used in the Yazoo Study Area model. The two methods used were Lag and Modified Puls routing. The Lag routing was kept consistent with the Yazoo River CWMS model as well as most the Modified Puls reaches. However, a few Modified Puls reaches were modified to simulate more attenuation on the Big Sunflower River. The routing reaches used within the HEC-HMS model are detailed in Table 2-7 below.

Table 2-7. Routing Reach Summary

Name	Method*
SFR QuivDodd_QuivSun	L
SFR QuiverSun_BigSun	L
SFR SunSunfl_SunQuiv	M-P
SFR SunQuiv_HollyInd	M-P
SFR HollyInd_BPhalia	M-P
SFR Leland_BPhalia	M-P
SFR BPhalia_LCallio	M-P
SFR LCallio_AnguiGag	M-P
SFR AnguiGag_AnguMth	M-P
SFR AnguMth_SunLow	L
DBR DeerN_LSunfl	L
SFR LittleSun_BigSun	L
SFR SunHollyB_East	L
SFR SunHollyE_LtlSun	L
SFR SunLSun_SFCntrl	L
SFR SFCntrl_DBDeer	L
SBR SBSteeleConnect	L
SBR BlkLong_SBGrace	M-P
SBR SBOtter_SBOnward	M-P
SBR SBOnward_SBMuddy	M-P
SBR SBMuddy_SBCntrl	L

*M-P = Modified Puls, L = Lag

Diversions

99. There were several diversions used in the Yazoo River CWMS HEC-HMS model. However, the diversions were removed from the Yazoo Study Area HEC-HMS model for simplicity as the breakouts would not significantly affect the timing component. The Swan Lake diversion was added into the model as a source because it directly adds flow into the system. The flow was calculated based on a diversion rating curve; flows greater than 7,500 cfs at Tallahatchie River at Swan Lake begin to cross basins to the Quiver River basin thus entering the Yazoo Study Area.

Precipitation-Runoff Calibration/Validation

100. Multiple years were chosen ranging from high precipitation years to low precipitation years in order to determine one set of parameters to represent conditions over the 42-year simulation. These years include:

a. Calibration Events

(1). 1991 – High Precipitation

(2). 2004 – Average Precipitation

(3). 2011 – Low Precipitation

(4). 2019 – High Precipitation

b. Validation Events

(1). 1983 - High Precipitation

(2). 1997 – Average Precipitation

(3). 2005 – Low Precipitation

(4). 2010 – Low Precipitation

Calibration/Validation Parameters and Approach

101. Table 2-8 shows the calibration and validation parameter and approach.

Table 2-8. Calibration and Validation Parameters and Approach

Process	Parameter	Calibration/Validation Approach
Evapotranspiration	Hamon Coefficient	This parameter is used by the Hamon routine to compute the amount of potential ET. This parameter was not varied from the default during model calibration.
	Crop Coefficient	This parameter is specified for each subbasin and is used to adjust the amount of potential ET at a subbasin-scale. The Dynamic Canopy method was used to allow a variable crop coefficient. This parameter was decreased by 0.5 times the calibrated crop coefficient and was increased up to a maximum of 1.5 depending upon the vegetative cover and/or the amount of active irrigation within each subbasin.
Infiltration	Initial Deficit	This parameter is event specific and represents the moisture conditions in the watershed at the beginning of a simulation. This parameter has very little impact on a continuous simulation as the model "warms up" after simulating the first couple of events.
	Maximum Deficit	This parameter sets an upper limit to the moisture deficit. This parameter was adjusted during calibration to three or four inches across all subbasins.
	Constant Loss Rate	The constant loss rate represents the basin average infiltration rate when the soil has reached a saturated state. This parameter varied from the Yazoo River CWMS model in that a range of 0.2 to 0.3 was chosen to represent the subbasins.
	Percent Impervious Cover	The percent impervious area parameter represents the percentage of the watershed where impervious land is directly connected to the stream network. This parameter was not varied from the Yazoo River CWMS model during model calibration.
Runoff Transform	Time of Concentration (T_c)	This parameter was varied slightly from the original Yazoo River CWMS estimates. The changes were to better match the unit hydrographs at stream gages.
	Storage Coefficient (R)	This parameter was set to two times the time of concentration across each subbasin.
Baseflow	GW 1 Initial Discharge	The initial discharge represents the flow rate contribution from ground water 1 at the beginning of the simulation. Initial discharge from GW 1 was set to zero.
	GW 1 Fraction	This parameter determines how the percolation is split to the reservoirs. In this case, it is how much of that percolation goes into the GW 1 reservoir. The fraction must be greater than zero and less than or equal to one. When the sum of the fractions is exactly one then there will be no aquifer recharge. When the sum is less than one, the remainder of the percolation becomes aquifer recharge.
	GW 1 Storage Coefficient	GW 1 was conceptualized to represent the fast responding portion of baseflow. Therefore, this coefficient was set to a smaller value than the GW 2 storage coefficient. This value was altered to best match the observed hydrograph shape and flow volumes. Efforts were made to develop a single value or acceptable range for each subbasin and/or zone regardless of the time of year.

Table 2-8. (Cont.) Calibration and Validation Parameters and Approach.

Process	Parameter	Calibration/Validation Approach
	GW 1 # of Reservoirs	This parameter sets the number of linear reservoirs within layer 1 which directly affects the attenuation and timing of computed runoff. This parameter was set to 3 reservoirs during model calibration.
	GW 2 Initial Discharge	Initial discharge is event specific and can vary throughout the year within a single subbasin. Therefore, this parameter was set to 0.1 cfs/sq. mi to match the initial flow at the beginning of each simulation.
	GW 2 Fraction	This parameter determines how the percolation is split to the reservoirs. In this case, it is how much of that percolation goes into the GW 1 reservoir. The fraction must be greater than zero and less than or equal to one. When the sum of the fractions is exactly one then there will be no aquifer recharge. When the sum is less than one, the remainder of the percolation becomes aquifer recharge.
	GW 2 Storage Coefficient	GW 2 was conceptualized to represent the slow responding portion of baseflow. Therefore, this coefficient was set to a larger value than the GW 1 storage coefficient. This value was altered to best match the observed hydrograph shape and flow volumes. Efforts were made to develop a single value or acceptable range for each subbasin and/or zone regardless of the time of year.
	GW 2 # of Reservoirs	This parameter sets the number of linear reservoirs within layer 2 which directly affects the attenuation and timing of computed runoff. This parameter was set to 3 reservoirs during model calibration.
Streamflow Routing	Lag Time	This parameter was not varied during model calibration.
	Storage-Discharge Function	This parameter was adjusted because preliminary results showed reach routing needed more attenuation in the Big Sunflower River. These adjustments were needed because the HEC-RAS model used storage areas for the overbank area, water in the overbank was not accounted for when storage-discharge information was originally computed in the HEC-RAS model.
	Number of Subreaches	This parameter was not varied during model calibration.

<u>Final Parameters</u>

102. After completing the calibration for the previously mentioned years, efforts were made to come up with a single parameter set to represent the 42-year continuous simulation. Once a single parameter set was chosen, several validation events were run. This would turn out to be an iterative process, and the parameters were adjusted until there was a comfortable balance between the calibration and validation results. In the following tables (Table 2-9, Table 2-10, Table 2-11, and Table 2-12), the final model parameters for evapotranspiration, infiltration, unit hydrograph transform, and baseflow are represented.

Table 2-9. Evapotranspiration (Dynamic Canopy)

Subbasin	Initial Storage (%)	Max Storage (IN)	Crop Method	Crop Gage
SF Doddsville Loc	0	0.01	Time-Series Gage	SF Doddsville Loc
SF QuiverSunflower	0	0.01	Time-Series Gage	SF QuiverSunflower
SF BigSunatQuiver	0	0.01	Time-Series Gage	SF BigSunatQuiver
SF LittleCalleo	0	0.01	Time-Series Gage	SF LittleCalleo
SF Anguilla Loc	0	0.01	Time-Series Gage	SF Anguilla Loc
DB DeerCreekN	0	0.01	Time-Series Gage	DB DeerCreekN
SF HollyBluff	0	0.01	Time-Series Gage	SF HollyBluff
SF LittleSunflower	0	0.01	Time-Series Gage	SF LittleSunflower
DB DeerCreekS	0	0.01	Time-Series Gage	DB DeerCreekS
SB Longwood	0	0.01	Time-Series Gage	SB Longwood
SB SteeleGrace	0	0.01	Time-Series Gage	SB SteeleGrace
SB MuddyBayou	0	0.01	Time-Series Gage	SB MuddyBayou
SB SteeleMouth	0	0.01	Time-Series Gage	SB SteeleMouth

Table 2-10. Infiltration (Deficit and Constant)

Subbasin	Initial Deficit (IN)	Maximum Storage (IN)	Constant Rate (IN/HR)	Impervious (%)
SF Doddsville Loc	0	4	0.2	18.6
SF QuiverSunflower	0	3	0.2	4.6
SF BigSunatQuiver	0	3	0.2	3
SF LittleCalleo	0	3	0.2	21.9
SF Anguilla Loc	0	3	0.3	11.4
DB DeerCreekN	0	4	0.2	2.9
SF HollyBluff	0	4	0.2	1.4
SF LittleSunflower	0	4	0.3	3.3
DB DeerCreekS	0	4	0.2	3.1
SB Longwood	0	3	0.2	4.9
SB SteeleGrace	0	3	0.3	5.6
SB MuddyBayou	0	4	0.3	2
SB SteeleMouth	0	4	0.3	3.2

Table 2-11. Transform (ModClark)

Subbasin	Time of Concentration (HR)	Storage Coefficient (HR)
SF Doddsville Loc	60	120
SF QuiverSunflower	60	120
SF BigSunatQuiver	75	150
SF LittleCalleo	50	100
SF Anguilla Loc	115	230
DB DeerCreekN	175	350
SF HollyBluff	30	60
SF LittleSunflower	15	30
DB DeerCreekS	75	150
SB Longwood	50	100
SB SteeleGrace	50	100
SB MuddyBayou	50	100
SB SteeleMouth	25	50

Table 2-12. Baseflow (Linear Reservoir)

Subbasin	GW1				GW2			
	Initial Q (cfs)	Fraction	Coeff (hrs)	# of Res	Initial Q (cfs)	Fraction	Coeff (hrs)	# of Res
SF Doddsville Loc	0	0.95	60	3	0.1	0.05	600	3
SF QuiverSunflower	0	0.8	120	3	0.1	0.2	1200	3
SF BigSunatQuiver	0	0.7	150	3	0.1	0.2	1500	3
SF LittleCalleo	0	0.7	100	3	0.1	0.2	1000	3
SF Anguilla Loc	0	0.7	115	3	0.1	0.2	1150	3
DB DeerCreekN	0	0.4	350	3	0.1	0.05	3500	3
SF HollyBluff	0	0.4	60	3	0.1	0.05	600	3
SF LittleSunflower	0	0.4	30	3	0.1	0.05	300	3
DB DeerCreekS	0	0.4	150	3	0.1	0.05	1500	3
SB Longwood	0	0.4	30	3	0.1	0.3	300	3
SB SteeleGrace	0	0.5	60	3	0.1	0.2	600	3
SB MuddyBayou	0	0.4	150	3	0.1	0.05	1500	3
SB SteeleMouth	0	0.4	50	3	0.1	0.05	500	3

<u>HEC-HMS Model Metrics</u>

103. Model performance was evaluated by comparing computed results against observed results at numerous locations. Model parameters were altered to minimize the differences between computed and observed discharge at each streamflow gage. When available, summary statistics were used to quantify model performance compared to observations (Moriasi et al. 2007).

Statistics include Nash-Sutcliffe Efficiency (NSE), Ratio of the Root Mean Square Error to the Standard Deviation Ratio (RSR), and Percent Bias (PBIAS).

104. NSE measures the relative magnitude of the residual variance compared to the measured data variance. NSE ranges between negative infinity and one, where an NSE equal to one is optimal. Values of NSE less than or equal to zero indicate the mean observed value is a better predictor than the simulated value. NSE is computed using the following equation:

$$NSE = 1 - \left[\frac{\Sigma_{i=1}^{n}(Y_i^{obs} - Y_i^{sim})^2}{\Sigma_{i=1}^{n}(Y_i^{obs} - \bar{Y}^{obs})^2}\right]$$

105. where n is the number of observed values compared to computed over the duration of the simulation, Y_i^{obs} is the observed values, Y_i^{sim} is the computed values, and $\bar{Y}^{obs}$ is the average of observed values.

106. RSR normalizes the root mean square error by using the standard deviation of the observations, incorporating the benefits of error index statistics so that the resulting statistic can be applied to various constituents. RSR is computed using the following equation:

$$RSR = \frac{RSME}{STDEV_{obs}} = \frac{\left[\sqrt{\Sigma_{i=1}^{n}(Y_i^{obs} - Y_i^{sim})^2}\right]}{\left[\sqrt{\Sigma_{i=1}^{n}(Y_i^{obs} - \bar{Y}_i^{sim})^2}\right]}$$

107. where RSME is the root mean square error, STDEVobs is the standard deviation of the observations, and $\bar{Y}_i^{sim}$ is the average of simulated values.

108. PBIAS measures the average tendency of the simulated data to be larger or smaller than the observed data. The optimal value for PBIAS is zero, with low absolute PBIAS indicating accurate model simulation. PBIAS is computed using the following equation:

$$PBIAS = \left[\frac{\Sigma_{i=1}^{n}(Y_i^{obs} - Y_i^{sim}) \times 100}{\Sigma_{i=1}^{n}(Y_i^{obs})}\right]$$

109. Summary statistic performance ratings are presented in Table 2-13.

Table 2-13. Performance Rating for Summary Statistics

Performance Rating	**NSE**	**RSR**	**PBIAS**
Very Good	$0.65<NSE\leq1.00$	$0.00<RSR\leq0.60$	$PBIAS< \pm15$
Good	$0.55<NSE\leq0.65$	$0.60<RSR\leq0.70$	$\pm15\leq PBIAS<\pm20$
Satisfactory	$0.40<NSE\leq0.55$	$0.70<RSR\leq0.80$	$\pm20\leq PBIAS<\pm30$
Unsatisfactory	$NSE\leq0.40$	$RSR>0.80$	$PBIAS\geq\pm30$

Model Results and Performance

110.The section below shows the model results from the preliminary submission of the model. While the HEC-HMS (hydrologic) model was used as inputs in the HEC-RAS (hydraulic) model, the hydraulic model results took precedent over the hydrologic model results. The Big Sunflower River at Anguilla and Steele Bayou at Grace observed flow data were developed from a backwater rating curve. Due to the complexity and uncertainty of a backwater rating, these two gage locations were primarily used as a visual check to calibrate the shape of the hydrograph. Furthermore, these two gages are the primary source of available flow data within the backwater area. With that in mind, the HEC-HMS model calibration contained more uncertainty, and thus more emphasis was placed on the HEC-RAS results, especially considering HEC-RAS results produced stage data which was easily checked with observed stage data at multiple locations. The results from the hydraulic model showed that the computed volume at Steele Bayou control structure was closer to the observed volume with the hydrologic model results shown in this section. With that being said, edits have already been made to the HEC-HMS model to improve results at the computation points.

111.Figure 2-40 through Figure 2-46 show several calibration/validation events for the stream gages that the model was calibrated to. All of the calibration/validation events are not shown due to the fact that the model is being judged on an overall performance for the 42-year simulation. However, these figures demonstrate the uncertainties within the model; including uncertainties in the boundary conditions and process parameters defined in HEC-HMS.

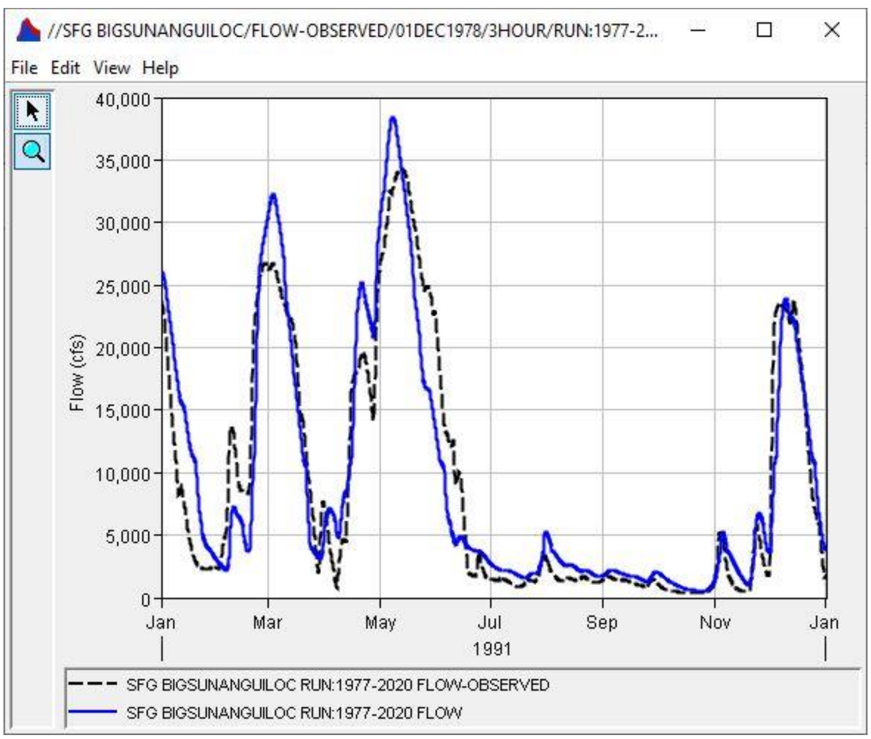

Figure 2-40. Big Sunflower River at Anguilla – 1991.

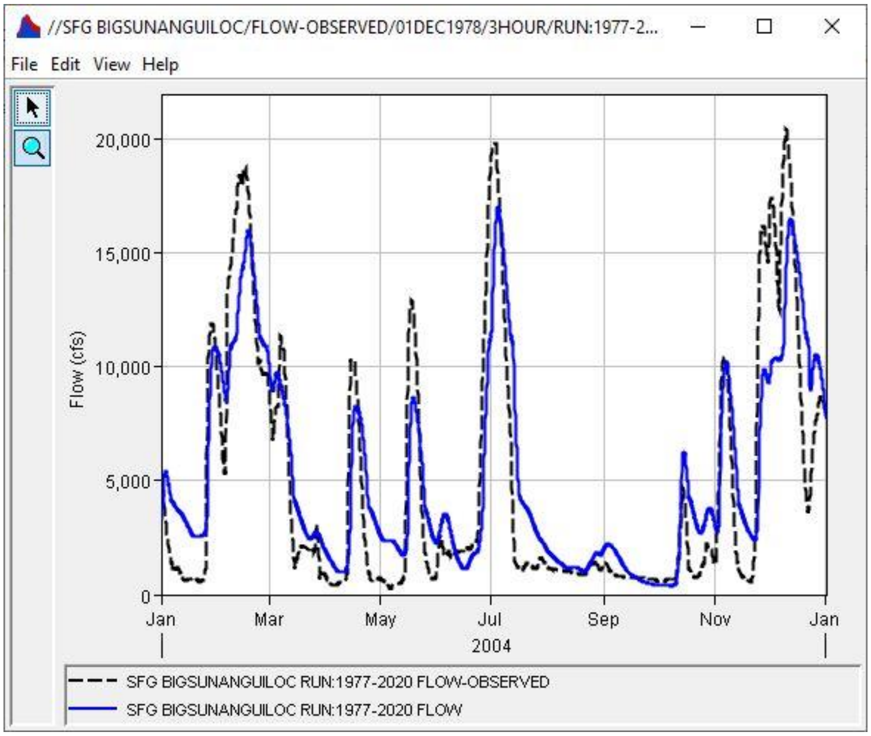

Figure 2-41. Big Sunflower River at Anguilla – 2004.

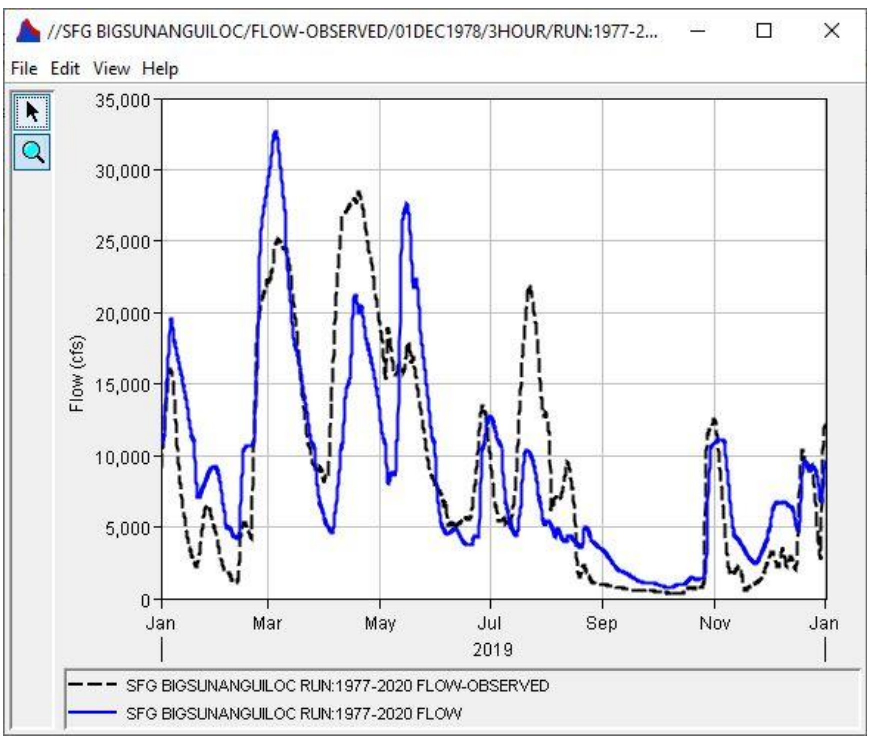

Figure 2-42. Big Sunflower River at Anguilla – 2019.

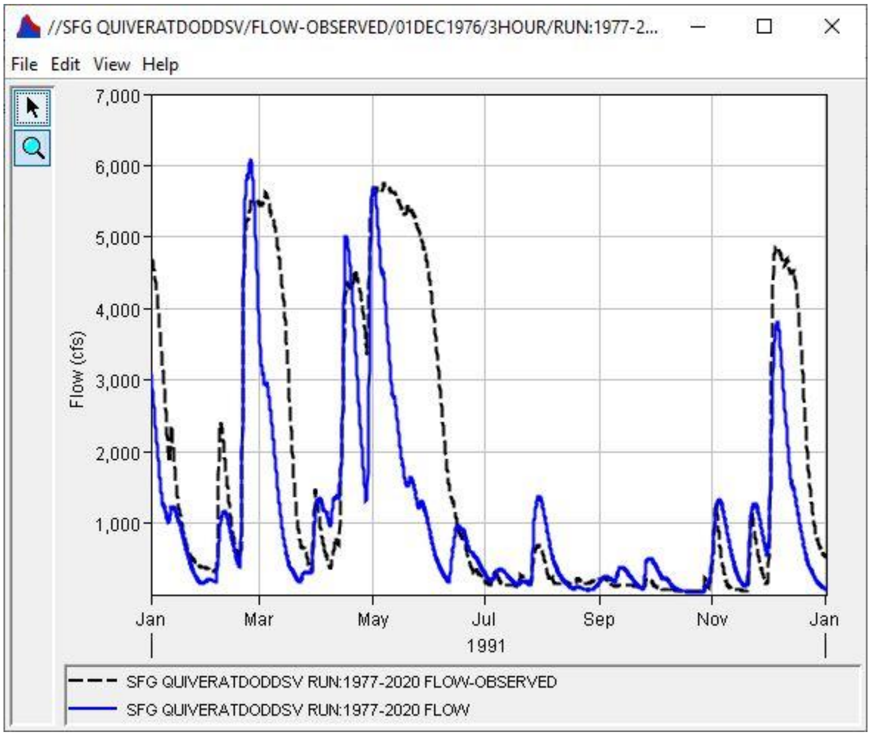

Figure 2-43. Quiver River at Doddsville – 1991.

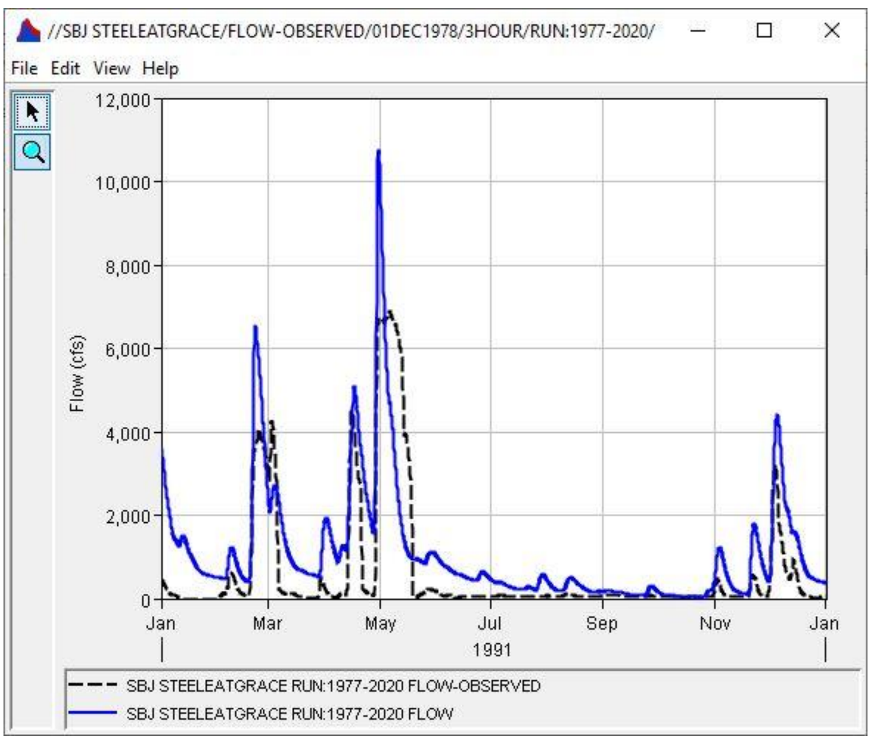

Figure 2-44. Steele Bayou at Grace – 1991.

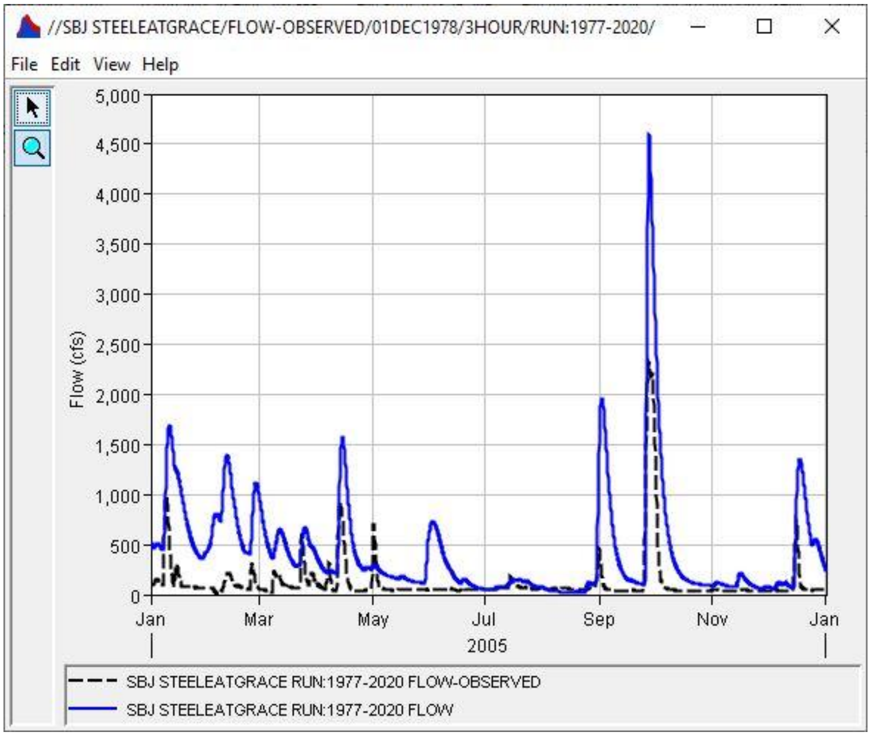

Figure 2-45. Steele Bayou at Grace – 2005.

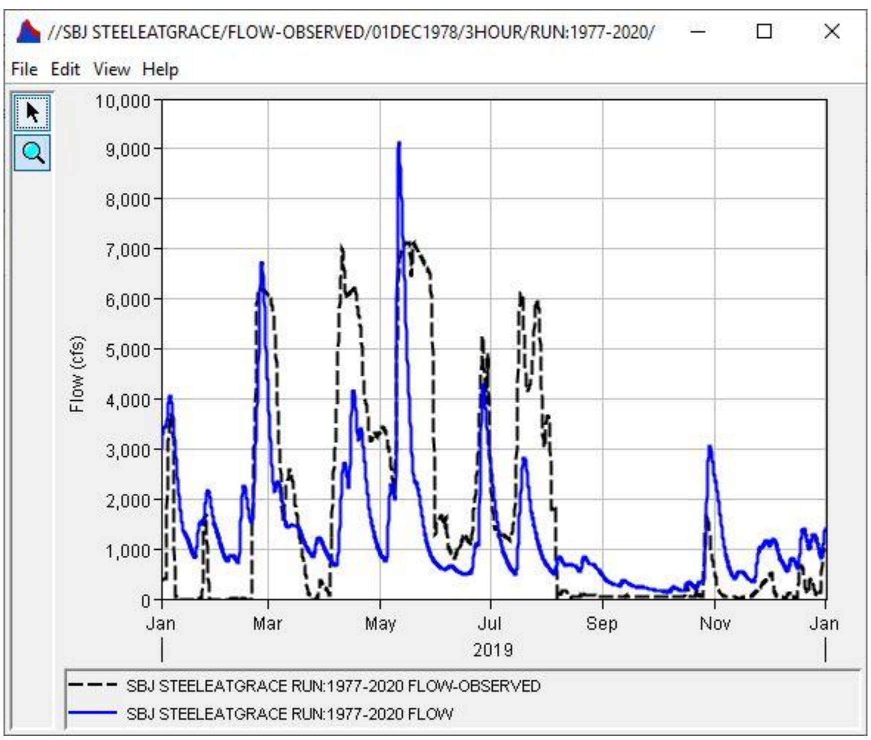

Figure 2-46. Steele Bayou at Grace – 2019.

112.Figure 2-47 through Figure 2-49 shows the average computed monthly flows compared against the average observed monthly flows at the three computation points for the 42-year period. The monthly plots help demonstrate the volumetric water balance throughout the year. While the model can more effectively capture flows for certain years compared to other years, the average monthly flows help to balance out model performance over the 42-year period. The figures shown below display that, in general, the average computed monthly flows is higher than the average observed monthly flows. As previously stated, changes to the model have already been made to eliminate bias from the HEC-HMS model. These modified results will be shown in a later section.

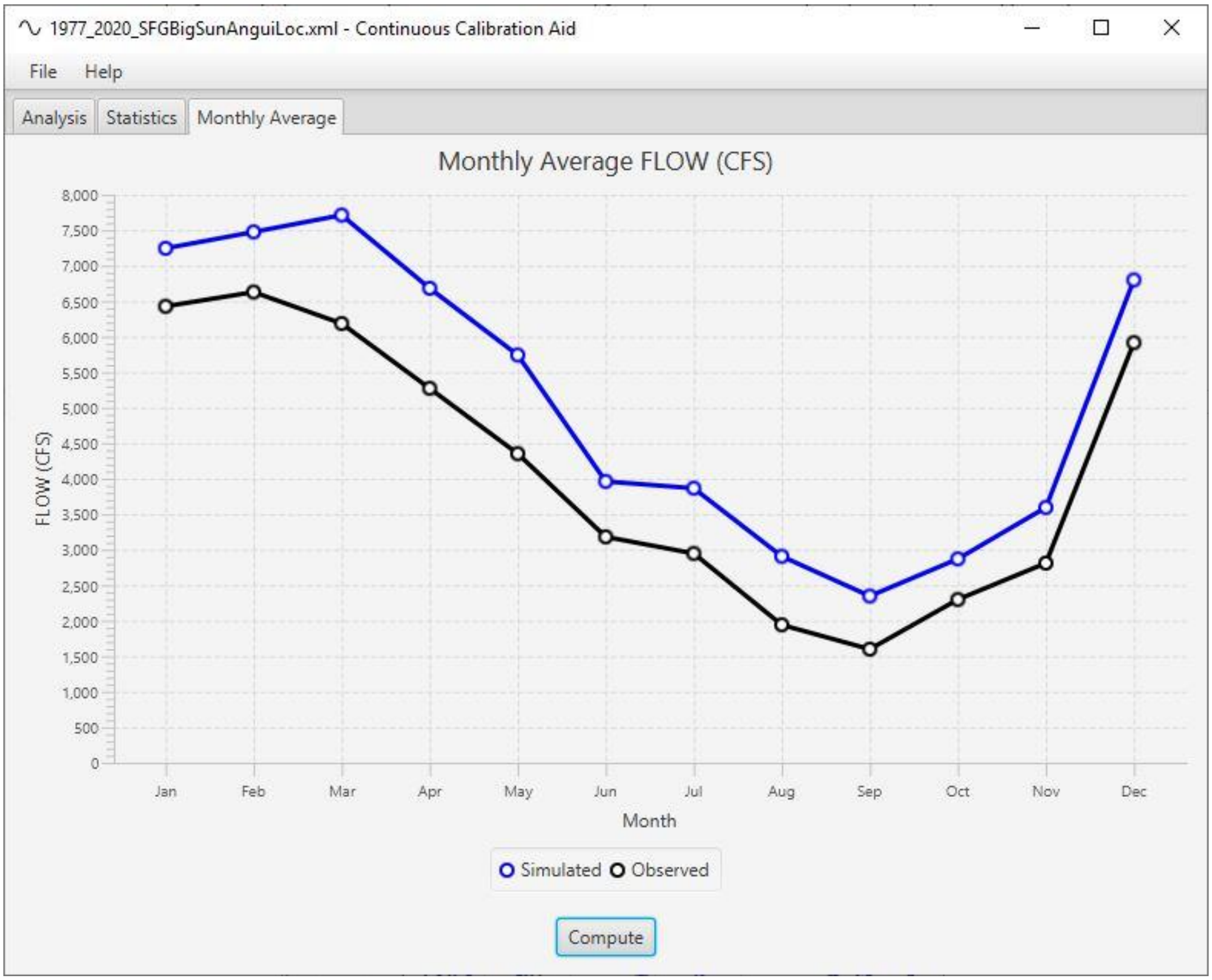

Figure 2-47. Big Sunflower River at Anguilla Monthly Flow Comparison.

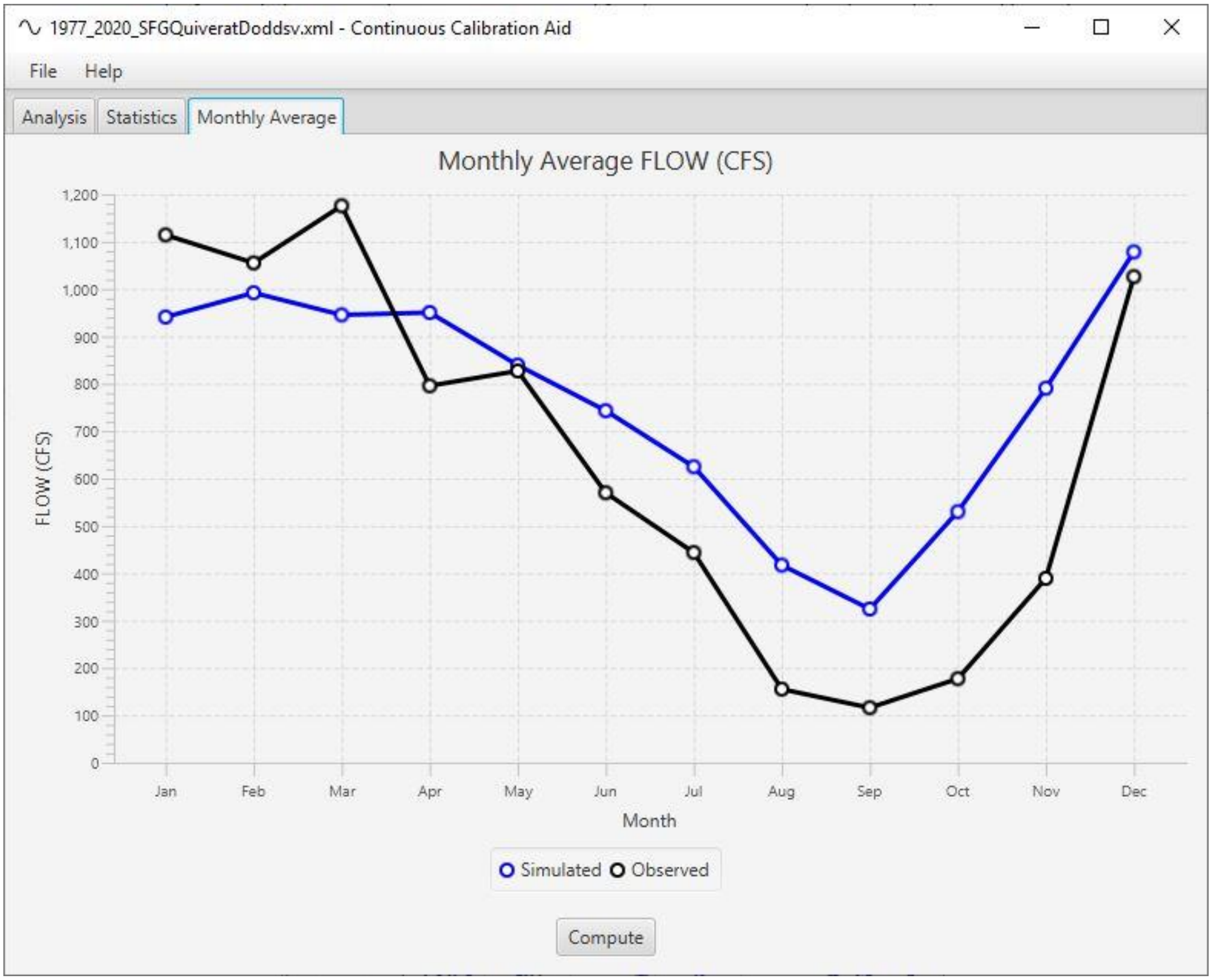

Figure 2-48. Quiver River at Doddsville Monthly Flow Comparison.

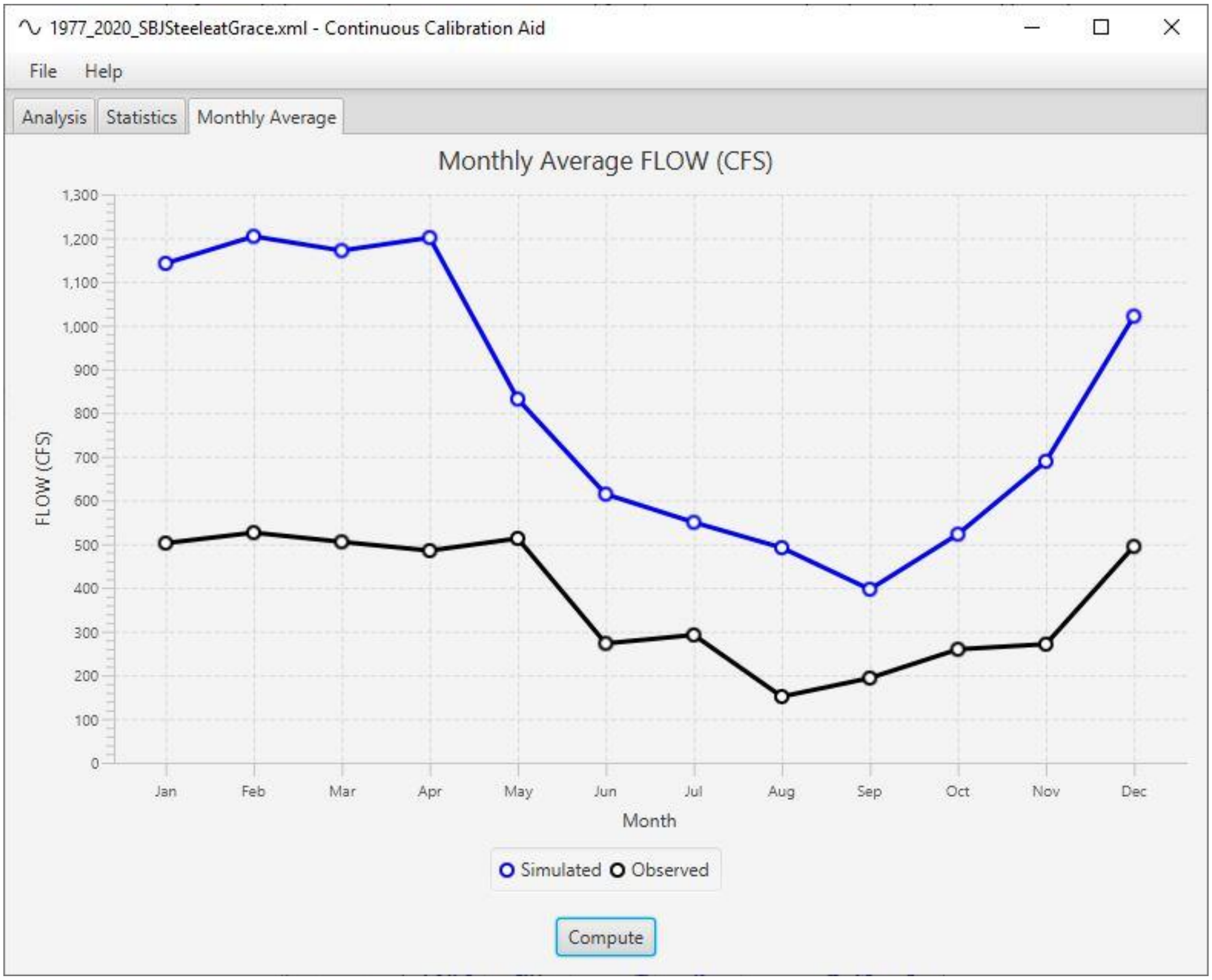

Figure 2-49. Steele Bayou at Grace Monthly Flow Comparison.

113. In Table 2-14, the model performance at each computation point is shown for the 42-year simulation. The performance ratings table can be found in the 'HEC-HMS Model Metrics' section above.

Table 2-14. Model Performance at Computation Points for Forty-Three Year Simulation

Computation Point	NSE	RSR	PBIAS	R2
Anguilla	0.70	0.55	-23.56	0.74
Doddsville	0.55	0.67	-17.10	0.56
Grace	0.01	1.00	-120.02	0.43

114. Based on Table 2-14, the Big Sunflower River at Anguilla had a performance rating of 'very good', the Quiver River at Doddsville had a performance rating of 'good' (the Quiver River at Doddsville only had data from 1997 to 1998), and Steele Bayou at Grace had a performance rating of 'unsatisfactory'. While Steele Bayou at Grace had an 'unsatisfactory' rating, it should

be noted that Steele Bayou at Grace only represents a small portion of the model so the model results should not completely be thrown out due to the poor performance at one computation point. Any computation point could have not performed as well as it should have due to uncertainties within the model. As stated before, in the monthly flow comparison figure, it is noticeable that the average computed monthly flows are higher than the average observed monthly flows. Another reason for the substandard performance is the uncertainty with the precipitation grid. Efforts were made to incorporate a scaled version of the USGS Soil Water Balance (SWB) model that used DayMet precipitation data; however, there was not enough confidence to use this method. Also, many different iterations were ran in GageInterp that used different precipitation gages, interpolation methods, and ranges to come up with the best precipitation grid from the given data.

Conclusions and Recommendations for Future Improvements

115. A HEC-HMS model was developed for the Yazoo Study Area for a 42-year period. The model utilized continuous simulation. Several calibration/validation events were chosen in order to come up with a single parameter set to represent the simulation window. Multiple statistical metrics were used to determine the model performance. Overall, the model performed well with the exception of Steele Bayou at Grace. Although Steele Bayou at Grace had an ‘unsatisfactory’ performance, this gage only represents a small portion of the watershed compared to the Big Sunflower River at Anguilla.

116. Several recommendations for future improvements to the Yazoo Study Area HEC-HMS model are provided below:

a. Develop or locate a more consistent precipitation dataset.

b. Reduce the baseflow in the streams while maintaining the peak flows through a reduction of the groundwater “one” coefficient and/or a reduction in the ModClark storage coefficient.

c. Integrate the gain/loss method for routing reaches to account for the flow loss.

d. Incorporate ‘Save States’ in HEC-HMS that would allow for the model to be calibrated to each individual year.

IMPROVED HEC-HMS MODEL RESULTS

117. Figure 2-50 through Figure 2-52 shows some of the events described in the previous model results. It should be noted that the computed flows are reduced to better match those of the observed flows. This model was not chosen because the flows match the lower flows well, instead the model was chosen because the model was overall low on the higher flow peaks in both the HEC-HMS and HEC-RAS models.

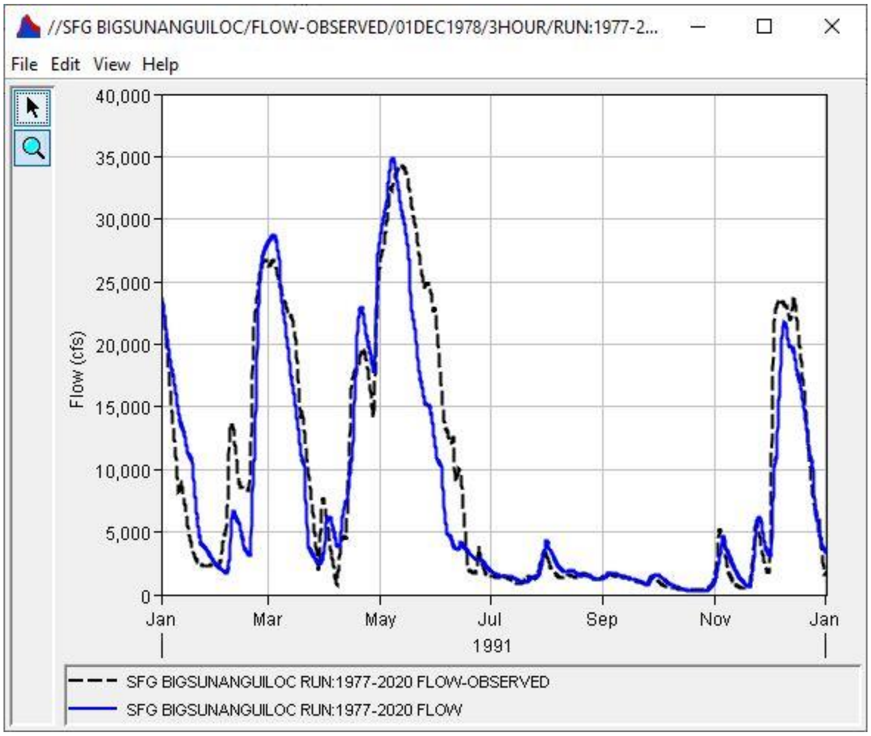

Figure 2-50. Improved Model - Big Sunflower River at Anguilla – 1991.

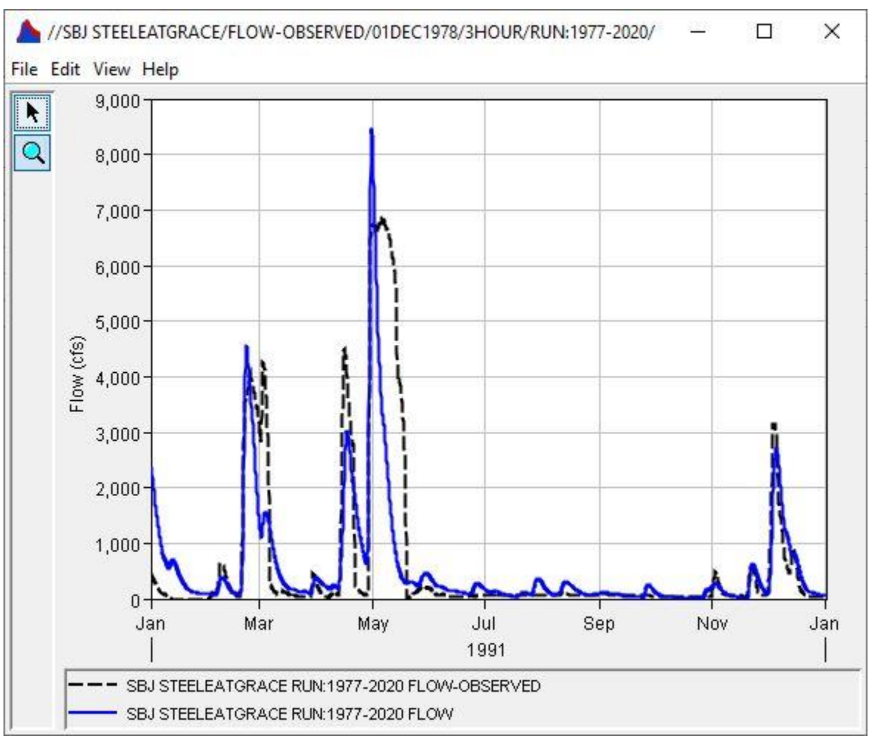

Figure 2-51. Improved Model – Steele Bayou at Grace – 1991.

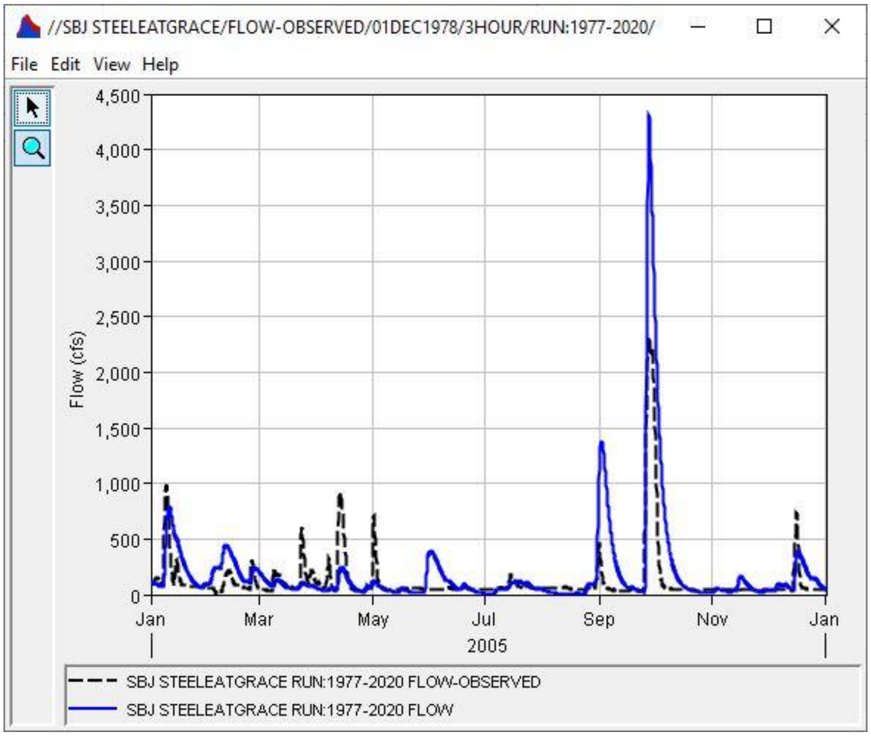

Figure 2-52. Improved Model - Steele Bayou at Grace – 2005.

118. In Figure 2-53 through Figure 2-55, the improved average computed monthly flows are compared against the average observed monthly flows at the three computation points for the 42-year period. As stated before, the monthly plots help demonstrate the volumetric water balance throughout the year. In general, the monthly flow comparison did improve for the Big Sunflower River at Anguilla and Steele Bayou at Grace. However, they did not improve for the Quiver River at Doddsville. This is due to a consistent change that was made to the linear reservoir baseflow parameter. This parameter will be further changed in the future to ensure there is no bias within the model.

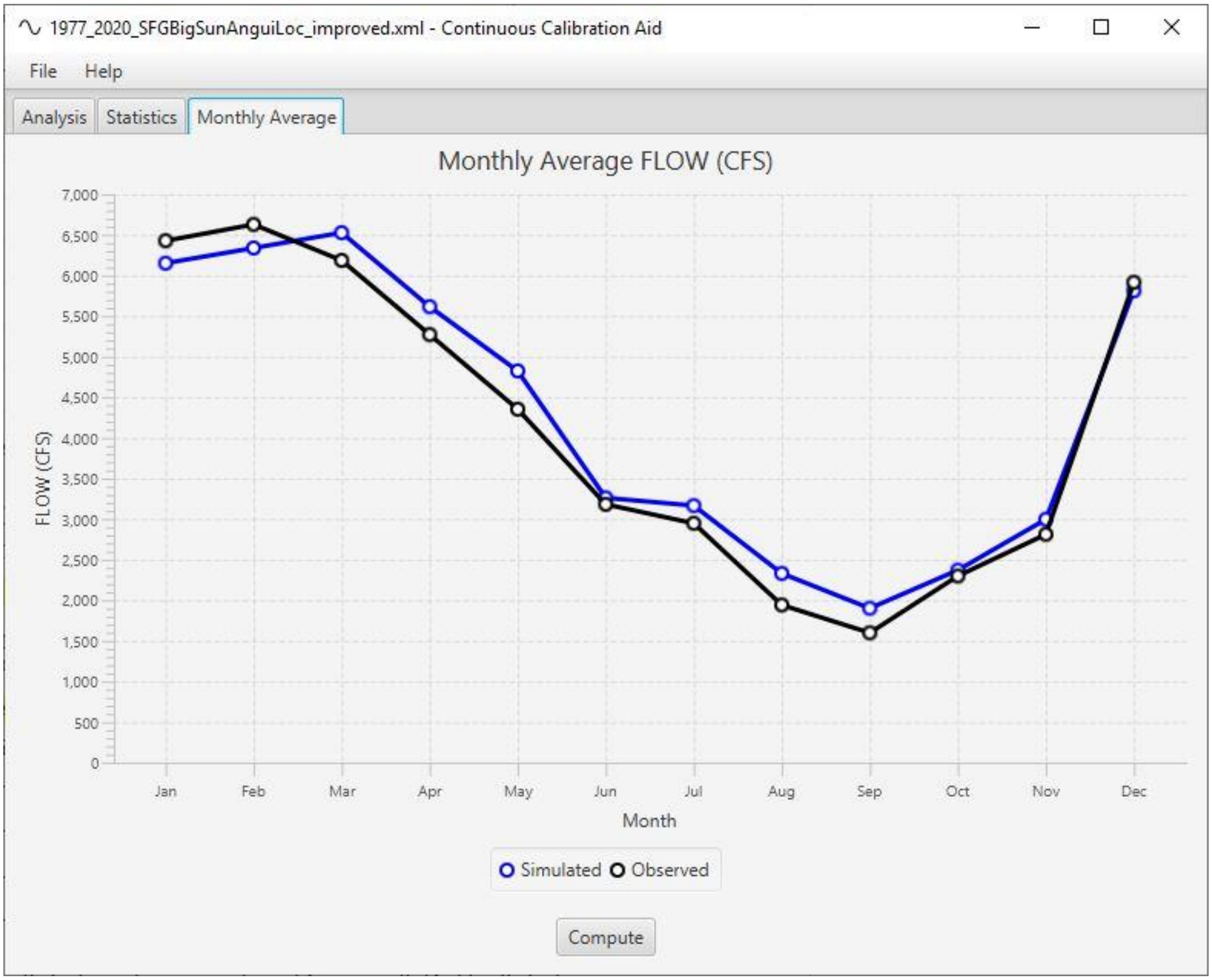

Figure 2-53. Improved Model - Big Sunflower River at Anguilla Monthly Flow Comparison.

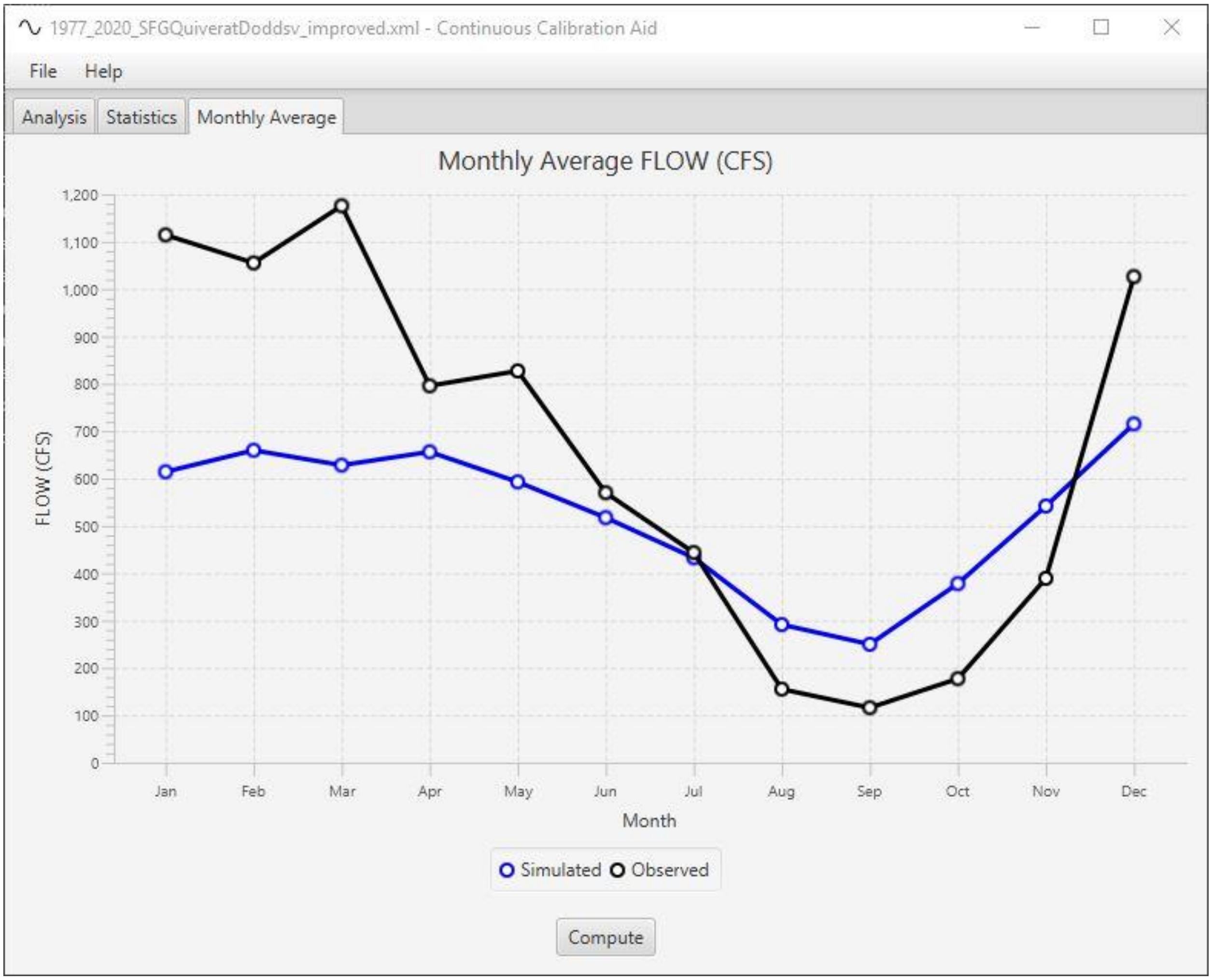

Figure 2-54. Improved Model - Quiver River at Doddsville Improved Monthly Flow Comparison.

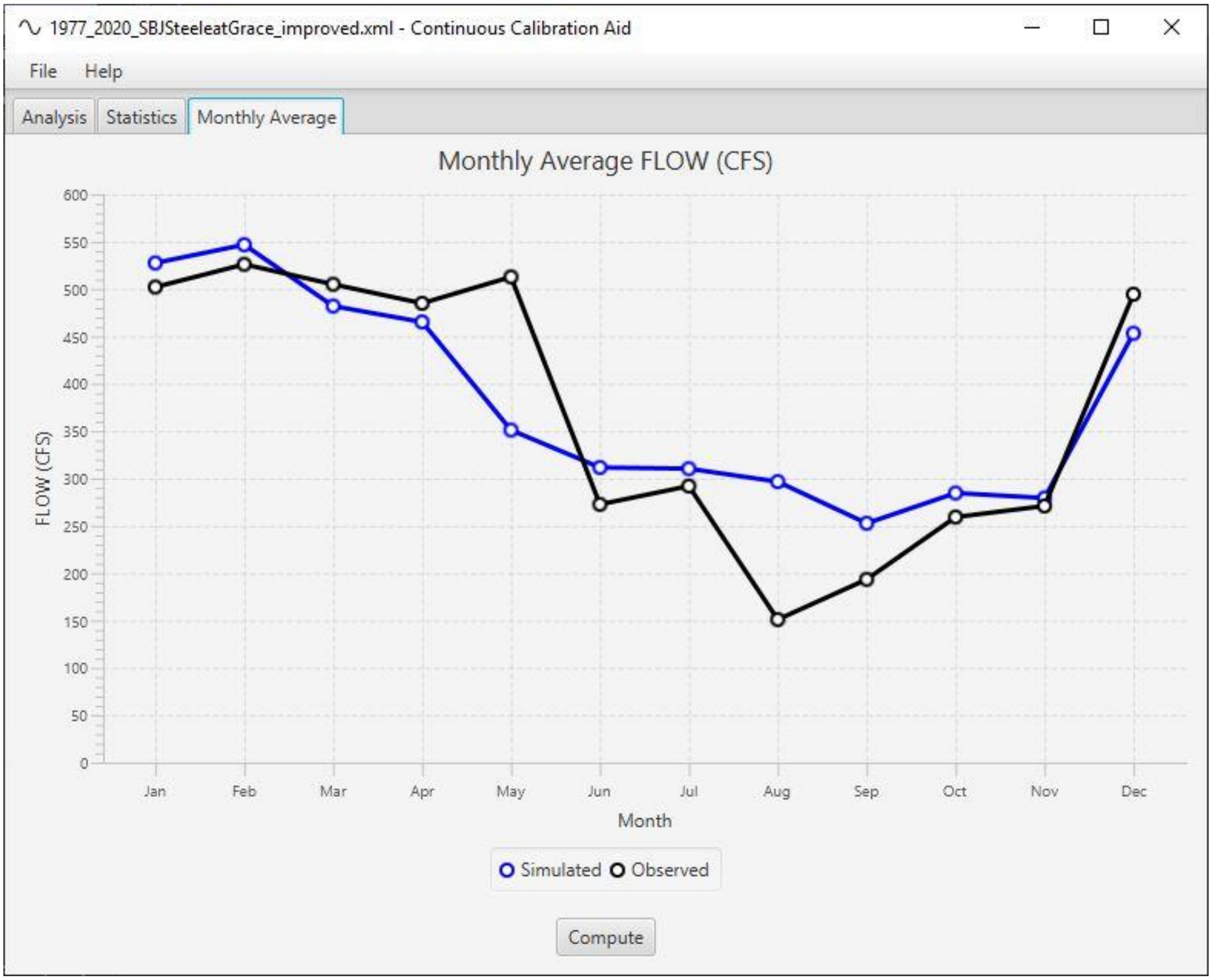

Figure 2-55. Improved Model - Steele Bayou at Grace Improved Monthly Flow Comparison.

119.In Table 2-15, below, the model performance at each computation point is shown for the 42-year simulation. The performance ratings table can be found in the 'HEC-HMS Model Metrics' section above.

Table 2-15. Improved Model - Performance at Computation Points for Forty-Three Year Simulation

Computation Point	NSE	RSR	PBIAS	R2
Anguilla	0.75	0.50	-3.55	0.75
Doddsville	0.46	0.74	19.88	0.53
Grace	0.43	0.76	-2.08	0.43

120.Based on Table 2-15, the Big Sunflower River at Anguilla still had a performance rating of 'very good', the Quiver River at Doddsville dropped down to a performance rating of 'satisfactory' (Doddsville only had data from 1997 to 1998 and represents a very small portion of the watershed), and Steele Bayou at Grace improved to a performance rating of 'satisfactory'.

HYDRAULIC MODEL SETUP

OVERVIEW

121.The updated hydraulic modeling was developed using the HEC-RAS (Hydraulic Engineering Center- River Analysis System) computer program, version 5.1 Alpha 2019-11-22. The alpha version of HEC-RAS was used because this was the first version that allowed for the use of pumps connected to 2D flow areas, and this version was not available beyond the alpha edition. The updated HEC-RAS model utilizes a 2D flow area that extends from the Yazoo Backwater Levee System at the southern and eastern boundaries to Mississippi Highway 82 at the northernmost boundary, and it extends to the Mississippi River Mainline Levee System to the west. The unsteady flow model incorporates and routes the variable flows with adjustments for channel roughness, geometry, and bathymetric data. The unsteady model's ability to simulate changes to the flow and water surface over time allows for a more accurate representation of hydraulic routing of water through the watershed. An existing model was updated by incorporating channels using surveyed bathymetric data, adding hydraulic structures to represent weirs, and revising channel roughness.

STUDY REACHES

122.The 2D flow area representing the Yazoo Study Area extends from the Yazoo Backwater Levee System as the downstream boundary and northward to Mississippi Highway 82. This area includes Steele Bayou, Little Sunflower, Big Sunflower, Bouge Phalia, and Deer Creek channels. Bridges that cross these channels were not modeled because they are considered to have no impact on water surface elevation. Three bridges were overtopped during the 2019 event and were considered for addition. However, these bridges were deemed to have little to no impact on the model results and were removed to improve stability and accuracy. Manning's override regions were created to adjust the Manning's "n" values within the channels. Thus, the model "reaches" used for calibration are the override regions within the Yazoo Backwater 2D flow area. The model reach extents are defined below. The Big Sunflower and Little Sunflower river names were shortened in the model as "Big Sun" and "Little Sun" accordingly.

123.Steele Bayou extends from the Steele Bayou Control Structure to the confluence of the Main Canal and Black Bayou. These channels extend further upstream to their intersection with MS Highway 82. The Little Sunflower/Steele Bayou connecting channel extends from the Steele Bayou Control Structure to the Little Sunflower Control Structure.

124.Little Sunflower River extends from the Little Sunflower Control Structure to the confluence with the Old Sunflower Channel. Old Sunflower River extends downstream to its confluence with the Big Sunflower River and the Holly Bluff Cut-off channel and upstream to the confluence of the Big Sunflower River and the upstream end of the Holly Bluff Cut-off.

125.The Big Sunflower River extends from the confluence with the Little Sunflower River at the downstream end to Mississippi Highway 82 at the upstream end. The Big Sunflower River includes the Holly Bluff Cut-off, which is a 6.5 mile channel that was built to bypass the Old Sunflower Bend reach.

126.Deer Creek North extends from the confluence with Little Sunflower River at the downstream end to Hollandale, Mississippi at the upstream end. Deer Creek South extends from the confluence with the Little Sunflower/ Steele Bayou Connecting Channel at the downstream end to Rolling Fork, Mississippi at the upstream end. Deer Creek South does not have bathymetric data and is considered to have little effect on the area since it runs dry for most of the year, and rain that falls within its banks is the only contribution to its flow. Deer Creek North and South are separated by a cut off at Rolling Fork that diverts the water from Deer Creek North into the Little Sunflower River.

TERRAIN

127.Topographic data for the hydraulic model is primarily based on airborne light detection and ranging (LiDAR) data. The LiDAR data is a 10-meter DEM from the seamless USGS National Elevation Dataset (NED, accessed January 2013). The vertical elevation units were converted from meters to feet, and the dataset was projected into the Albers Projection, using the North American 1983 Datum. All elevations are listed as NAVD 88.

128.Because LiDAR data does not capture elevations below the water surface, bathymetric data was burned into the terrain using a 1D model with cross sections and surveys taken in 1991, 1992, 2001, 2009, 2010, 2014, and 2020 in support of the 2011 Big Sunflower Maintenance Project and various projects associated with the Steele Bayou Sediment Reduction Project. Additional surveys were conducted along Steele Bayou in March 2020. The surveys were conducted in collaboration with the Vicksburg District Geospatial Data Section and ERDC CHL survey personnel. The team surveyed 18 cross sections in various locations within Steele Bayou basin. The cross sections were conducted using the U.S. State Plane NAD83 Mississippi West FIPS 2302 coordinate system and the NAVD 88 Geoid-18 vertical datum. Measurements were all taken in U.S. Survey feet. The survey team ran single beam cross sections in the survey areas within Steele Bayou Basin and took real-time Kinematic (RTK) data where the top bank was accessible. Figure 2-43 identifies the general areas within Steele Bayou that were surveyed during March 2020.

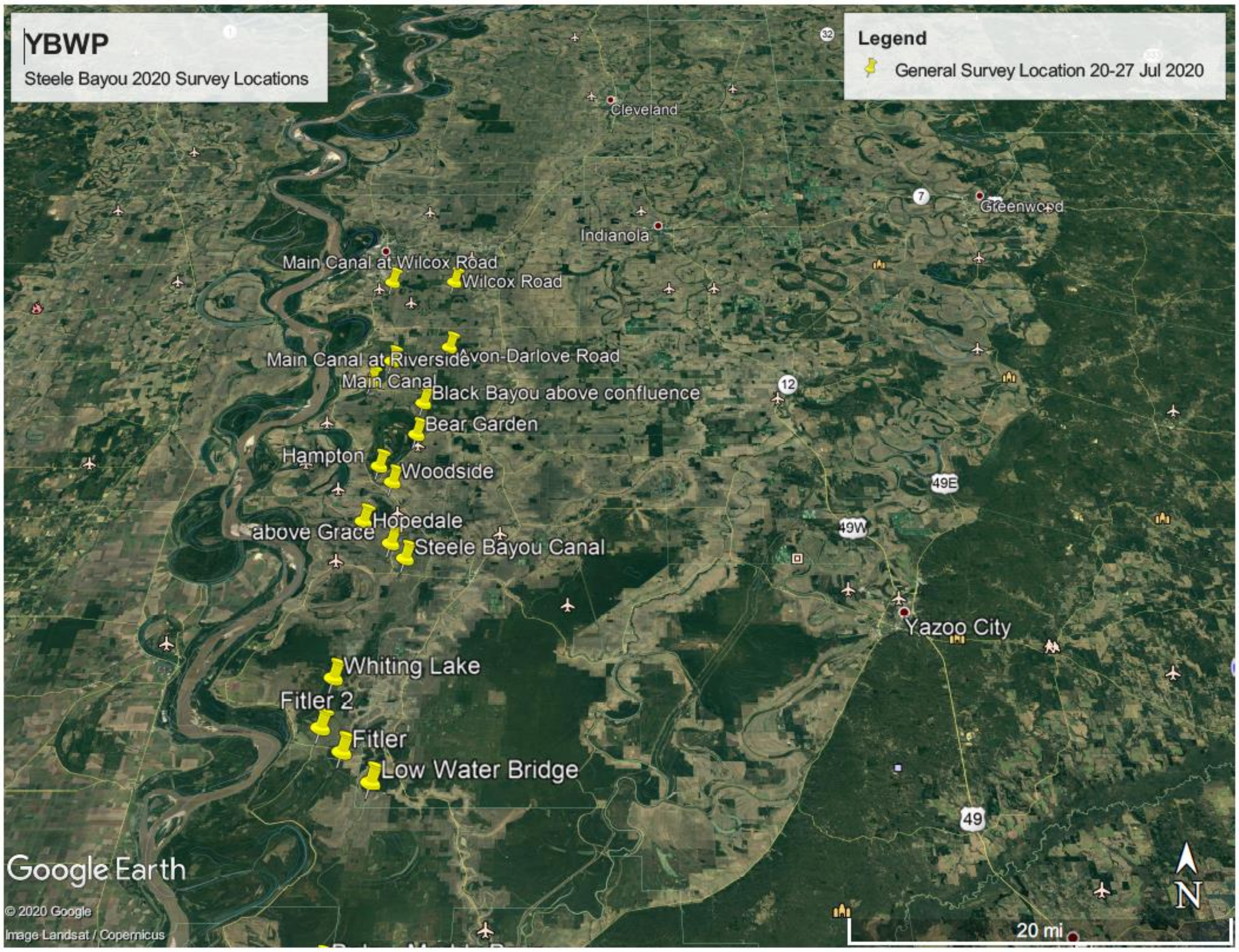

Figure 2-56. Locations within Steele Bayou that were surveyed during March 2020.

129. Cross-sections were drawn where survey data was available. In areas where survey data was unavailable, cross-sections were interpolated. Interpolation was either performed by HEC-RAS or by adjusting the upstream cross section to match the slope of the existing cross-sections. The eastern side of the basin had more extensive cross section coverage though segments had to be stitched together from multiple years. Interpolation was only needed around complex curves on the eastern side of the model due to more available cross sections. On the western side of the model, which included Steele Bayou and Deer Creek, cross-sections were more widely spaced with some being as far apart as 15 miles. Multiple cross-sections were interpolated in these areas, which could lead to a high level of uncertainty in channel geometry.

130. Aerial imagery was used to determine where weirs and other hydraulic structure were located to insure they were properly represented. Any man-made or dredged channels were estimated in the model using as-built plans or surveyed channel thalwegs.

131. Once cross-sections were determined to be a proper representation of the channel, RASMapper was used to create a channel terrain file. The channel terrain files were merged in ESRI ARC-Map. By merging the channels into the LiDAR, bridge decks, or other features misrepresenting the channel, could be removed and a more accurate channel volume could be

determined. Figure 2-57 shows the cross section for the Yazoo Study Area, indicated in red, along the centerlines of rivers modeled, indicated in blue. Some cross sections within the figure have been lengthened so they are more visible from this extent.

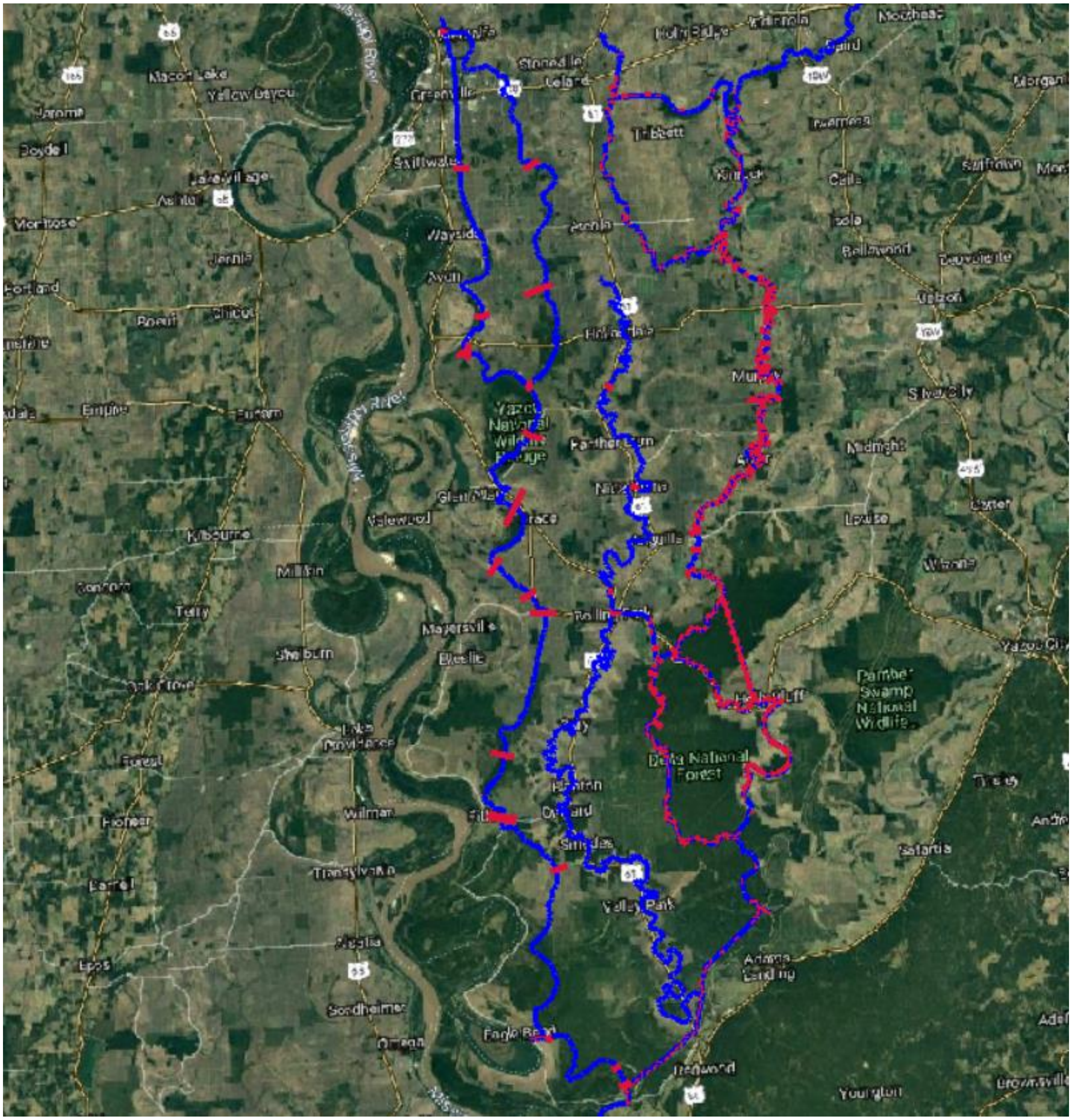

Figure 2-57. The cross sections for the Yazoo Study Area, indicated in red, along the centerlines of the rivers modeled, indicated in blue.

TWO DIMENSIONAL FLOW AREAS

Overview

132.This model utilizes three 2D flow areas, including one for the Yazoo Study Area, named "Yazoo Backwater" in the model, one for the Tara overflow area, and one for the Yazoo River. The 2D flow area for the Yazoo River was used to input riverside stage boundaries for the Little Sunflower and Steele Bayou Control Structures. The Yazoo River was temporarily placed into the model as a 1D reach; however, 1D was determined to be too unstable to accurately model the flow leaving the control structures. The 1D geometry also proved to be a less accurate calibration for the riverside stages, which led to the control structure gates not being opened at appropriate times.

133.The cell size throughout most of the 2D flow area is 2000 feet. Refinement regions were created around the channels, with cell sizes ranging from 200 feet to 500 feet. Channels not represented by refinement regions are represented using breaklines, due to lack of channel terrain survey information in some locations. Breaklines were utilized to represent roads and other high ground in the 2D flow area. Cells enforcing the breaklines are as small as 50 feet.

Internal Hydraulic Structures

134.Internal hydraulic structures were used to represent structures that cross the channel. The coordinates and elevations of structures were provided in a kmz file. Structures and their information are listed in Table 2-16.

Table 2-16. Coordinates and Elevations of Internal Hydraulic Structures

Name	Latitude	Longitude	Elevation
Weir E	33.1316883342	-90.9972838539	97.0
Main Canal Weir 2	33.2537683542	-91.0005132539	103.5
Black Bayou Weir 4	33.365164546	-90.9545944169	107.0
Black Bayou Weir 3	33.2823493887	-90.9246694127	101.5
Black Bayou Weir 2	33.1576412193	-90.9248162494	96.0
Black Bayou Weir 1	33.1219421683	-90.9584477152	93.0
SB Weir Rolling Fork	32.9076077378	-90.9533827388	86.0
Steele Bayou Weir 1	32.7494575452	-91.0282707263	78.0
Bogue Phalia Weir 1	33.2355860402	-90.8106721248	92.0
Big Sun Lock 1 Weir	33.1731825829	-90.6836090928	82.5

Storage Areas (SA)/2D Connection

135.Multiple SA/2D connections were used to connect 1D and 2D flow areas to one another (Table 2-17). Connections were used at Muddy Bayou control structure, Steele Bayou Control Structure, and Little Sunflower Control Structure; all three were controlled via gate rules. The Muddy Bayou Structure includes the gates as well as a roughly 0.6 foot gap between the gates and the top of the bridge that was discovered during the 2019 flood event.

Table 2-17. SA/2D Connections Used to Connect 1D and 2D Flow Areas

Name	Connections	Gates	Gate Invert (feet, MSL)
Steele	Yazoo Backwater – Yazoo River	4 sluice: 30x22.5 feet	60
Little Sunflower	Yazoo Backwater – Yazoo River	2 sluice: 30x22.5 feet	60
Muddy Bayou	Eagle Lake – Yazoo Backwater	2 sluice: 12x20 feet	65
	*Note: This structure also contains a 270x0.6 feet overflow area with an invert elevation of 96.6 feet, MSL		
48" Culvert	Eagle Lake – Tara Overflow	N/A	N/A
EL_5000	Tara Overflow – Yazoo Backwater	N/A	N/A
Eagle Lake Connection	Eagle Lake – Yazoo Backwater	N/A	N/A
	*Elevations along this ridge beside Eagle Lake were taken from a previous survey		
Muddy_ROB	Eagle Lake – Yazoo Backwater	N/A	N/A
Muddy_LOB	Eagle Lake – Yazoo Backwater	N/A	N/A

Manning's "n" Roughness for 2D

136. The roughness of the 2D flow area was based off the 2016 National Land Cover Database (NLCD) for the Contiguous U.S. Table 2-18 shows the values used for the 2D land cover data. The Manning's "n" value for cultivated crops was used as a calibration point, since much of the land in the Yazoo Study Area is used for crop cultivation. High water events most frequently occur during crop season. Thus, it was assumed that the increase in vegetation would increase the overbank roughness during this time of year. HEC-RAS does not currently allow the Manning's "n" value to be changed throughout the year in a 2D flow area, as a result, the cultivated crop value remains high even during non-crop seasons.

Table 2-18. Manning's n-Values used for 2D Flow Areas in the Yazoo Study Area HEC-RAS Model

Name	Manning's "n"	Override Values
Woody wetlands	0.08	----
Developed, open space	0.04	----
Open water	0.03	----
Cultivated crops	0.03	0.05
Barren land rock/sand/clay	0.025	----
Emergent herbaceous wetlands	0.065	----
Developed, medium intensity	0.08	----
Evergreen forest	0.12	----
Developed, low intensity	0.095	----
Developed, high intensity	0.15	----
Deciduous forest	0.13	----
Grassland/herbaceous	0.09	----
Mixed forest	0.12	----
Pasture/hay	0.03	----
Shrub/scrub	0.1	----

137. Manning's override regions were created to adjust the Manning's "n" values within the channels. These regions were created using banklines exported from the cross-sections that were used to create the channel terrain. Manning's "n" values within channels were calibrated with

observed stage data from gages that model data could be compared to. Manning's "n" values used for each channel are provided in Table 2-19.

Table 2-19. Manning's n-Values Used in Channel Override Regions

River	Reach	Manning's "n"
Bogue Phalia	Reach 1	0.032
Bogue Phalia	Cut-off	0.035
Bogue Phalia	Reach 2	0.035
Big Sun	Reach 1	0.038
Big Sun	Reach 2	0.035
Big Sun	Reach 3	0.035
Big Sun	Reach 4	0.03
Holly Bluff	Cut-off	0.03
Old Sun Bend	Reach 1	0.03
Old Sun Bend	Reach 2	0.03
Little Sun	Reach 1	0.03
Little Sun	Reach 2	0.03
Little Sun	Reach 3	0.03
Steele Bayou	Reach 1	0.04
Steele Bayou	Reach 2	0.035
Black Bayou	Reach 1	0.035
Little Sun – Steele Bayou	Connection	0.03
Deer Creek	Reach 1	0.035

Boundary Conditions

138. Calibrated flows from the HEC-HMS model were used throughout the HEC-RAS model. An observed stage hydrograph served as the boundary condition for the riverside of the Little Sunflower and Steele Bayou Control Structures, and the structures were operated using a basic rules set. The structures were opened when the landside water surface elevation was above 70 feet, MSL, and the landside water surface elevation is higher than the riverside water surface elevation. Only historic events were modeled as the observed data could be used to assess model accuracy.

139. Additional boundary conditions were used within the 2D flow area to represent flows at critical locations. All flows were calibrated using HEC-HMS local inflow points, except for the Phalia at Leland boundary, which is an observed flow that was input into HEC-HMS. HEC-HMS rewrote this data as an output that was used as an input in HEC-RAS. Big Sun at Quiver was also based on observed data. A gage exists upstream of the HEC-RAS input location at Big Sunflower at Sunflower with observed flow. Then, flow was routed through HEC-HMS on the Quiver River. These two flows were combined and output from HEC-HMS on the Big Sunflower River at Highway 82. Precipitation inflow was added to Eagle Lake to prevent the pool from remaining stagnate throughout the model run. The same boundary conditions were used in the "With-Pump" and "Without-Pump" scenarios; the only difference between the "With-Pump" and "Without-Pump" scenarios was the addition of the pump station within the geometry. Table 2-20 provides information on each of the boundary conditions.

Table 2-20. Boundary Conditions for the Yazoo Study Area HEC-RAS Model

2D Flow Area	HEC-RAS Location	Boundary Condition Type	HEC-HMS Connection	HEC-HMS Data Type
Yazoo Backwater	Phalia at Leland	Observed Flow Hydrograph	PHALIAATLELAND	FLOW
Yazoo Backwater	Main Canal at Longwood – 2	Flow Hydrograph	LONGWOOD	FLOW
Yazoo Backwater	Steele at Grace	Flow Hydrograph	STEELEGRACE	FLOW
Yazoo Backwater	Deer Creek North	Flow Hydrograph	DEERCREEKN	FLOW
Yazoo Backwater	Big Sun at Little Calleo	Flow Hydrograph	LITTLECALLEO	FLOW
Yazoo Backwater	Big Sun at Holly Bluff – 2	Flow Hydrograph	HOLLYBLUFF	FLOW
Yazoo Backwater	Steele Mouth	Flow Hydrograph	STEELEMOUTH	FLOW
Yazoo Backwater	Big Sun at Quiver	Flow Hydrograph	BIGSUNATQUIVER	FLOW
Yazoo Backwater	Little Sun -2	Flow Hydrograph	LITTLESUNFLOWER	FLOW
Yazoo Backwater	Little Sun – 1	Flow Hydrograph	LITTLESUNFLOWER	FLOW
Yazoo Backwater	Steele at Muddy Bayou	Flow Hydrograph	MUDDYBAYOU	FLOW
Yazoo Backwater	Main Canal at Longwood – 1	Flow Hydrograph	LONGWOOD	FLOW
Yazoo Backwater	Deer Creek South	Flow Hydrograph	DEERCREEKS	FLOW
Yazoo Backwater	Big Sun at Holly Bluff – 1	Flow Hydrograph	HOLLYBLUFF	FLOW
Yazoo Backwater	Big Sun at Anguilla	Flow Hydrograph	ANGUILLA LOC	FLOW
Yazoo River	Little Sun RS	Stage Hydrograph	N/A	N/A
Yazoo River	Steele Riverside	Stage Hydrograph	N/A	N/A
N/A	Eagle Lake	Lateral Inflow	N/A	N/A

Pumping Station

140. For the “With-Pump” scenario, a pump station was added to the base geometry. The pumping station was added at the confluence of the Little Sunflower/Steele Bayou Connecting Channel and Deer Creek South. Twelve pumps were modeled with a combined capacity of 14,000 cfs. Due to restrictions in HEC-RAS, the pumps were divided into two different pumping groups, with six pumps each. Each pump group was placed in a different 2D cell, and the starting time was staggered to eliminate instability within the model and to more accurately simulate pump operation, as the pumps will most likely be turned on in stages while the water level increases rather than all 12 pumps being turned on instantaneously. Table 2-21 shows the “on” and “off” elevations for each pump. The pump flow was based off an average efficiency

curve for pumps originally considered for the project. These efficiency curves were provided by the pump manufacturers.

Table 2-21. The "On" and "Off" Elevations for each Pump within the Yazoo Study Area HEC-RAS Model

Pump Group	Pump Number	Pump "on" Elevation (feet MSL)	Pump "off" Elevation (feet MSL)
1	1	87.0	86.9
	2	87.05	86.95
	3	87.1	87.0
	4	87.15	87.05
	5	87.2	87.1
	6	87.25	87.15
2	7	87.0	86.9
	8	87.05	86.95
	9	87.1	87.0
	10	87.15	87.05
	11	87.2	87.1
	12	87.25	87.15

CALIBRATION AND WITHOUT-PUMP SCENARIO

Overview

141.Four events were provided for calibration of the model. These years represented different event conditions on the Yazoo River and in the Yazoo Study Area. The entire year was examined to monitor how the model handled both high water events and low water periods since the ultimate goal was to run the entire period-of-record. Calibration years were 1991, 2004, 2011, and 2019. The 1991 and 2019 calibration years represented high Yazoo Backwater and high Mississippi River events. The 2004 calibration year represented an average Yazoo Backwater and Mississippi River event. The 2011 calibration year represented a low Yazoo Backwater and high Mississippi River event.

142.The starting elevation of the 2D flow area was entered as the elevation of Steele Bayou landside on the beginning date of the model run. Currently, HEC-RAS does not have the capability to have more than one starting water surface elevation within a 2D flow area. This created an artificially low elevation in the upper region of the 2D flow area. In order to establish an accurate starting elevation for the upper parts of the region, the HEC-RAS model was run from 01 December of the previous year. For the 2019 calibration event, a restart file beginning at the start of the high water event in September of 2018 was created. This hot start file prevented running additional months each time, eliminating any unnecessary run times.

143.With a 40-year period-of-record, it was assumed that all events would not calibrate with the same level of accuracy due to silt buildup and erosion throughout the basin over the period-of-record. With that in mind, the calibration for the period-of-record is not perfect, but rather the best model representation for such a long duration.

Calibration

144. The HEC-RAS calibration was originally completed primarily in HEC-RAS. However, after it was determined uncertainty existed within the precipitation data, the calibration focus shifted to the precipitation data and HEC-HMS parameters. The HEC-HMS parameters were adjusted and then re-integrated into the HEC-RAS model. This back-and-forth calibration between the HEC-HMS and HEC-RAS model was performed iteratively in order to narrow down the best parameters for calibration. This calibration approach also allowed for more variables, in addition to the roughness factor in the 2D flow areas, to be modified simultaneously.

145. The calibration events and the "Without-Pump" scenario used the same geometry. Results of calibration were compared at six gage locations: Steele Bayou landside, Little Sunflower landside, Steele Bayou at Grace, and Big Sunflower at Little Calleo, Holly Bluff, and Anguilla. Stage outputs at these locations were obtained by inserting reference points in the 2D flow area.

146. Figure 2-58 through Figure 2-63 shows some of the calibration run results versus the observed data. It is evident in the figures below that some years resulted in hydrographs that were much closer to the observed information than other years. Additionally, gages in the upper portion of the basin experienced higher degrees of error compared to gages at Little Sunflower and Steele Bayou Control Structures. Calibration runs also showed that stages were consistently too high during low flow periods, but it was deemed more important to accurately portray peaks over low flow since the modeling effort was primarily concerned with higher events in which pumps would operate. The discrepancies between years at a single gage could have resulted from using one set of channel data for the entire period-of-record or from inherent errors within the precipitation data.

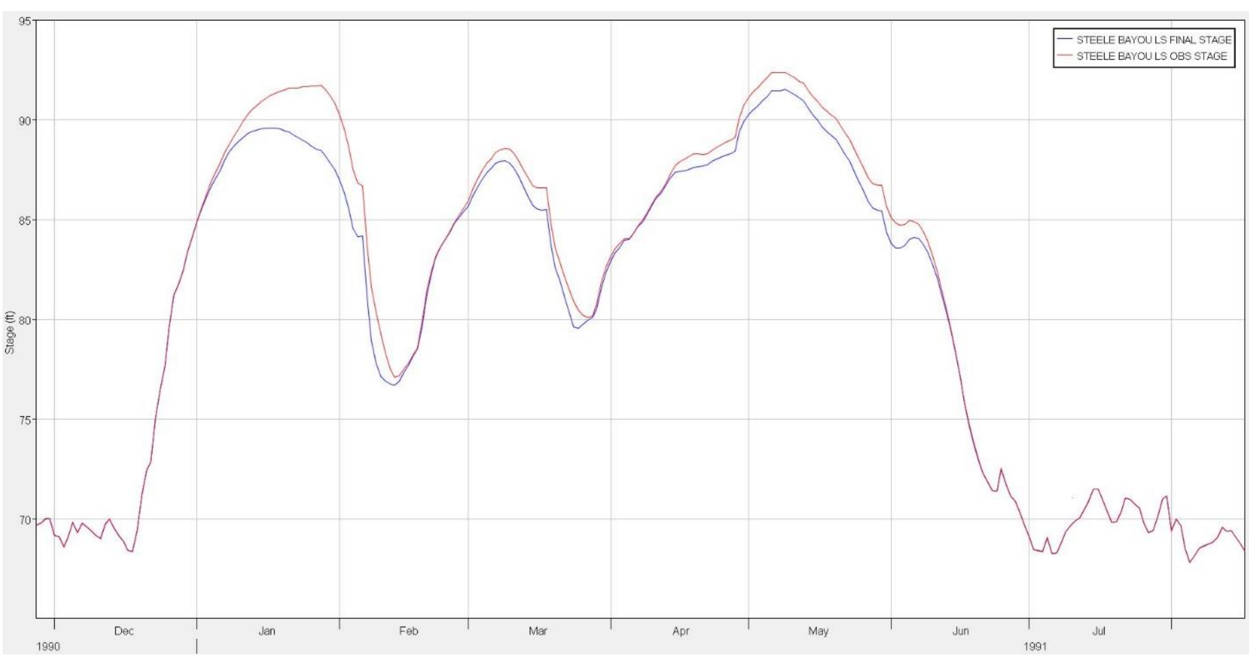

Figure 2-58. Steele Bayou Landside 1991 Calibration.

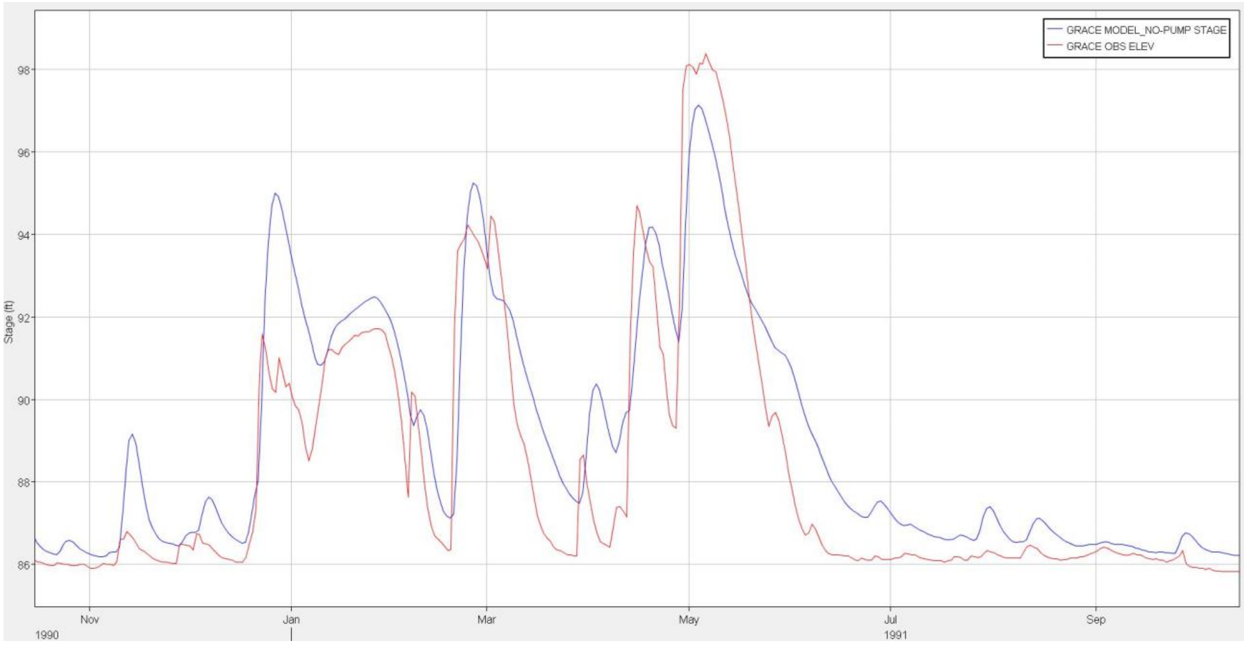

Figure 2-59. Steele Bayou at Grace 1991 Calibration.

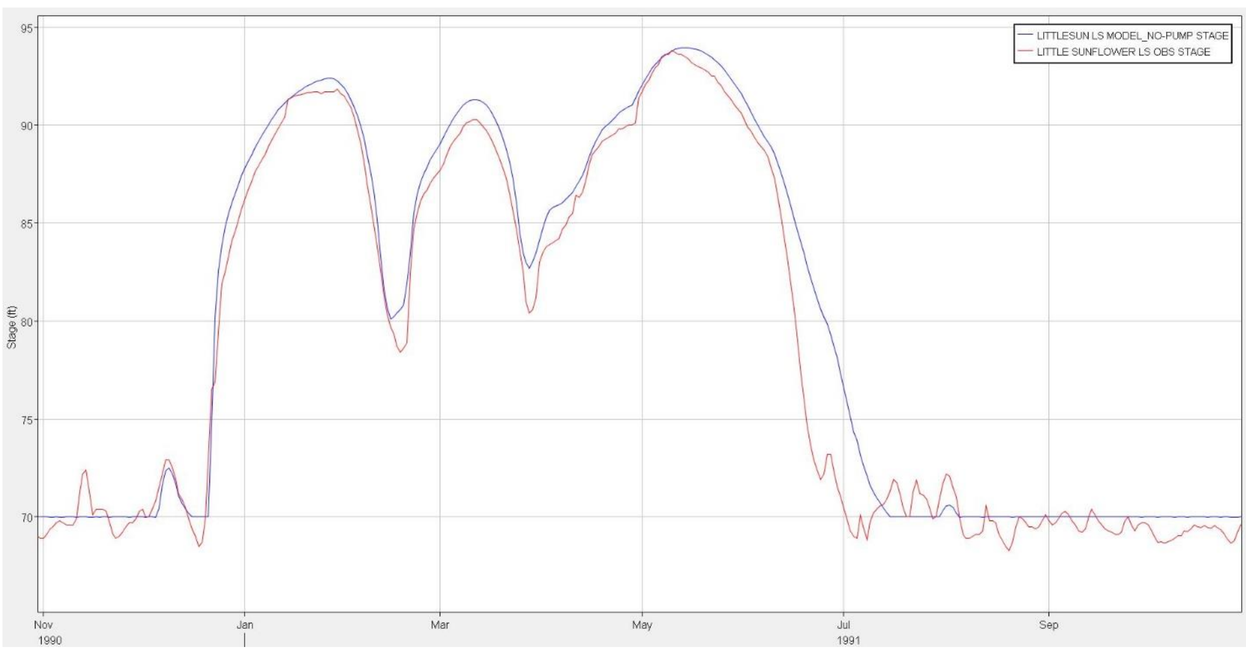

Figure 2-60. Little Sunflower Control Structure 1991 Calibration.

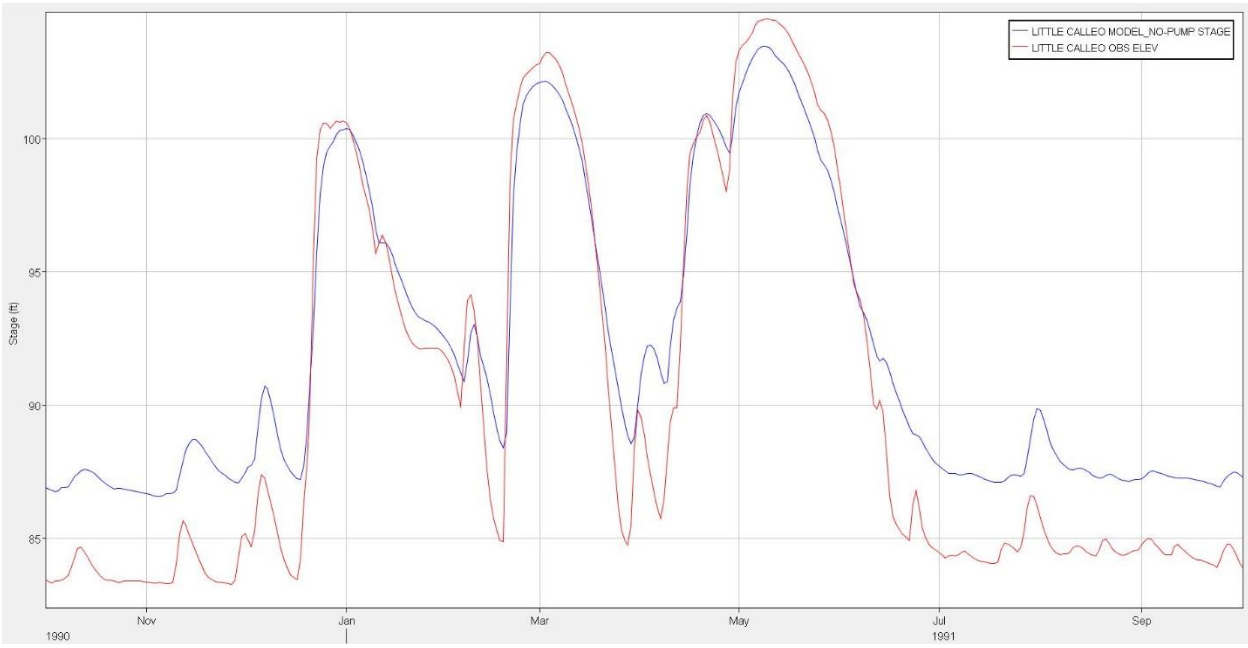

Figure 2-61. Big Sunflower at Little Calleo 1991 Calibration.

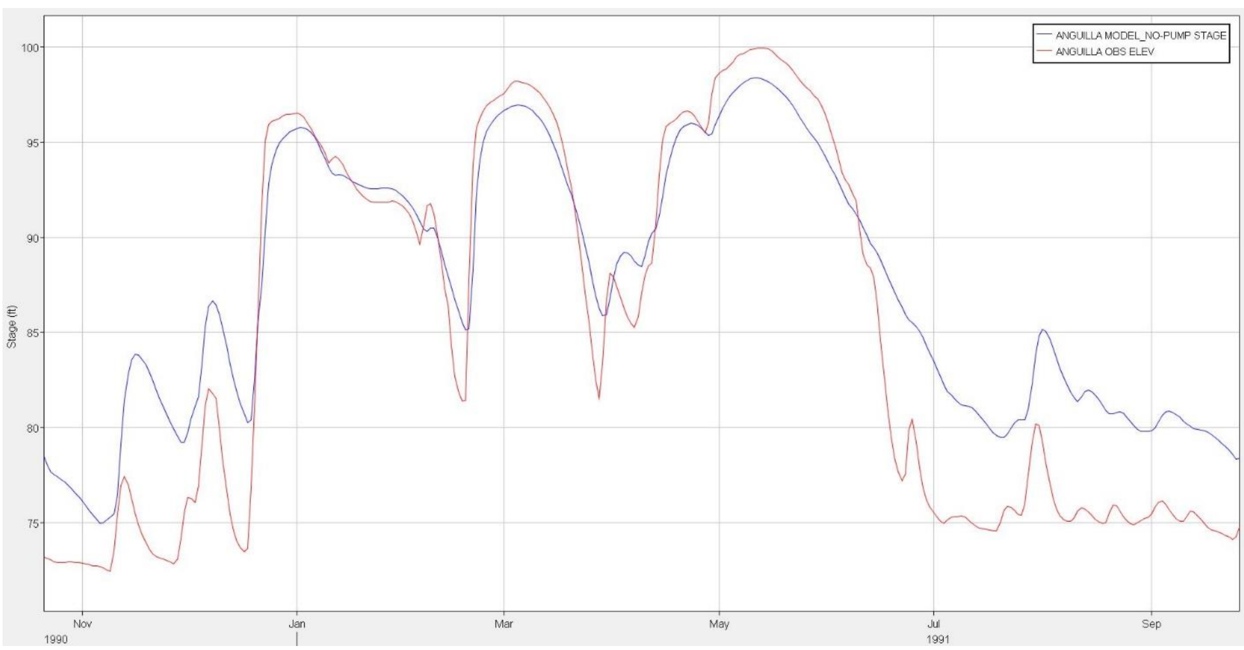

Figure 2-62. Big Sunflower at Anguilla 1991 Calibration.

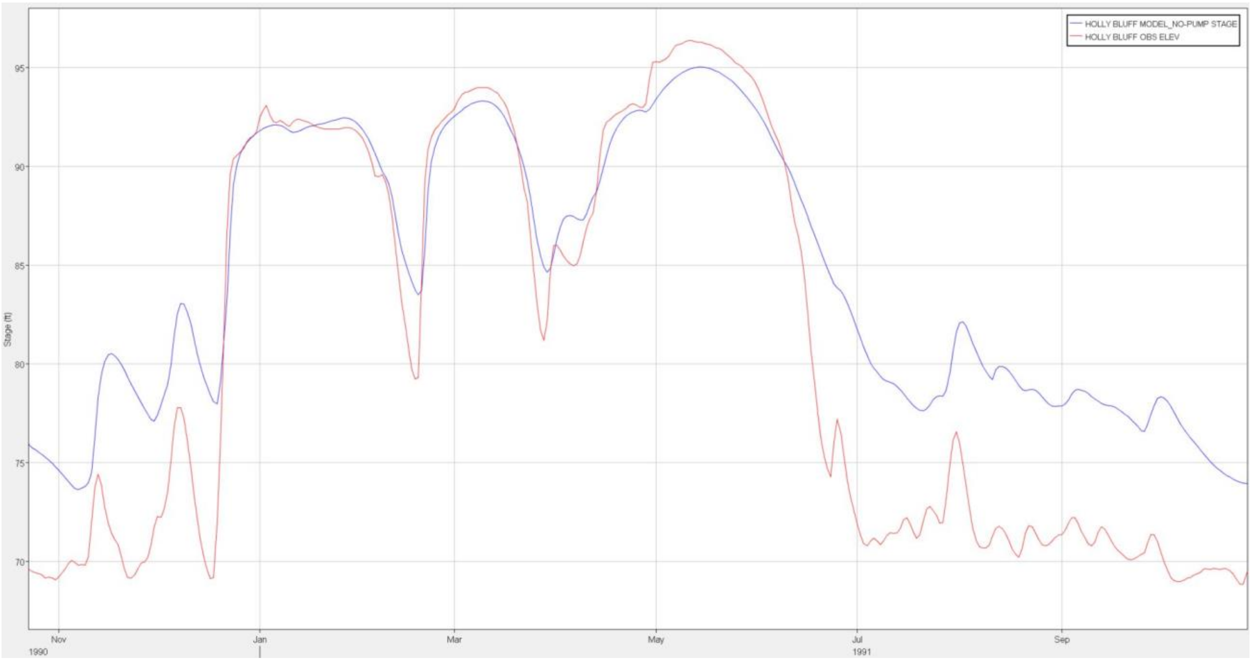

Figure 2-63. Big Sunflower at Holly Bluff Calibration 1991.

147.Figure 2-64 through Figure 2-75 shows the 2004 calibration. The 2004 calibration had the highest uncertainty with the precipitation data. The peaks for this year were lower than the observed data, and the timing was off at certain gages. Changing calibration parameters drastically to correct for the high level of uncertainty in years, such as 2004, would have decreased the level of accuracy seen in years that the precipitation had less uncertainty.

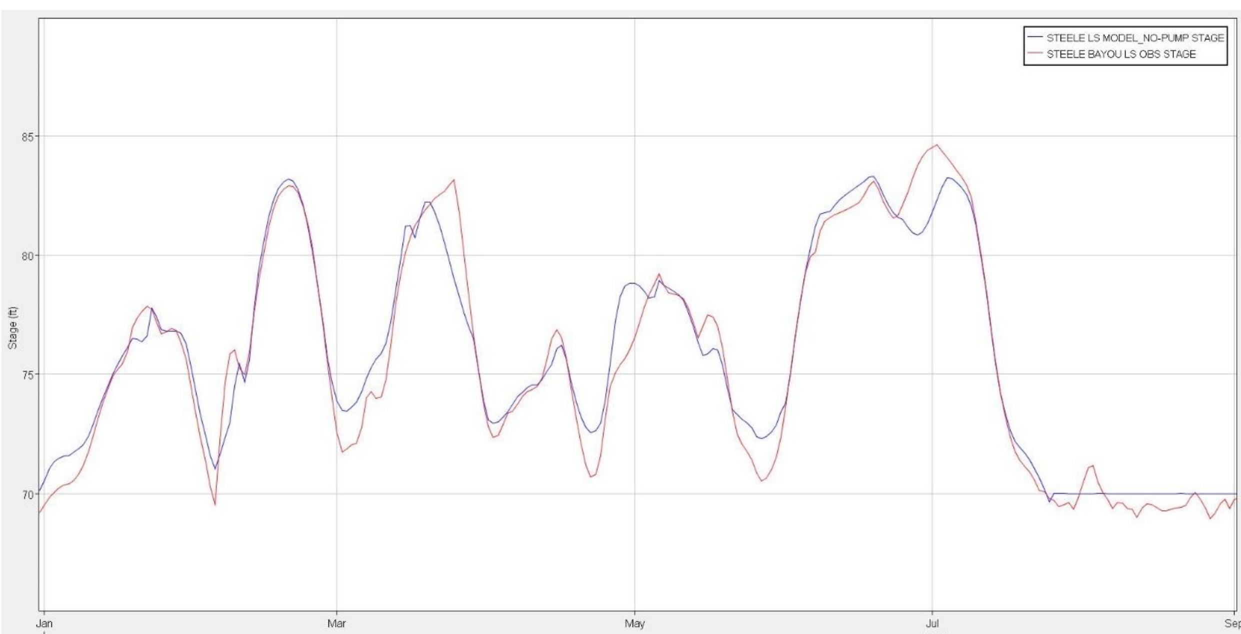

Figure 2-64. Steele Bayou Landside 2004 Calibration.

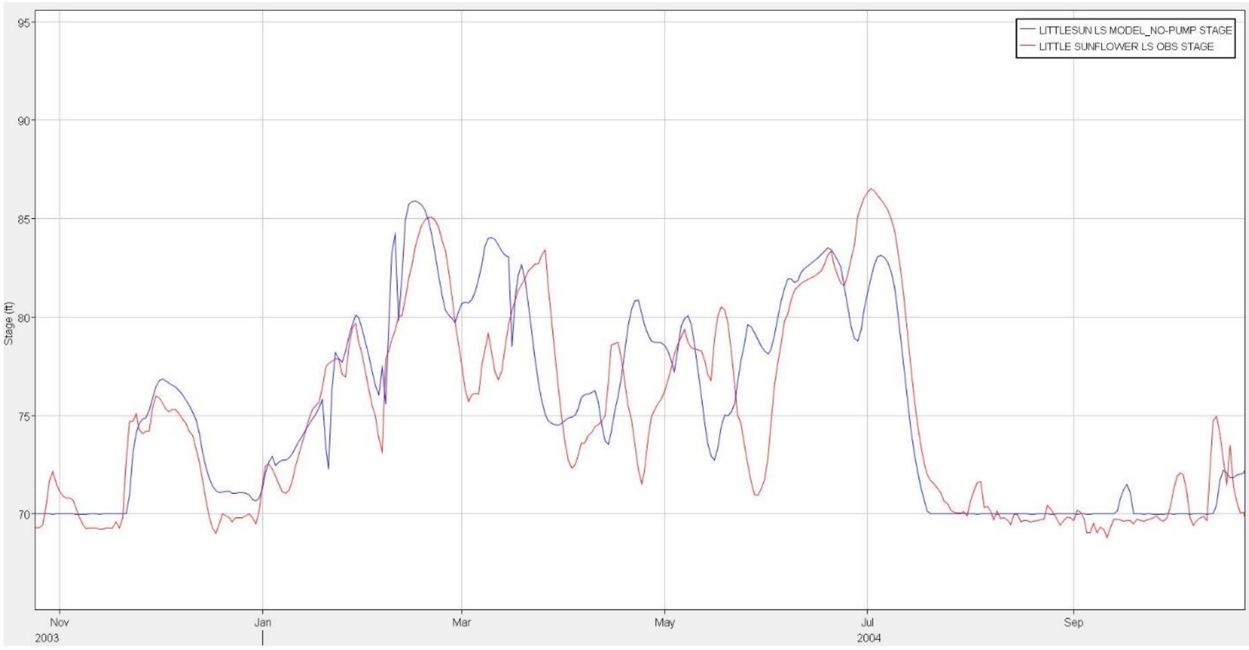

Figure 2-65. Little Sunflower Control Structure 2004 Calibration.

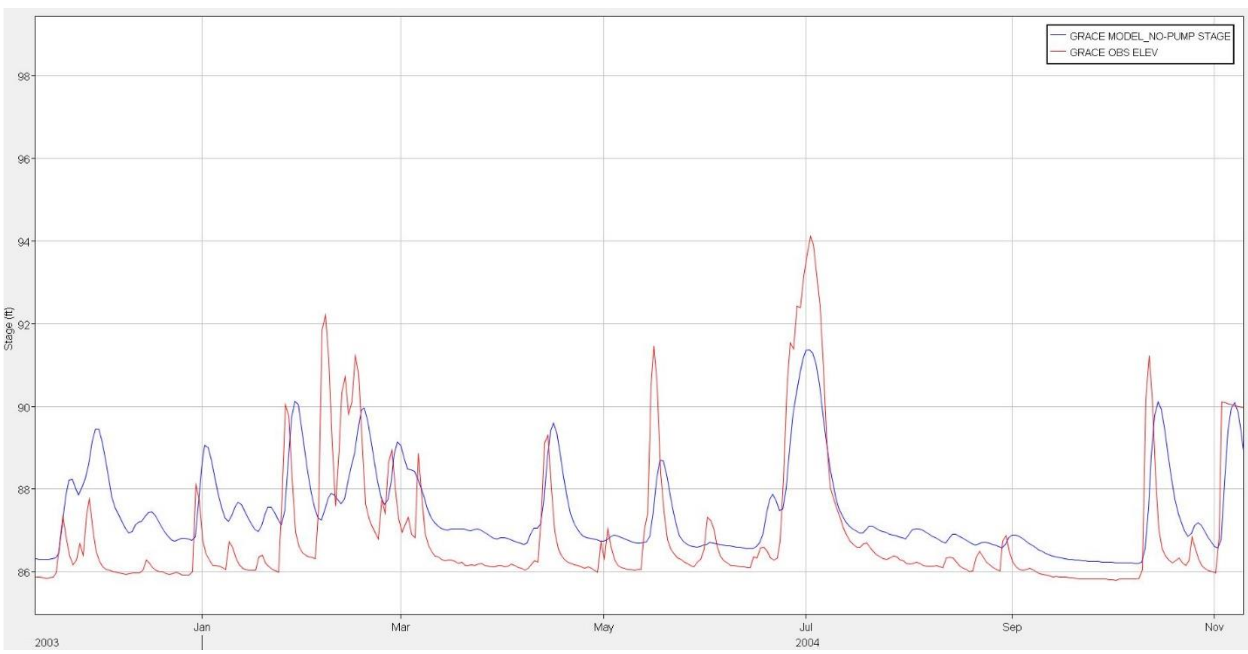

Figure 2-66. Steele Bayou at Grace 2004 Calibration.

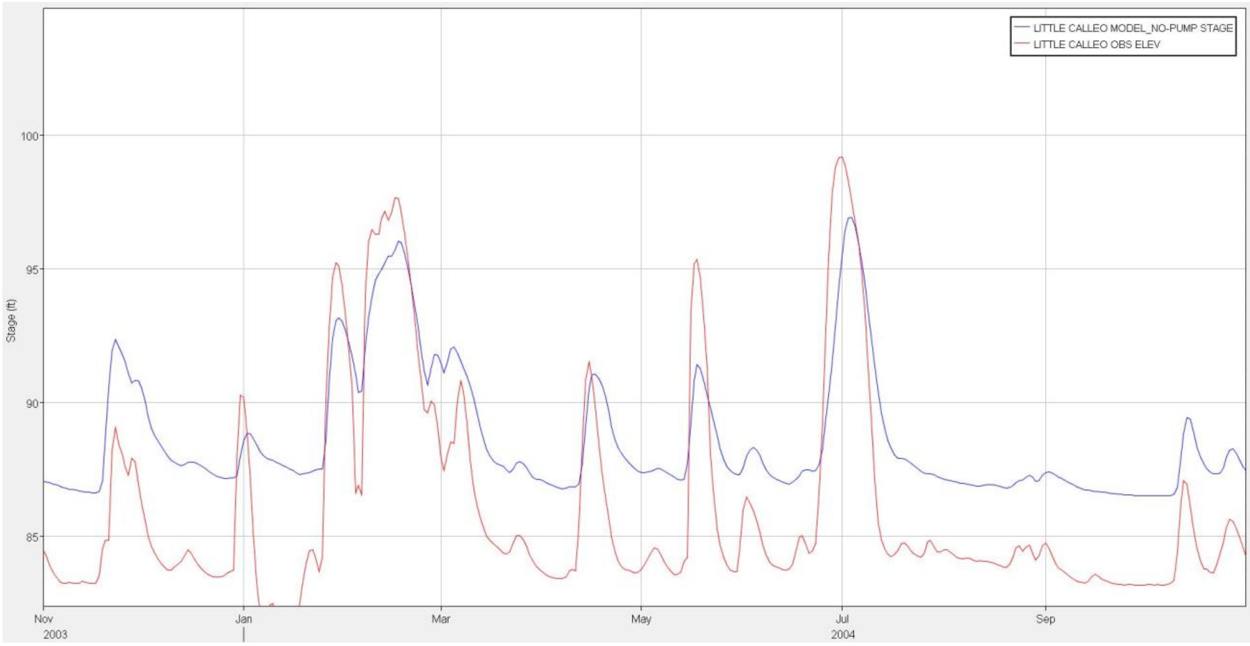

Figure 2-67. Big Sunflower at Little Calleo 2004 Calibration.

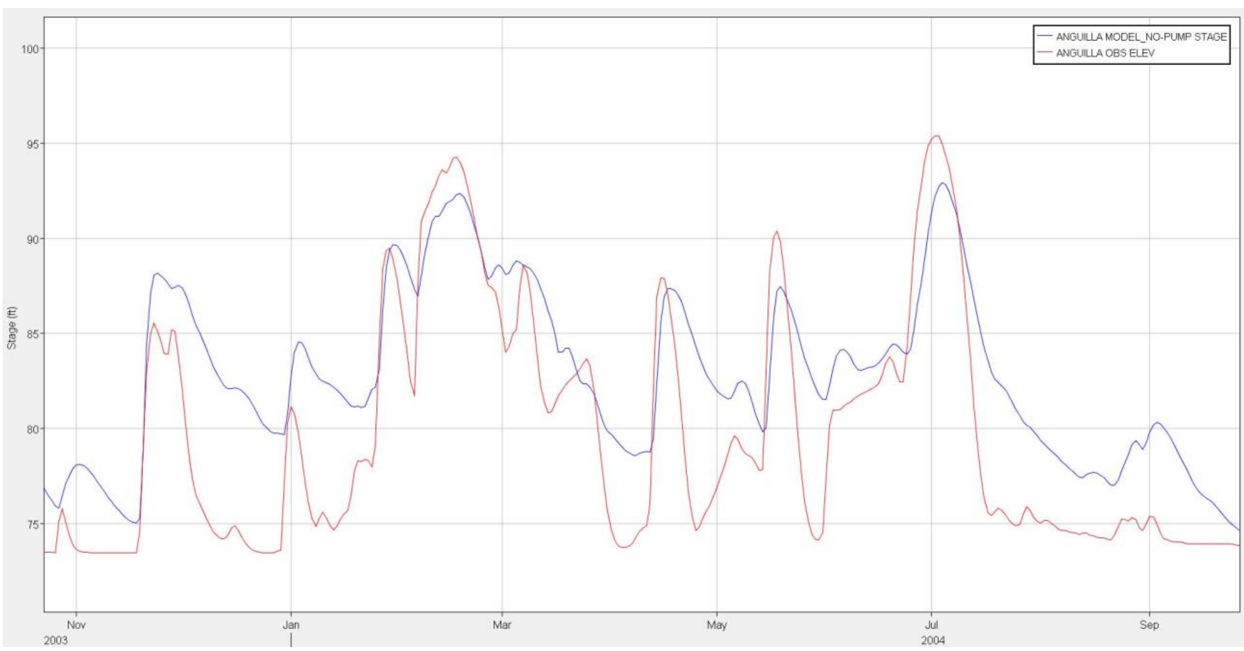

Figure 2-68. Big Sunflower at Anguilla 2004 Calibration.

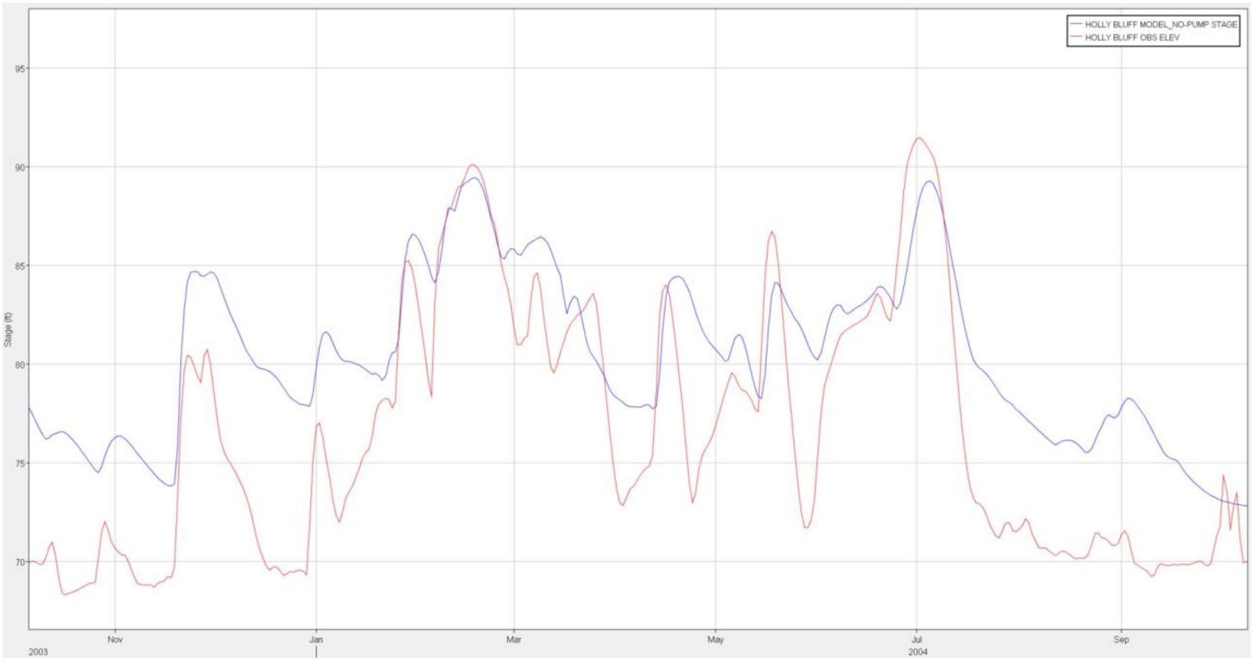

Figure 2-69. Big Sunflower at Holly Bluff 2004 Calibration.

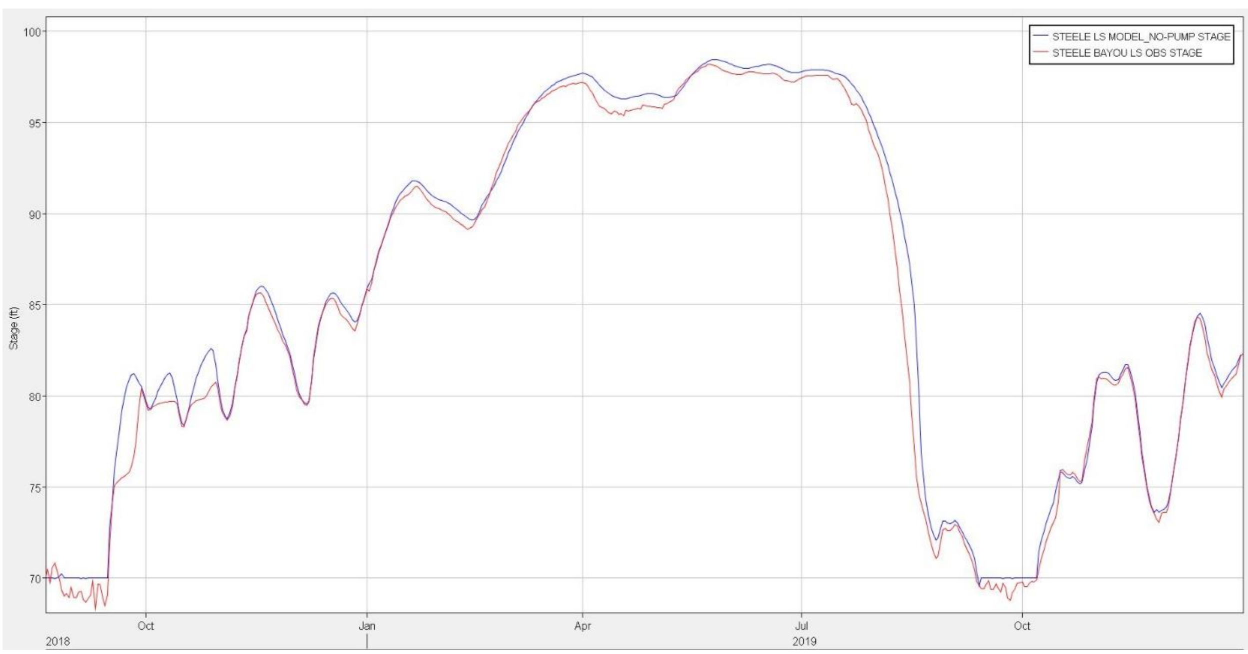

Figure 2-70. Steele Bayou Landside 2019 Calibration.

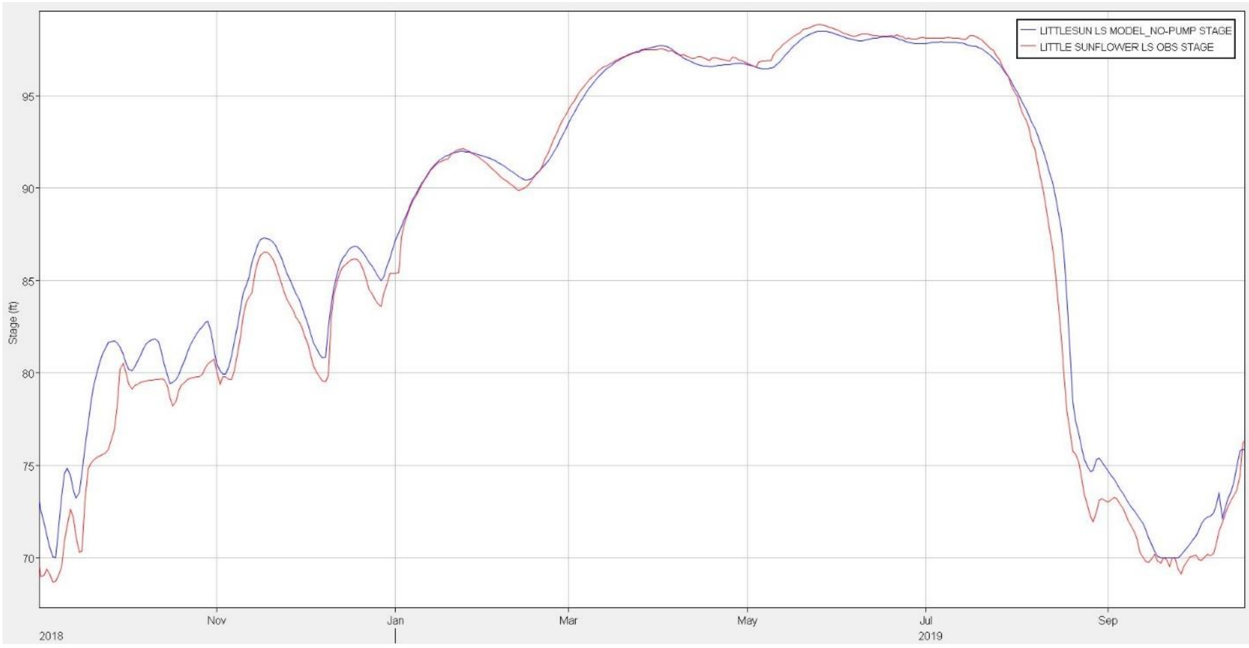

Figure 2-71. Little Sunflower Control Structure Landside 2019 Calibration.

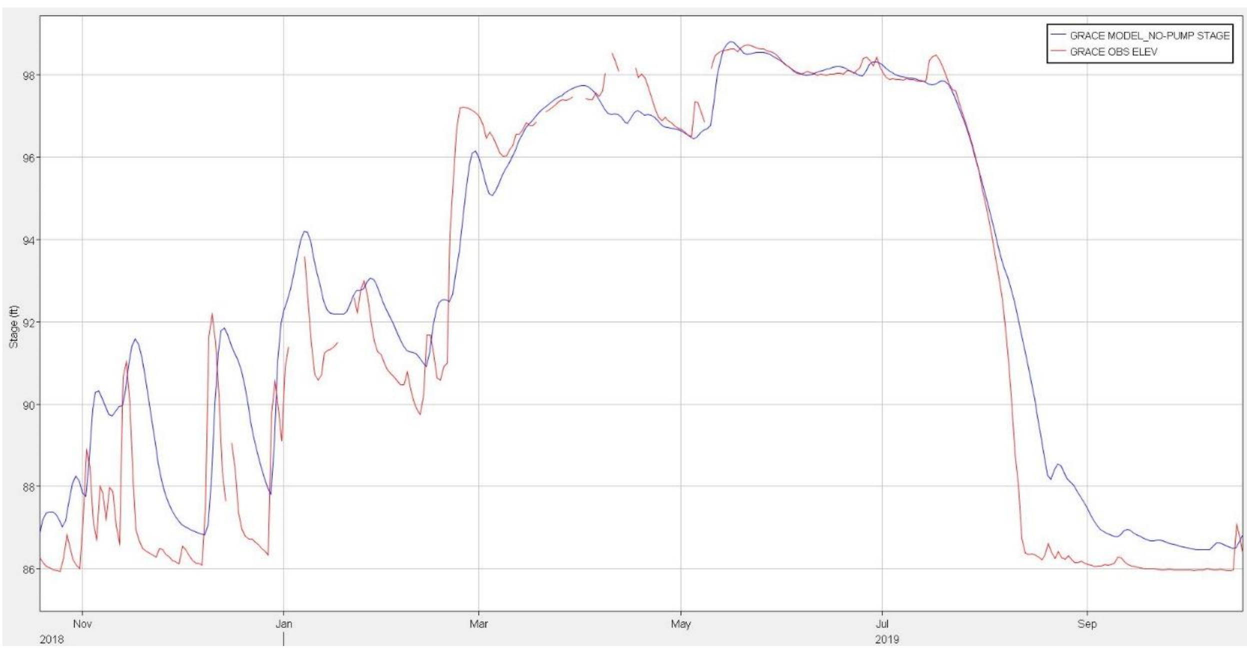

Figure 2-72. Steele Bayou at Grace 2019 Calibration.

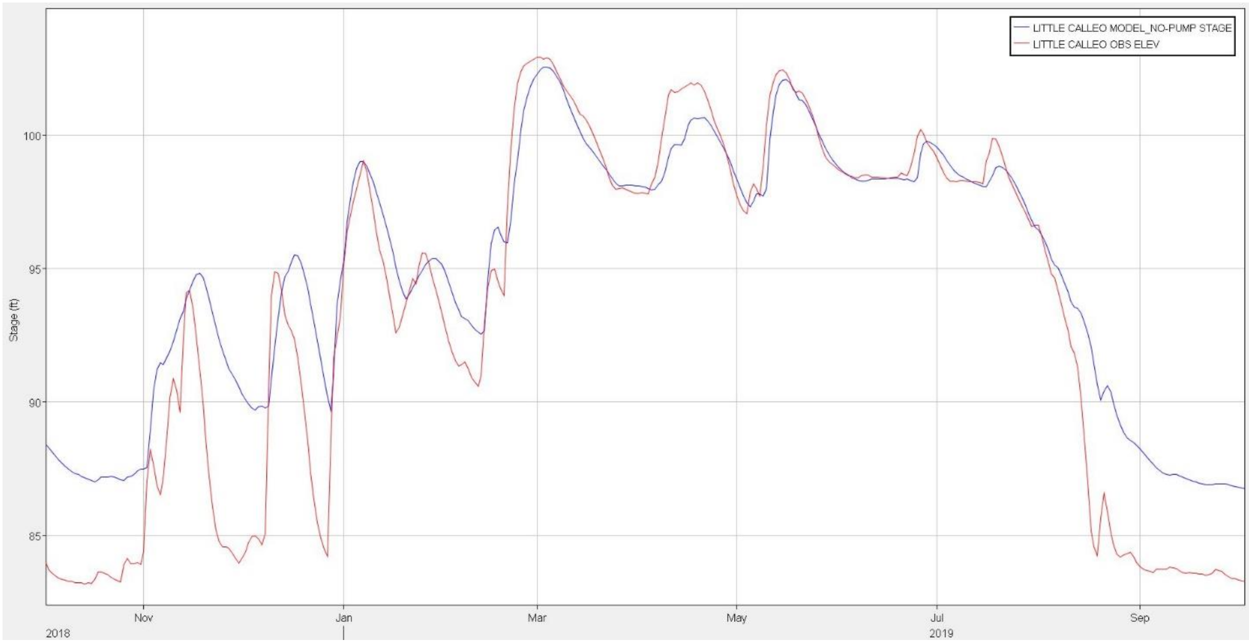

Figure 2-73. Big Sunflower at Little Calleo 2019 Calibration.

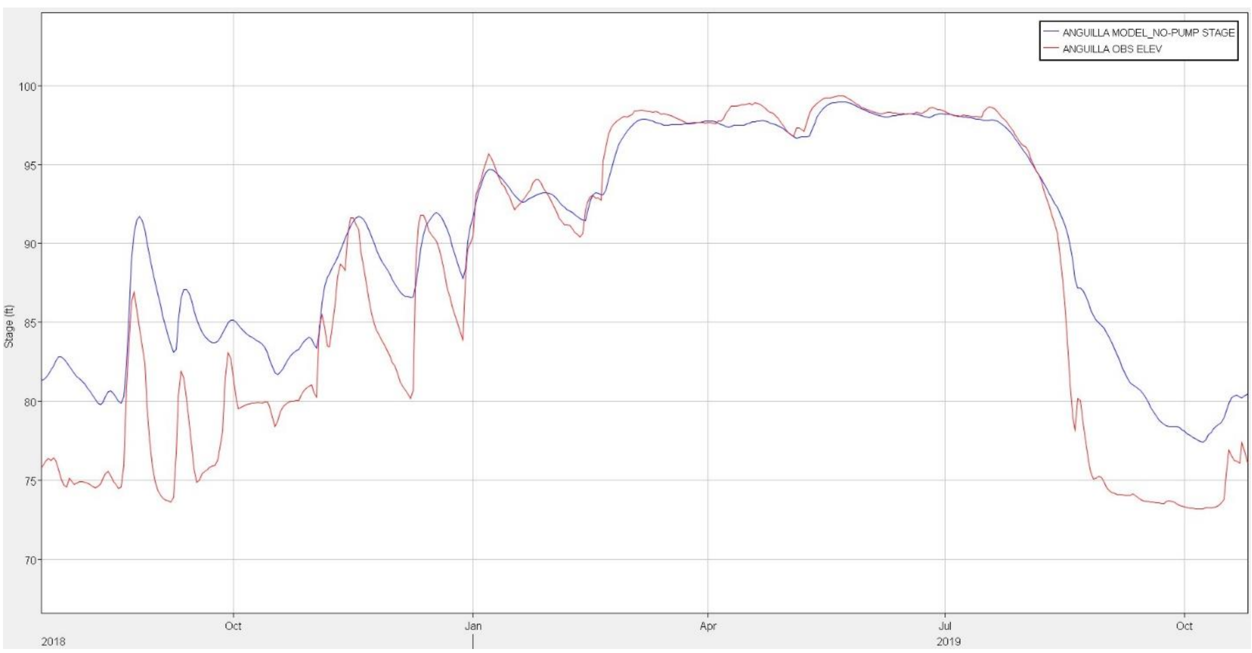

Figure 2-74. Big Sunflower at Anguilla 2019 Calibration.

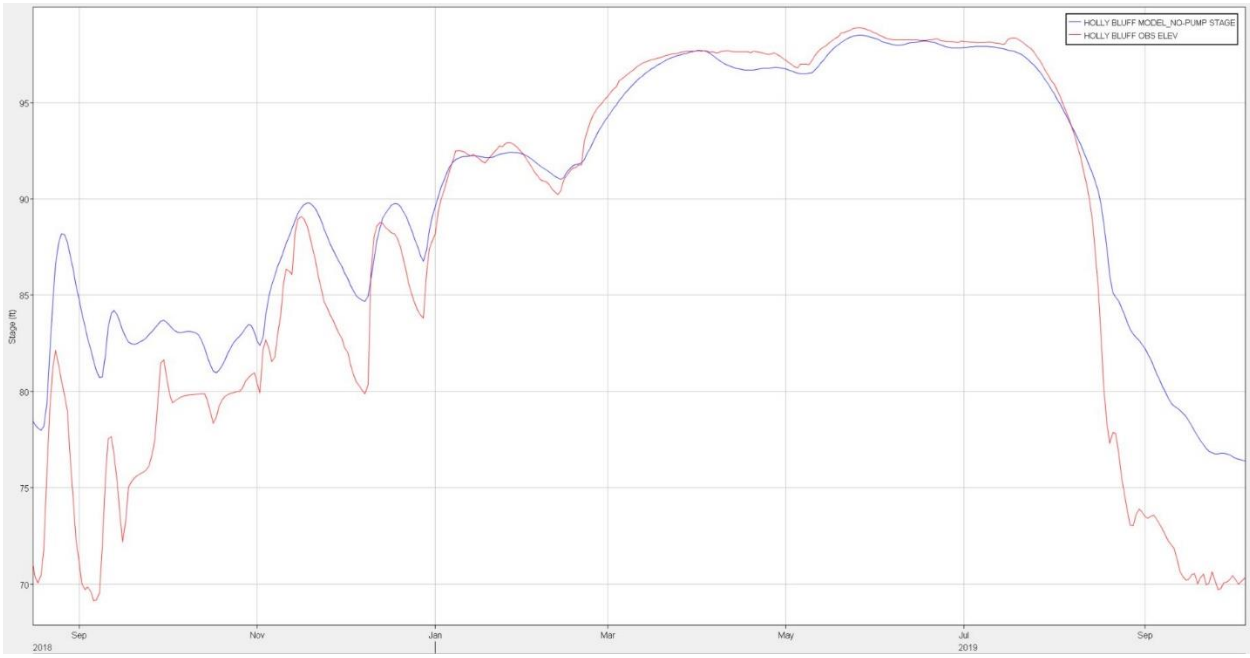

Figure 2-75. Big Sunflower at Holly Bluff 2019 Calibration.

<u>Validation</u>

148. Validation runs were performed on four years in addition to the calibration runs. These years included 1983, 1997, 2005, and 2010, and ensured the calibration parameters were not falsely skewing the data to appear accurate. Once the model was calibrated and verified, the "Period-of-record" run was made.

149. Figure 2-76 to Figure 2-87 shows some of the validation run results. The results from the verification runs show similar discrepancies to those that were identified from the calibration runs. However, validation was considered to be appropriate because the results at Steele Bayou and Little Sunflower showed the same level of accuracy as the calibration runs. The timing between the calibration and validation results did slightly differ at Steele Bayou at Grace. However, after changing parameters in both the HMS and RAS models, it was concluded that the difference in timing was caused by errors in the timing of the precipitation data.

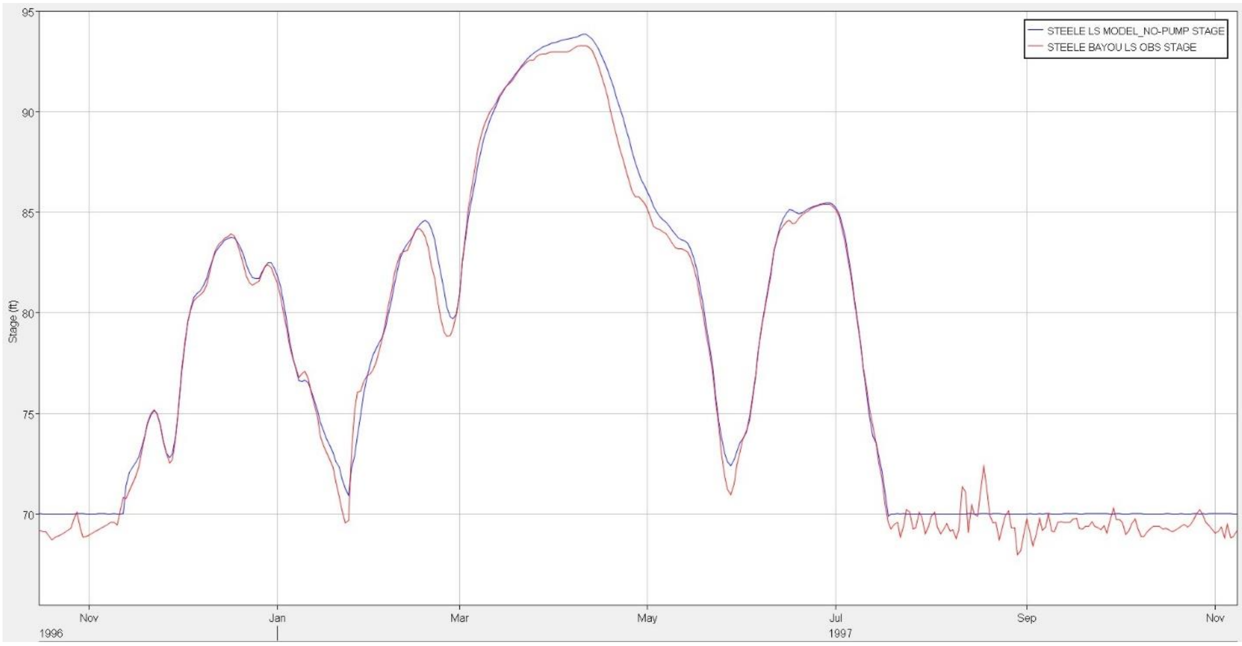

Figure 2-76. Steele Bayou Landside 1997 Validation.

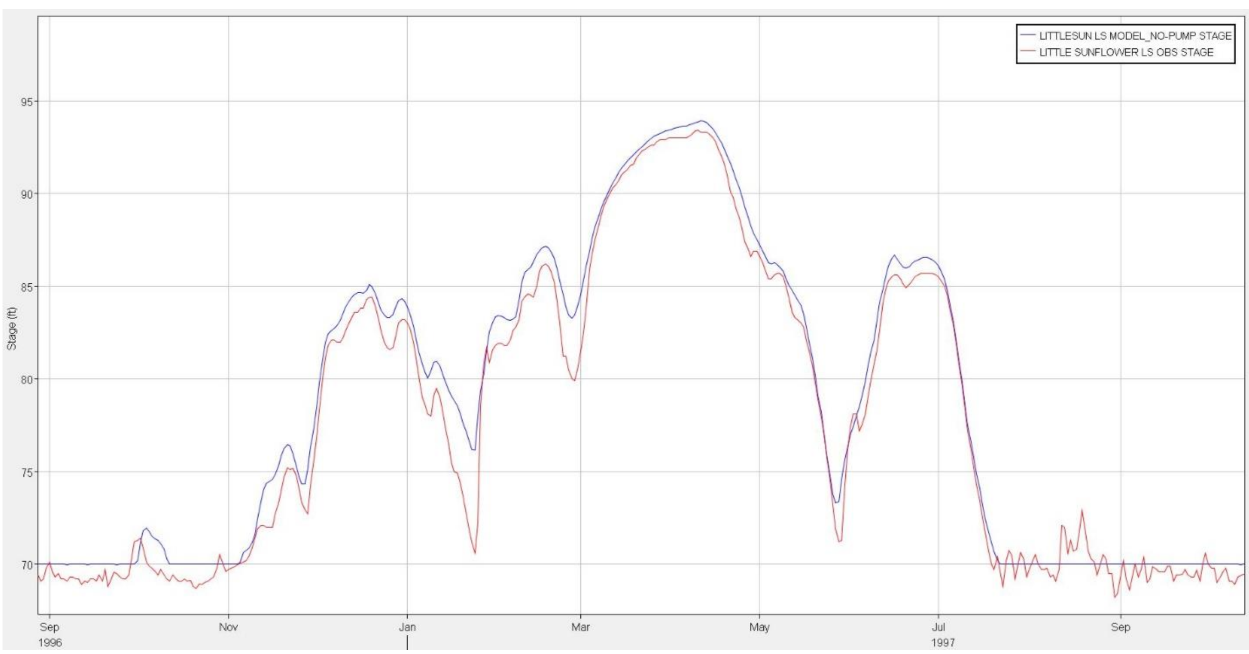

Figure 2-77. Little Sunflower Control Structure 1997 Validation.

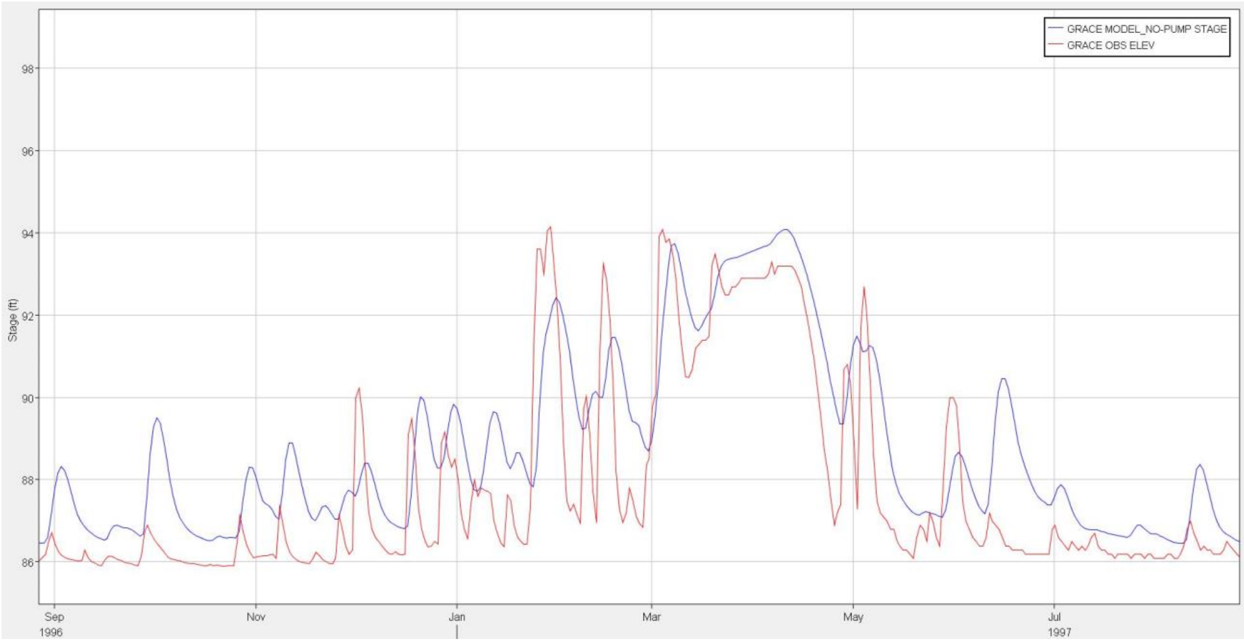

Figure 2-78. Steele Bayou at Grace 1997 Validation.

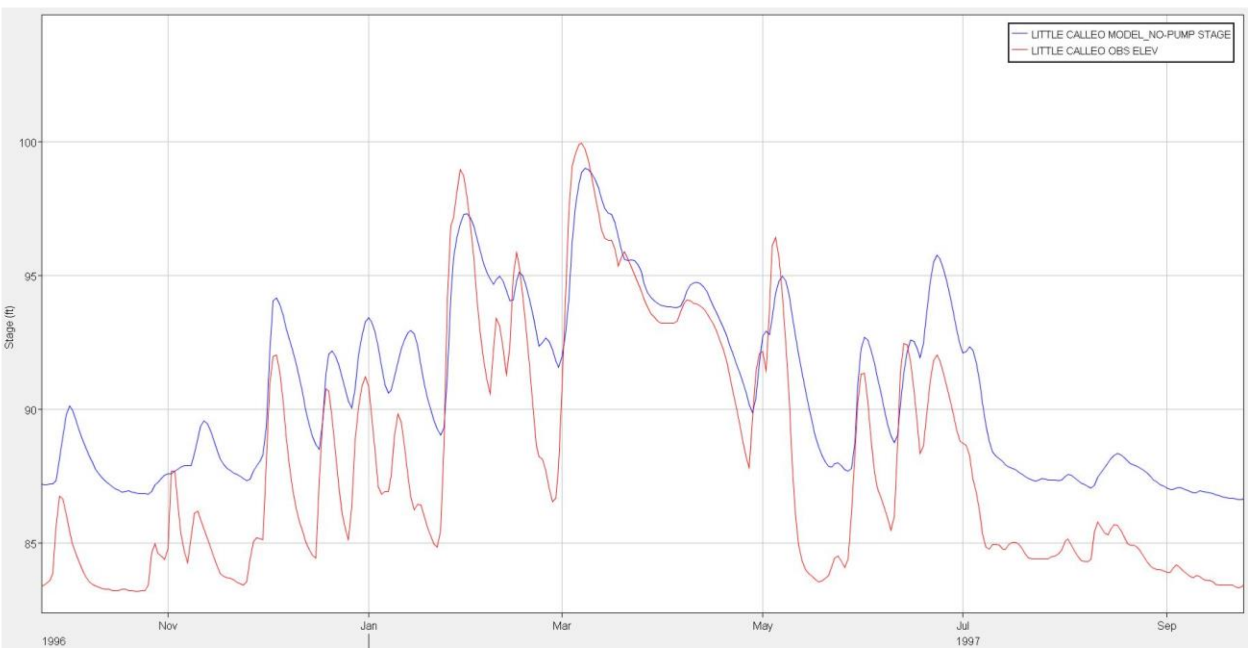

Figure 2-79. Big Sunflower at Little Calleo 1997 Validation.

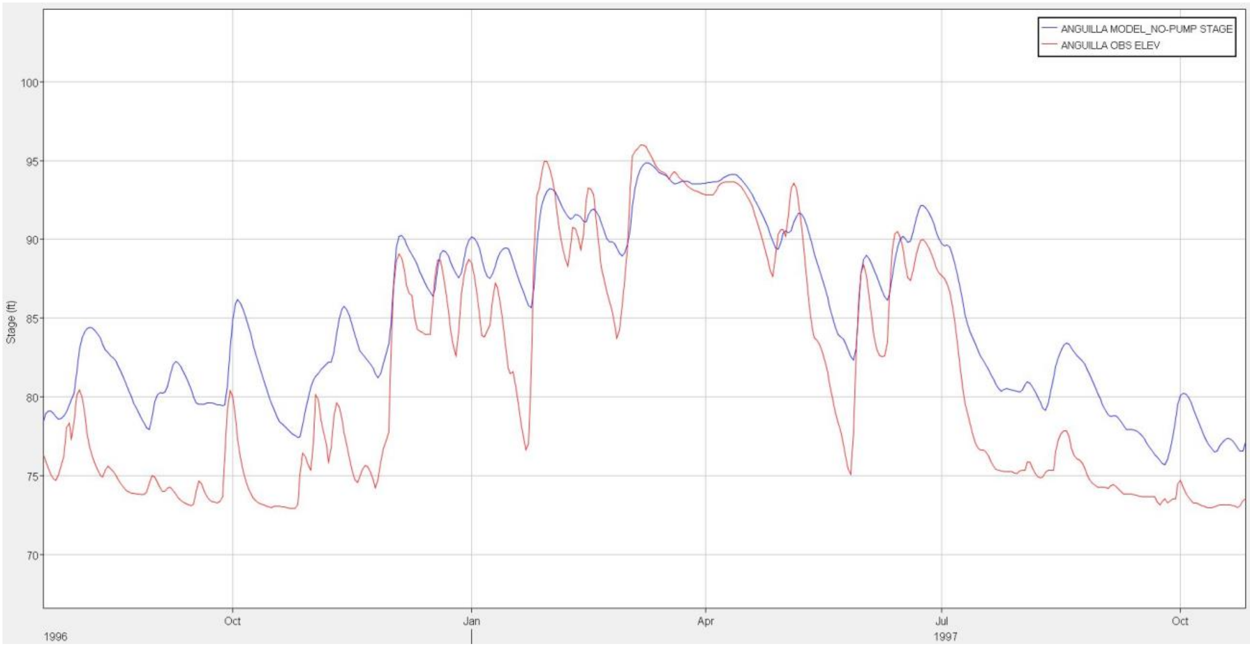

Figure 2-80. Big Sunflower at Anguilla 1997 Validation.

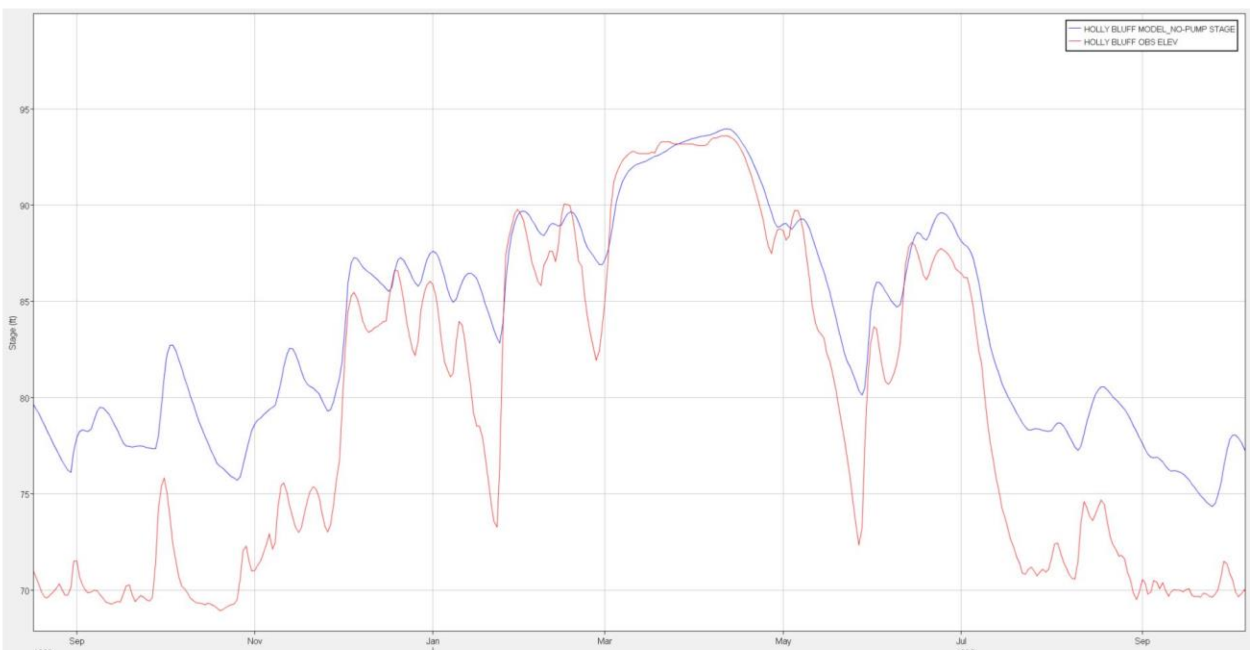

Figure 2-81. Big Sunflower at Holly Bluff 1997 Validation.

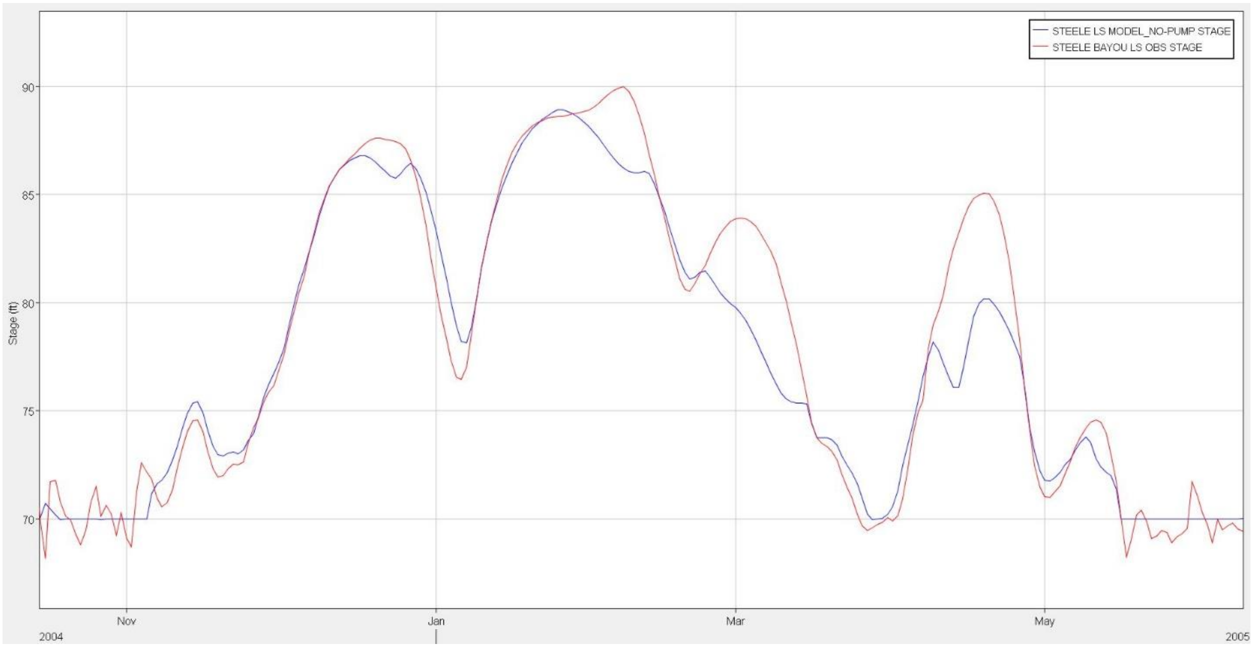

Figure 2-82. Steele Bayou Landside 2005 Validation.

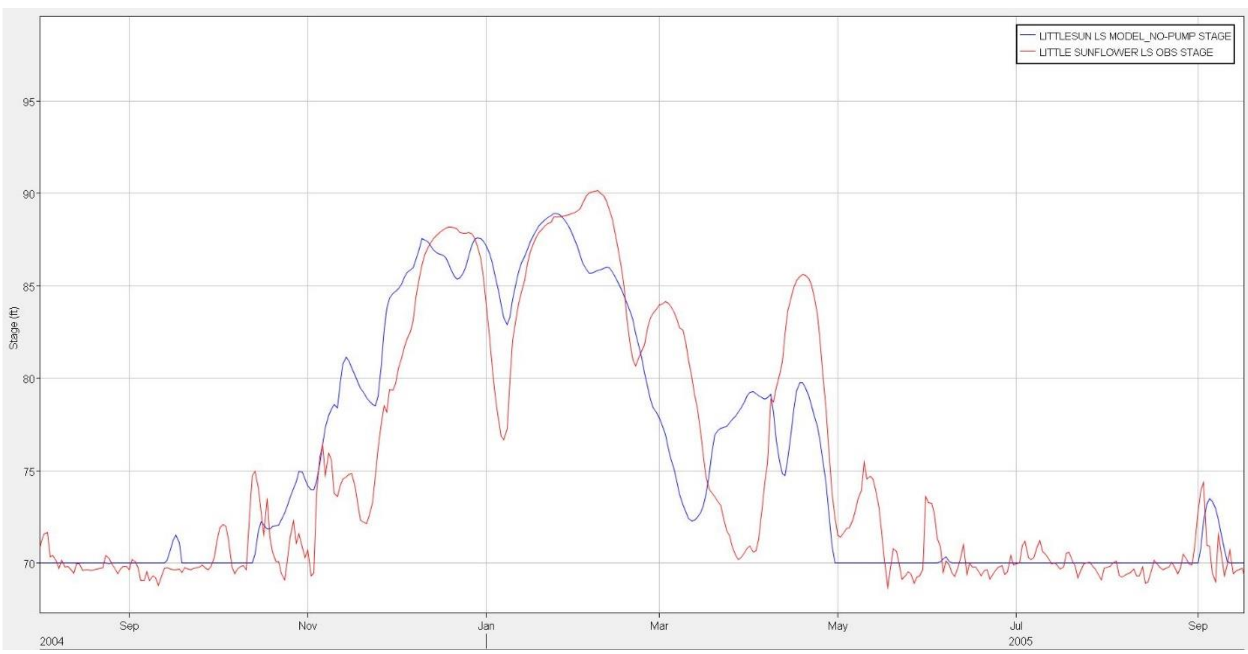

Figure 2-83. Little Sunflower Control Structure 2005 Validation.

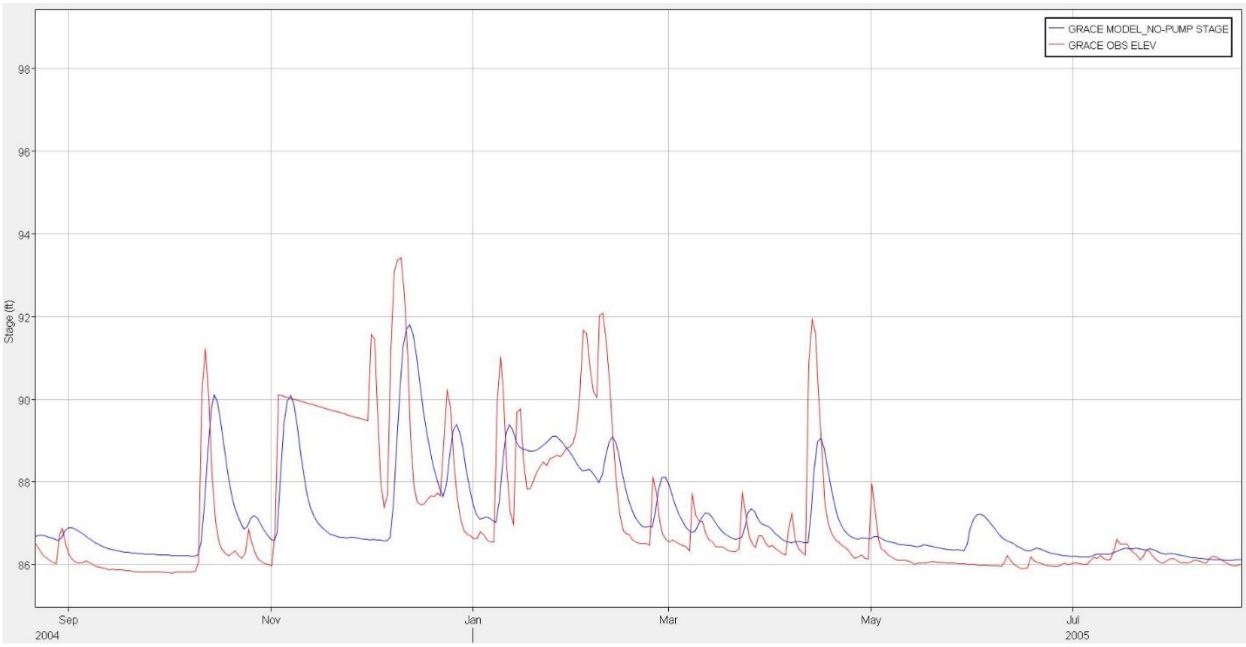

Figure 2-84. Steele Bayou at Grace 2005 Validation.

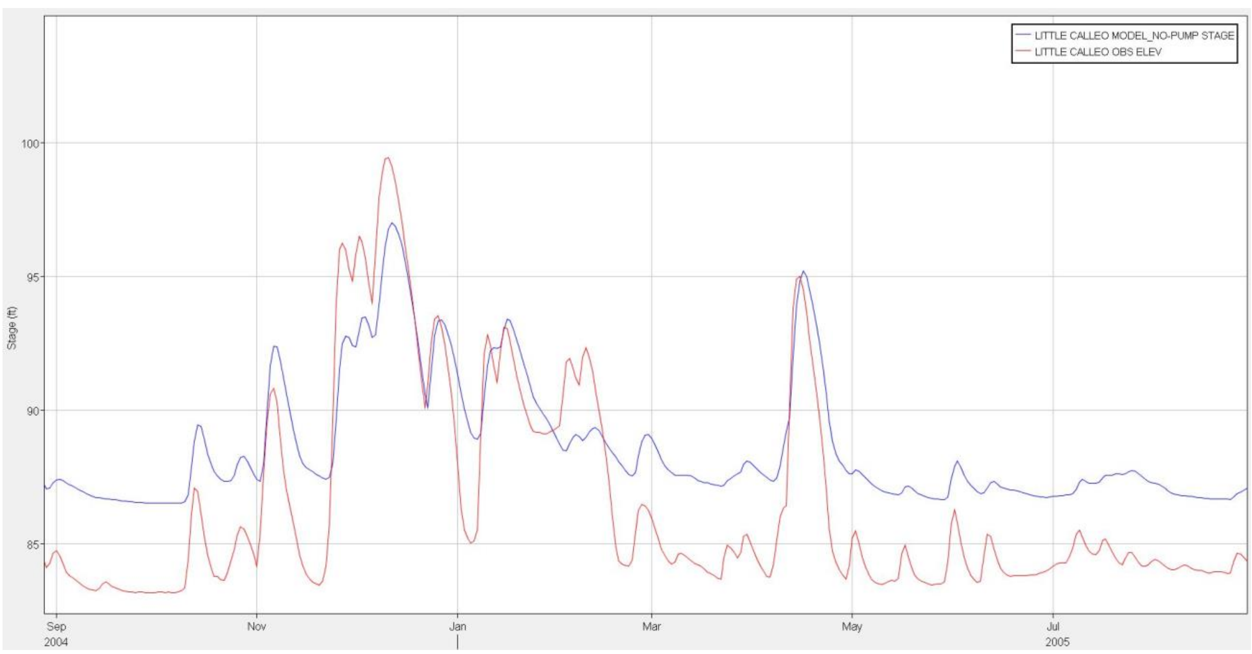

Figure 2-85. Big Sunflower at Little Calleo2005 Validation.

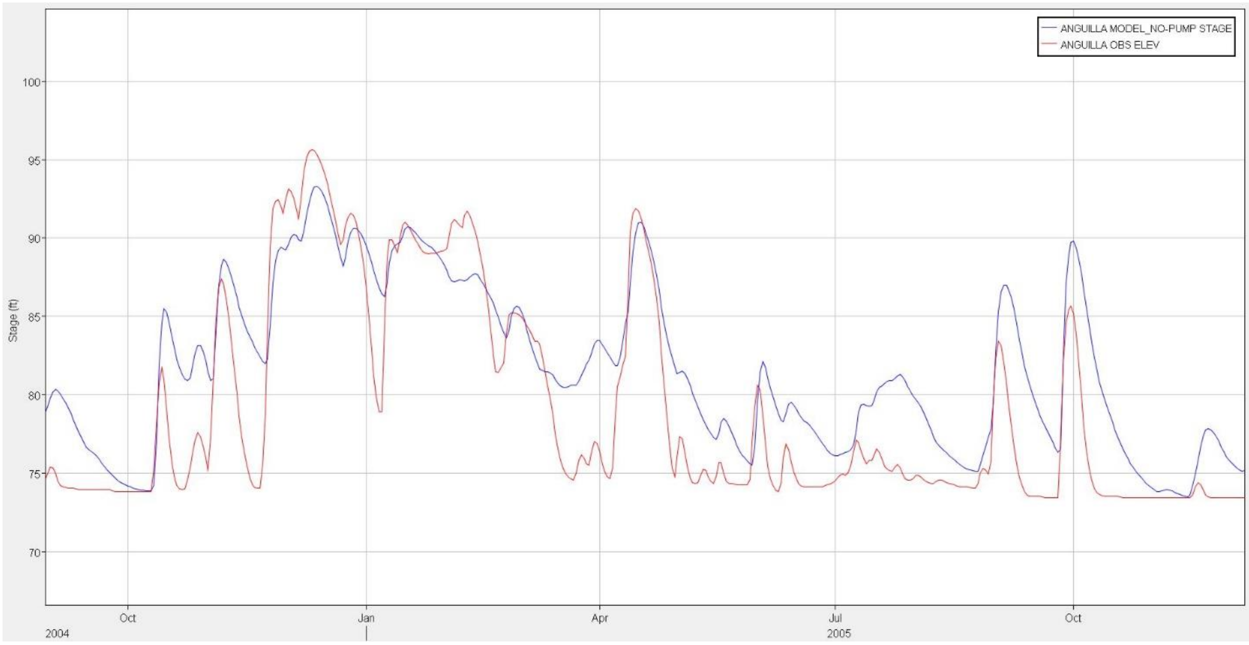

Figure 2-86. Big Sunflower at Anguilla 2005 Validation.

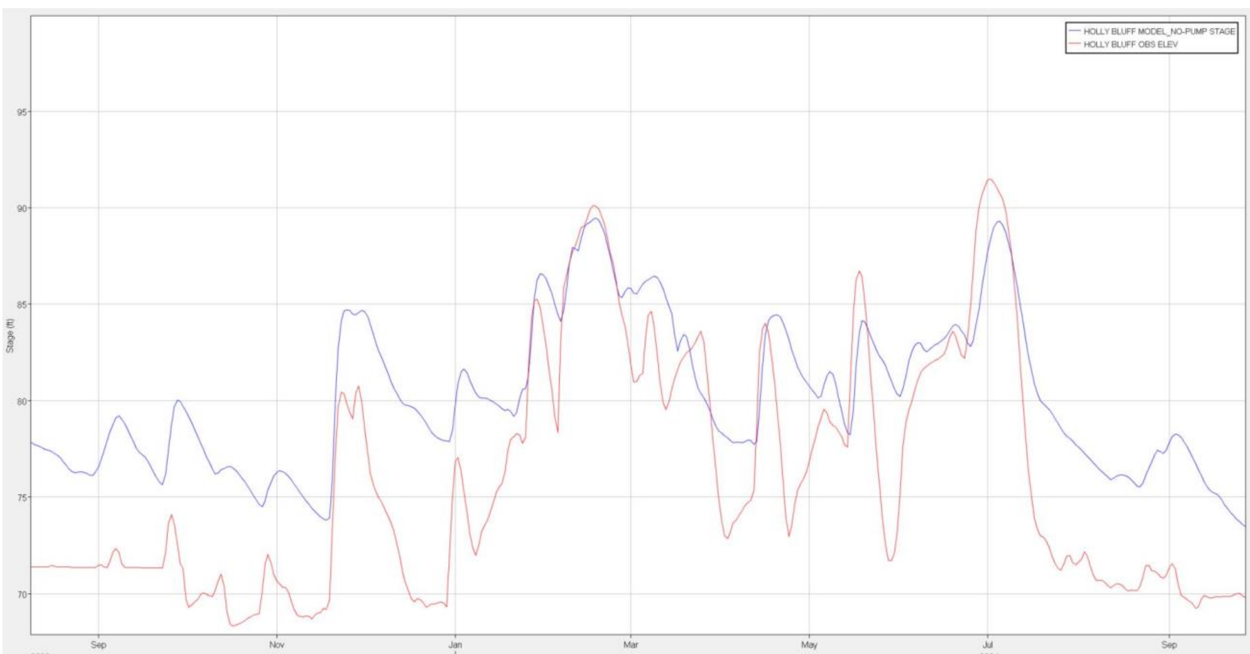

Figure 2-87. Big Sunflower at Holly Bluff 2005 Validation.

<u>Sensitivity</u>

150. The sensitivity of the model to Manning's "n" values and precipitation inputs were tested to determine which had more of an impact on calibration. The Manning's "n" value for cultivated crop had the largest impact on results, relative to other Manning's values because it is the most prevalent value throughout the area. The Manning's "n" value for cultivated crop was increased from an original value of 0.03 to 0.05 to slow the flow of water after it overtopped the main

channel area. The Manning's "n" of the channels were also tested. These values did not significantly impact calibration results and were rarely changed after initial runs.

151.The precipitation data had a more significant impact on calibration results. Much of the precipitation data was obtained with a degree of uncertainty. The high level of uncertainty, associated with the precipitation data, made model calibration more difficult to recreate observed stages, particularly for the 2004 event. Due to this level of uncertainty, the period-of-record was run using results based on two different precipitation datasets, precipitation from gages stations from NCEI and gridded Stage IV precipitation from UCAR. Refer to the 'Hydrologic Model Setup' section above for more information on precipitation calculations. In some cases, weekly precipitation values showed as much as a 40% variation between the two precipitation datasets. These results proved that precipitation was the driving force behind the uncertainty within the model results. However, the level of uncertainty between the two precipitation datasets is unknown.

Period-of-record Runs

152.The period-of-record (POR) was considered to be from 01 January 1978 to 31 December 2019. The POR began on 01 January 1978, after the Yazoo Backwater Levee System and the Little Sunflower Control System was completed, which eliminated the need to use simulated data for base conditions. To decrease the run time and the possibility of data loss, the POR was divided into 5-year sections, with the beginning of each section including the last two months of the previous section to allow the model to properly warm-up.

RESULTS

153.Water surface elevations (WSEL) were taken from six gage locations throughout the basin: Steele Bayou at Grace, Steele Bayou Control Structure landside, Little Sunflower Control Structure landside, Big Sunflower at Little Calleo, Anguilla, and Holly Bluff. Observed stages were used as final "without-pump" results. For "with-pump" final results, the "with-pump" model run outputs were subtracted from the "without-pump" model run outputs to determine a relative impact of the pumps on the water level. The resulting dataset was then subtracted from the observed dataset. This relative difference method allowed for an analysis specifically of the impacts of the pumps, while minimizing the uncertainty of model calibration errors. The "with-pump" results were further screened to remove stages when Steele Bayou landside and Little Sunflower landside gages were below 87.0 feet, MSL, during times when control structures were not opened and releasing flow. This modification was added to replicate the pump station design of maintaining 87.0 feet, MSL, or higher. The Figure 2-88 through Figure 2-105 shows a comparison of the observed dataset and the resulting "with-pump" dataset. Gages further upstream experienced less of a difference from the pump station than the gages at the control structures. Upstream gages also experienced less of an impact when the flooding was primarily headwater flooding versus backwater flooding.

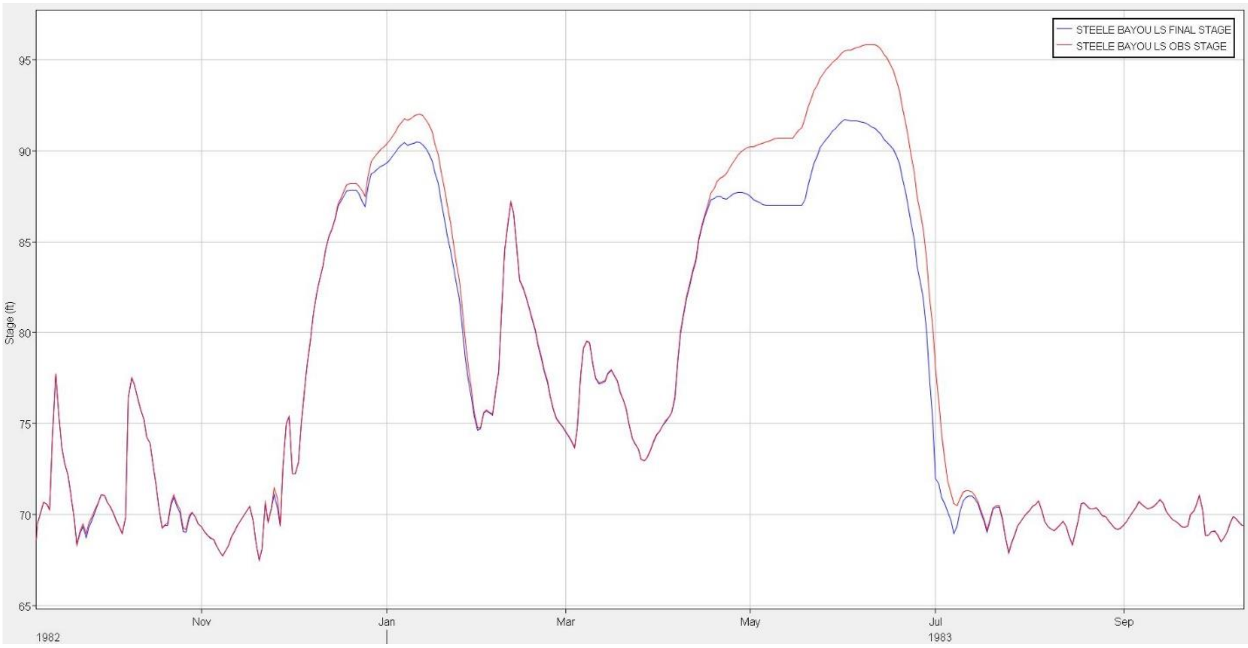

Figure 2-88. Steele Bayou Control Structure Landside 1983 Comparison.

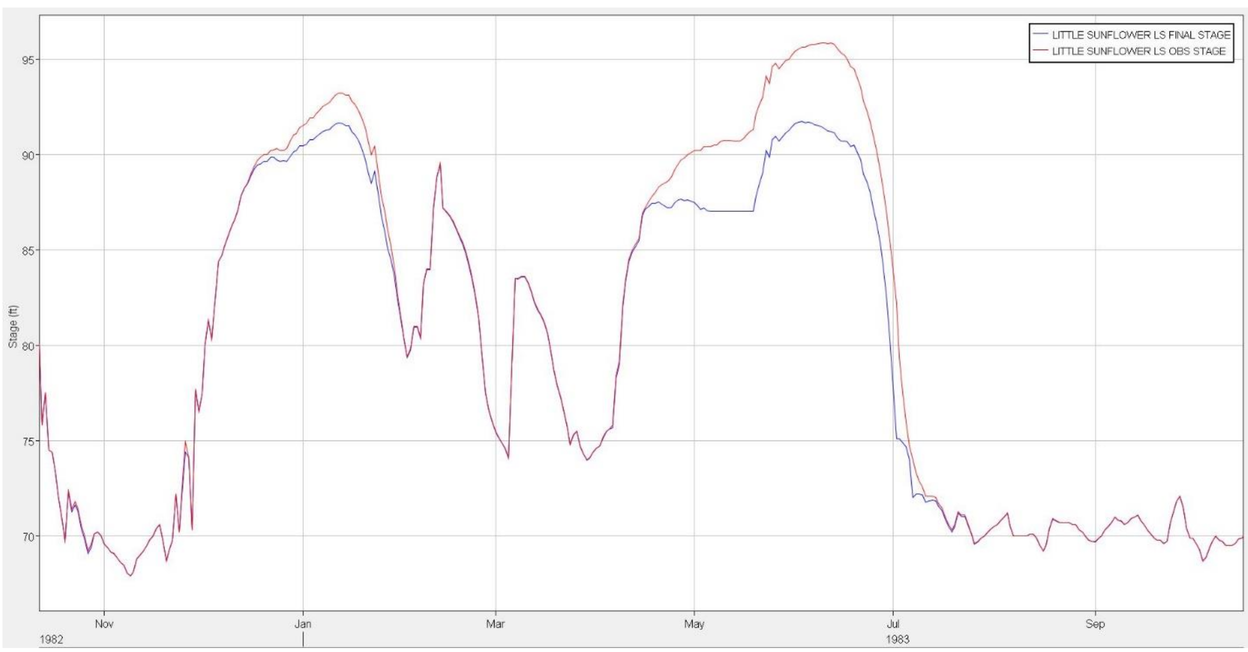

Figure 2-89. Little Sunflower Control Structure Landside 1983 Comparison.

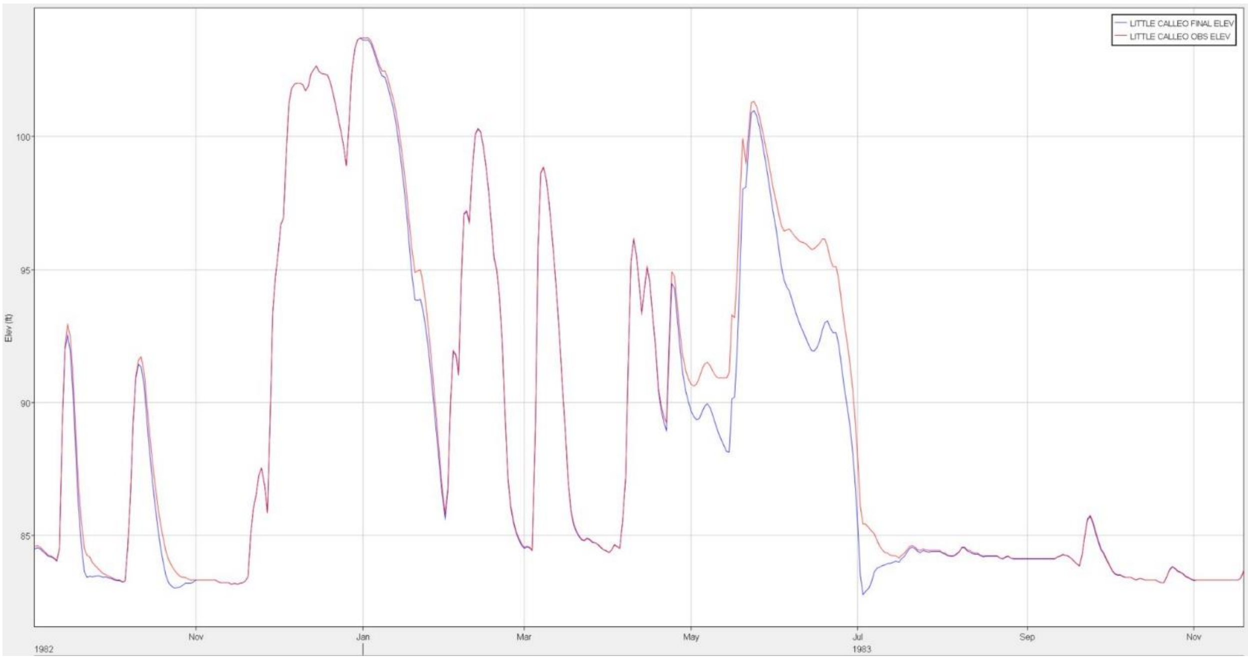

Figure 2-90. Big Sunflower at Little Calleo 1983 Comparison.

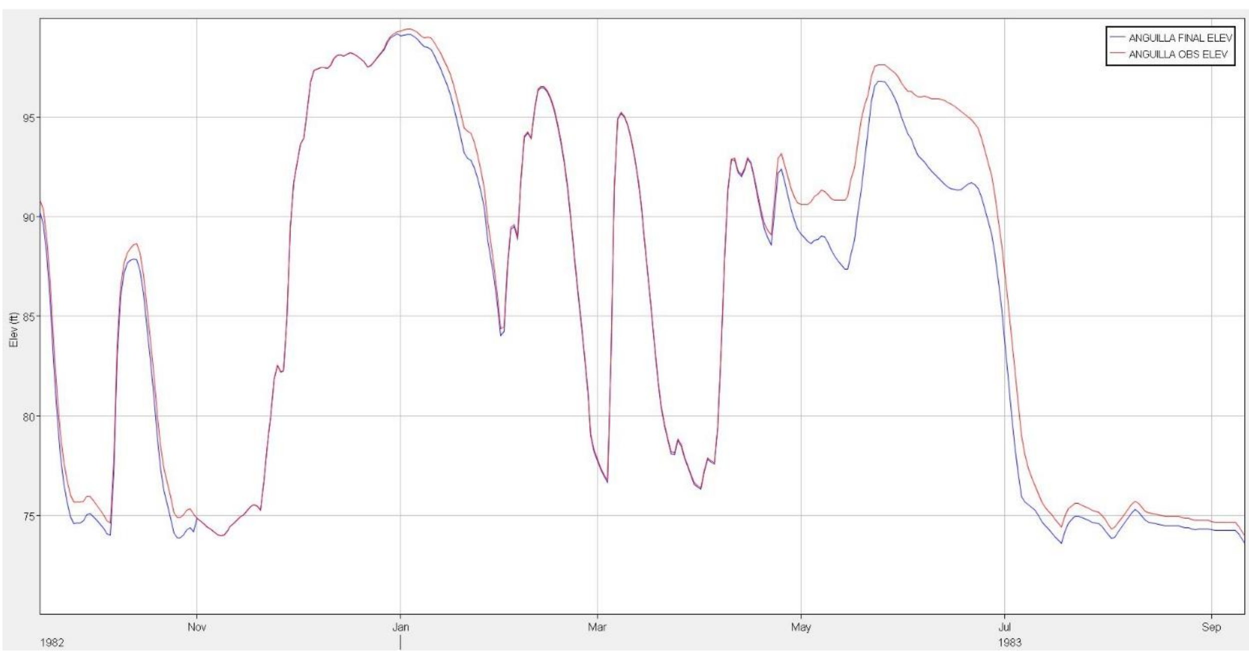

Figure 2-91. Big Sunflower at Anguilla 1983 Comparison.

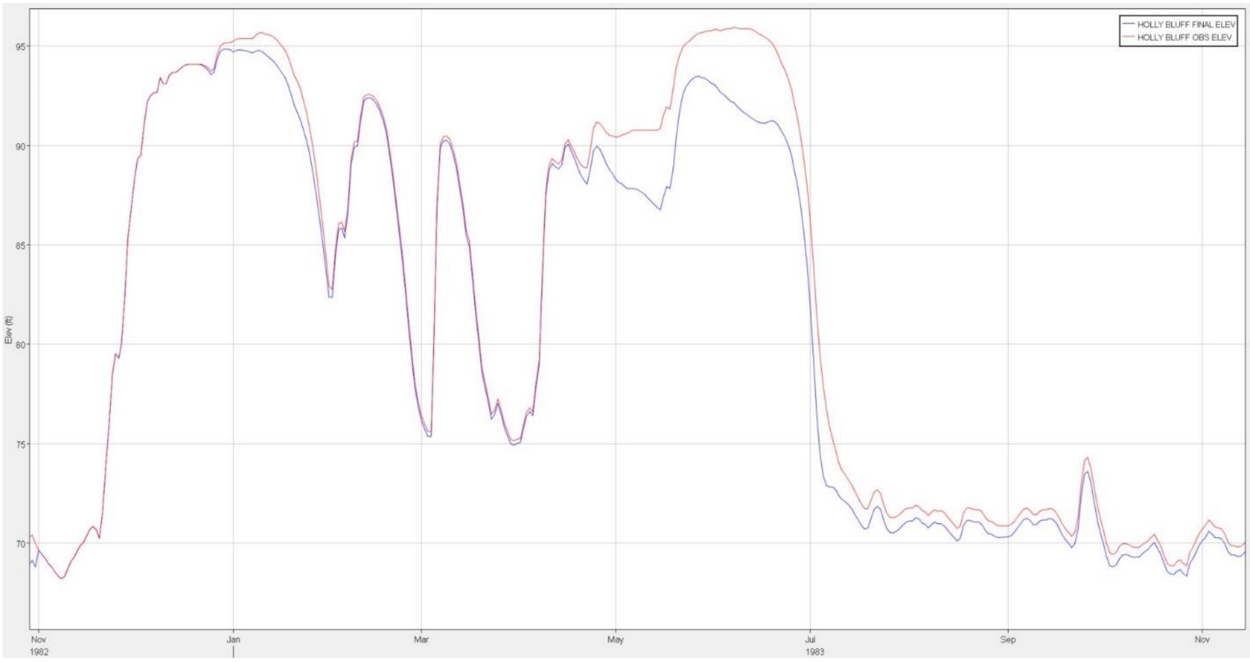

Figure 2-92. Big Sunflower at Holly Bluff 1983 Comparison.

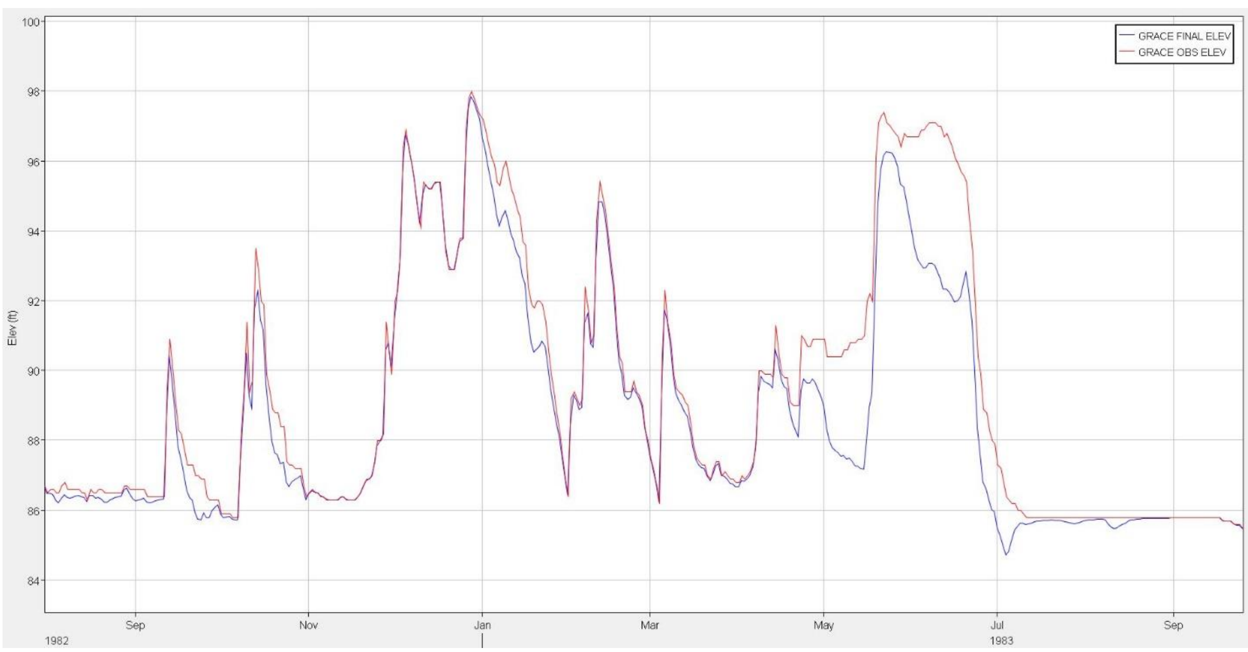

Figure 2-93. Steele Bayou at Grace 1983 Comparison.

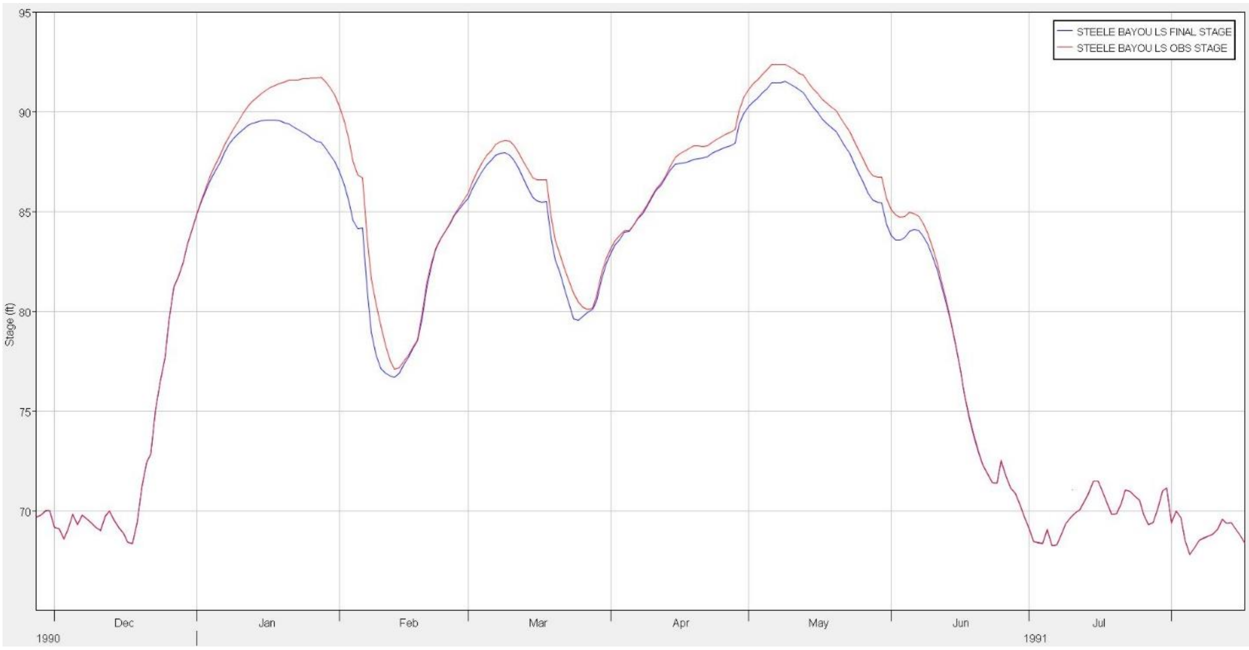

Figure 2-94. Steele Bayou Landside 1991 Comparison.

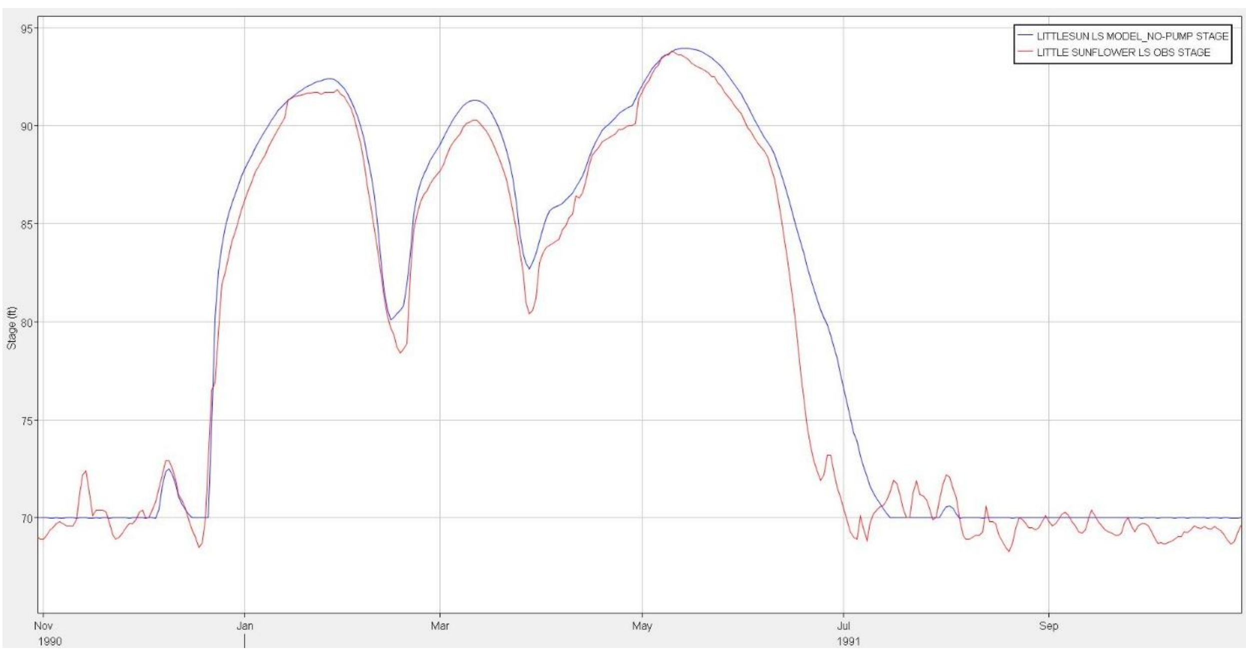

Figure 2-95. Little Sunflower Landside 1991 Comparison.

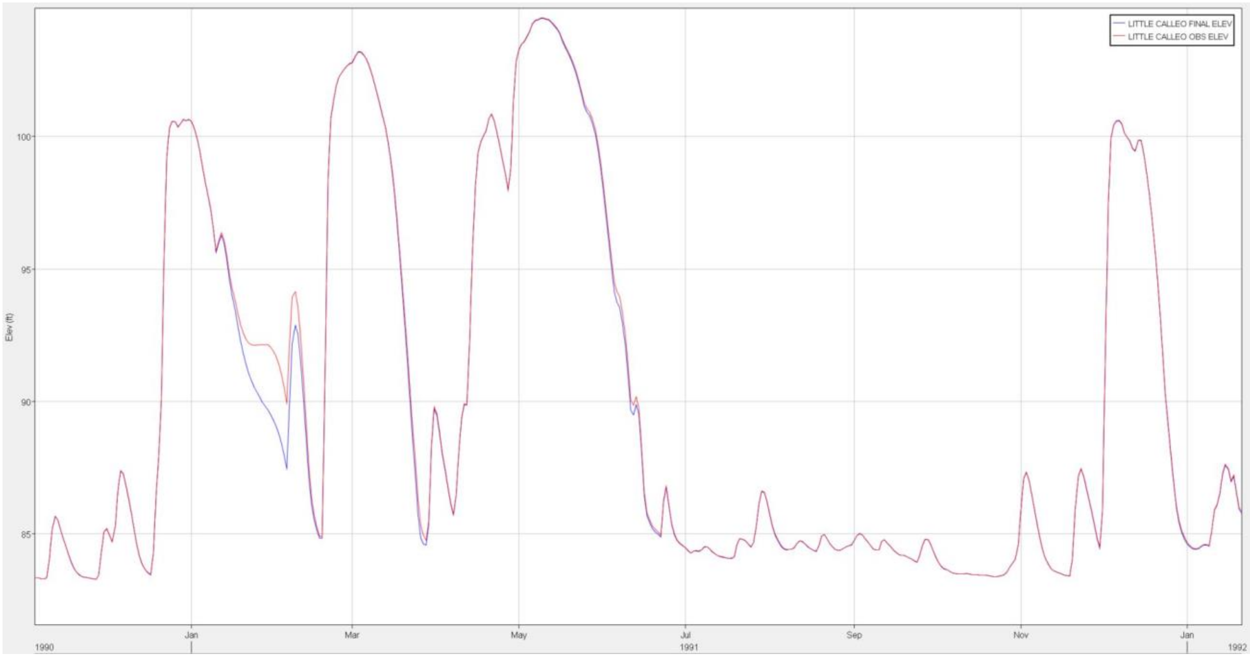

Figure 2-96. Big Sunflower at Little Calleo 1991 Comparison.

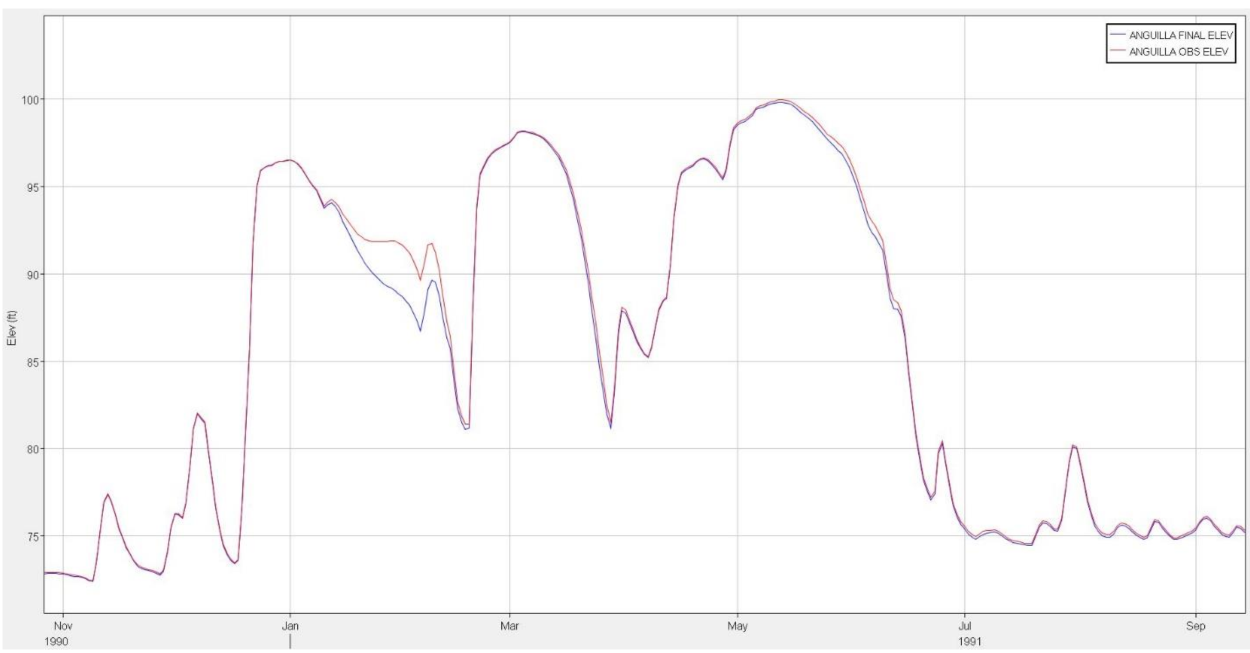

Figure 2-97. Big Sunflower at Anguilla 1991 Comparison.

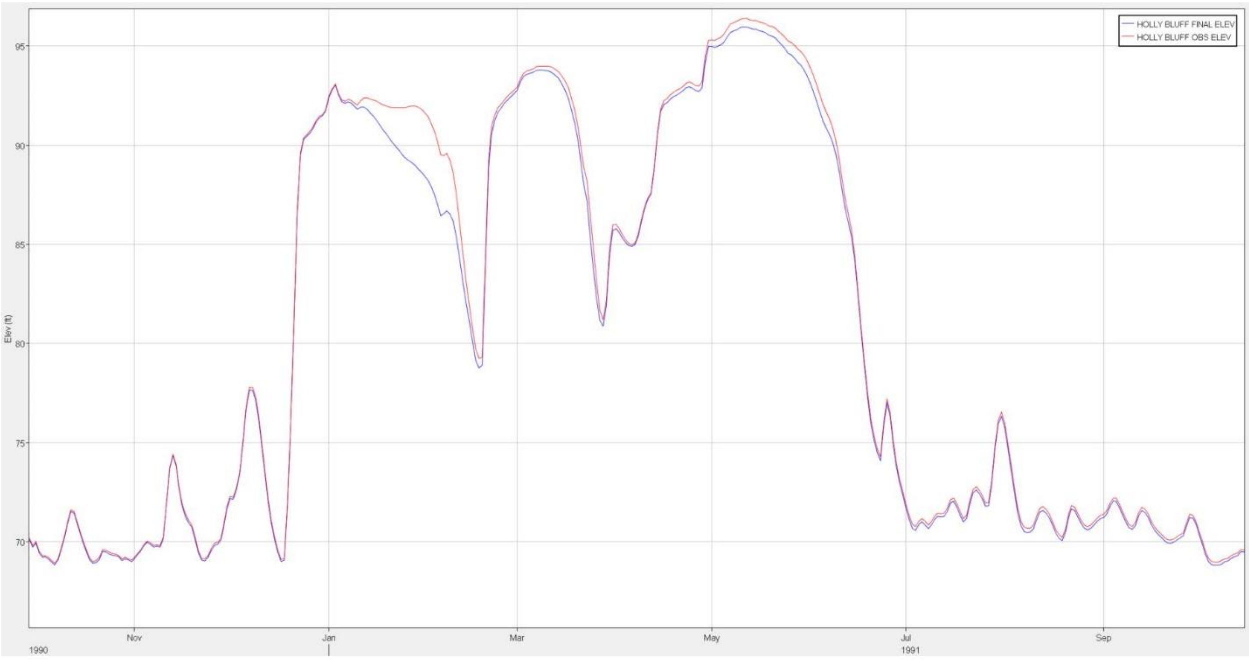

Figure 2-98. Big Sunflower at Holly Bluff 1991 Comparison.

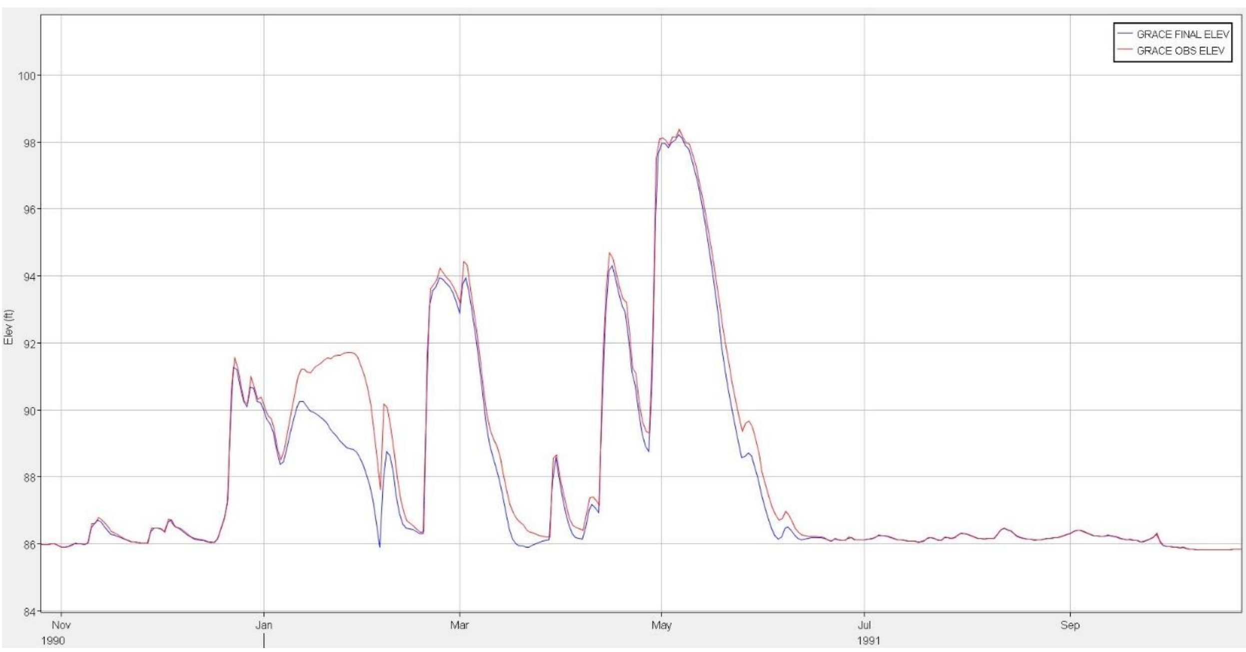

Figure 2-99. Steele Bayou at Grace 1991 Comparison.

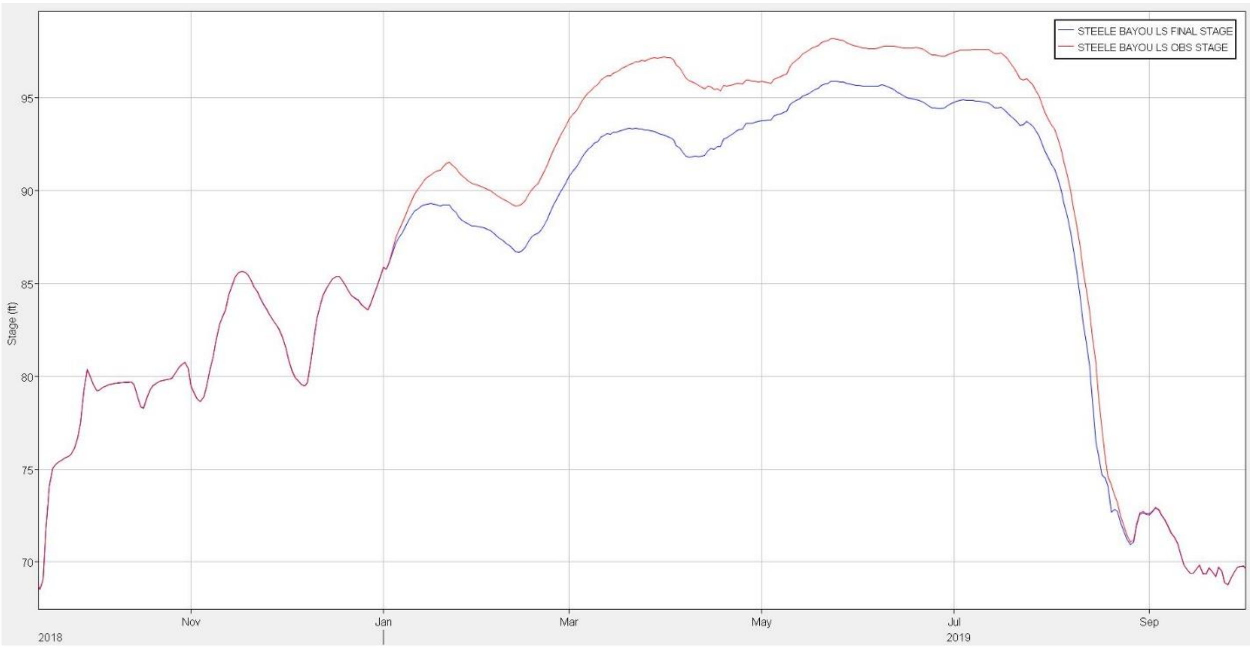

Figure 2-100. Steele Bayou Landside 2019 Comparison.

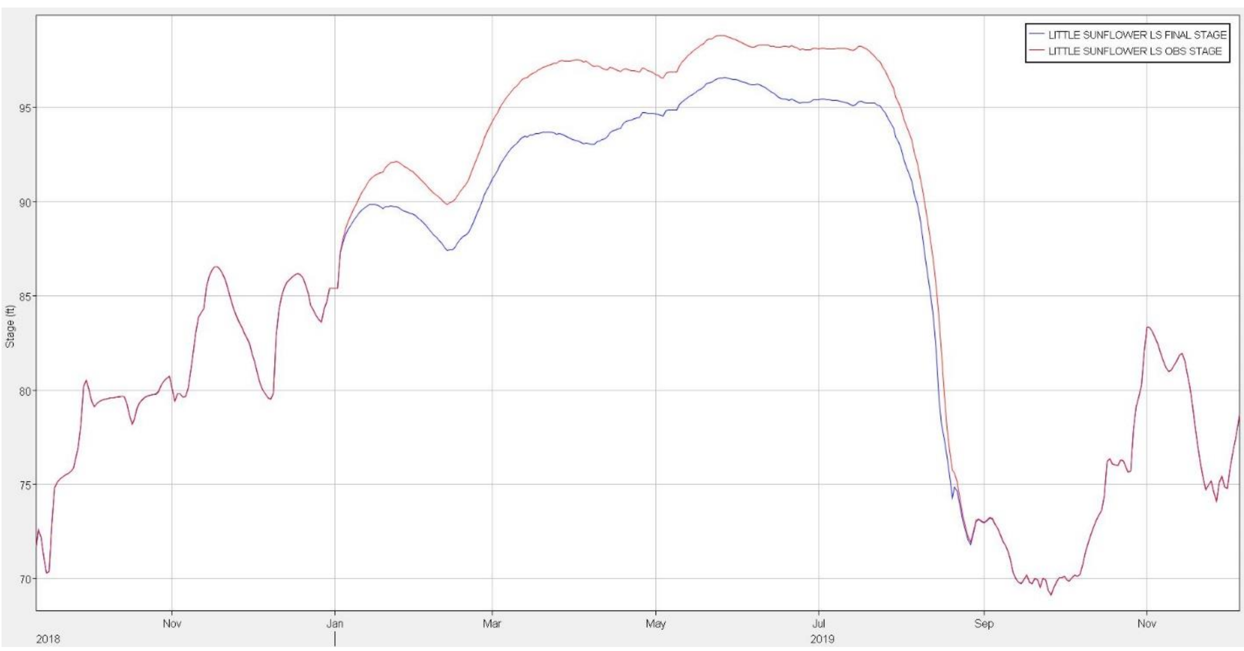

Figure 2-101. Little Sunflower Landside 2019 Comparison.

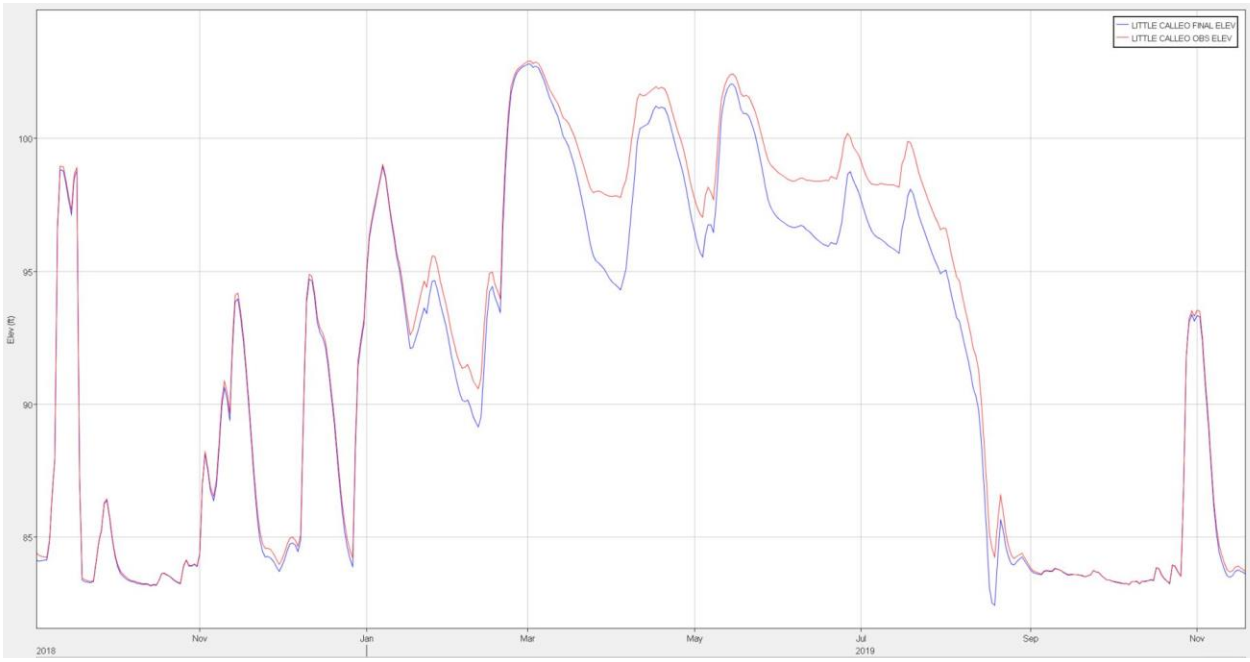

Figure 2-102. Big Sunflower at Little Calleo 2019 Comparison.

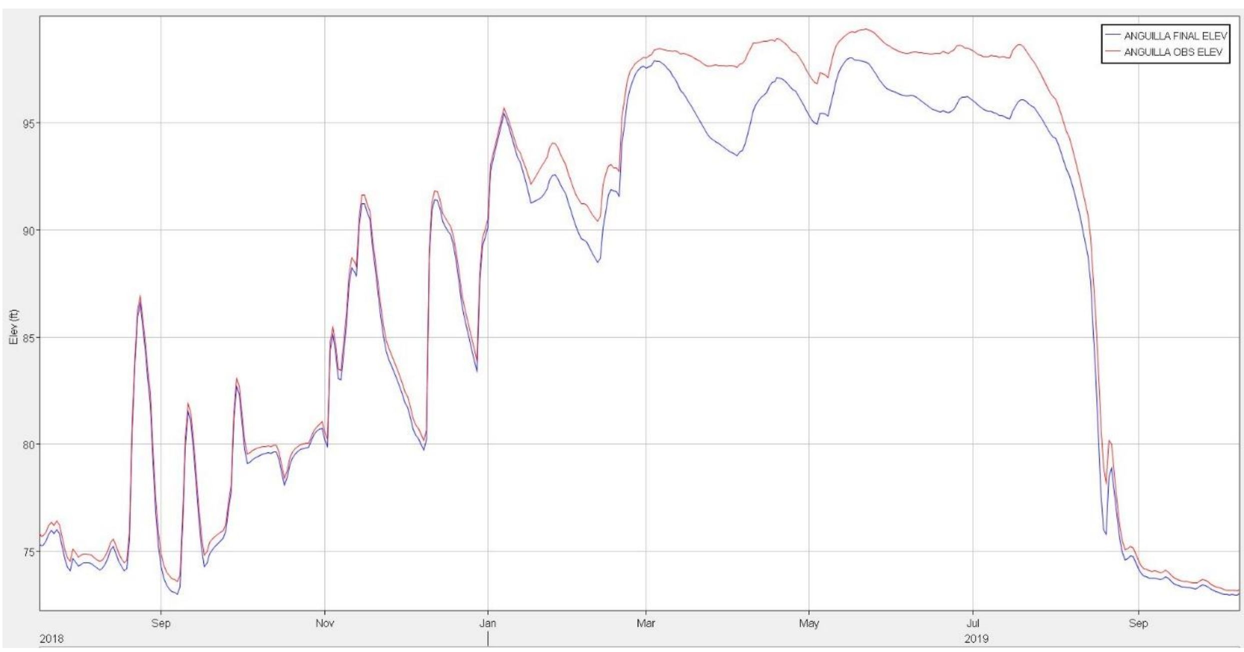

Figure 2-103. Big Sunflower at Anguilla 2019 Comparison.

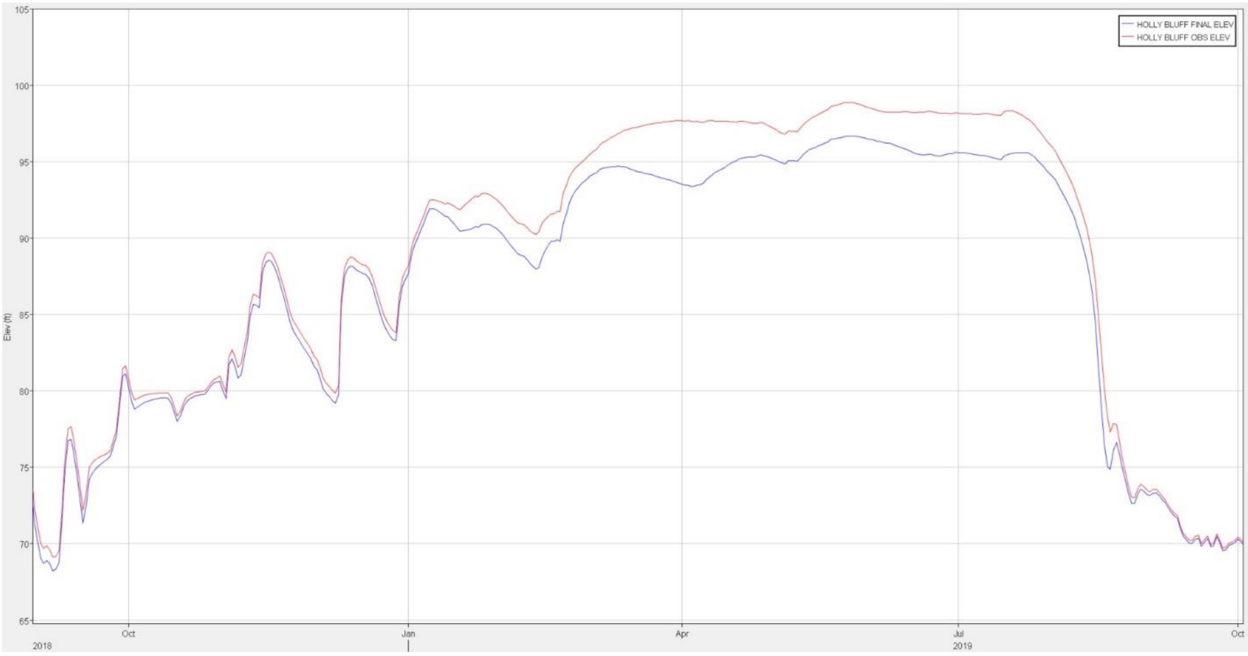

Figure 2-104. Big Sunflower at Holly Bluff 2019 Comparison.

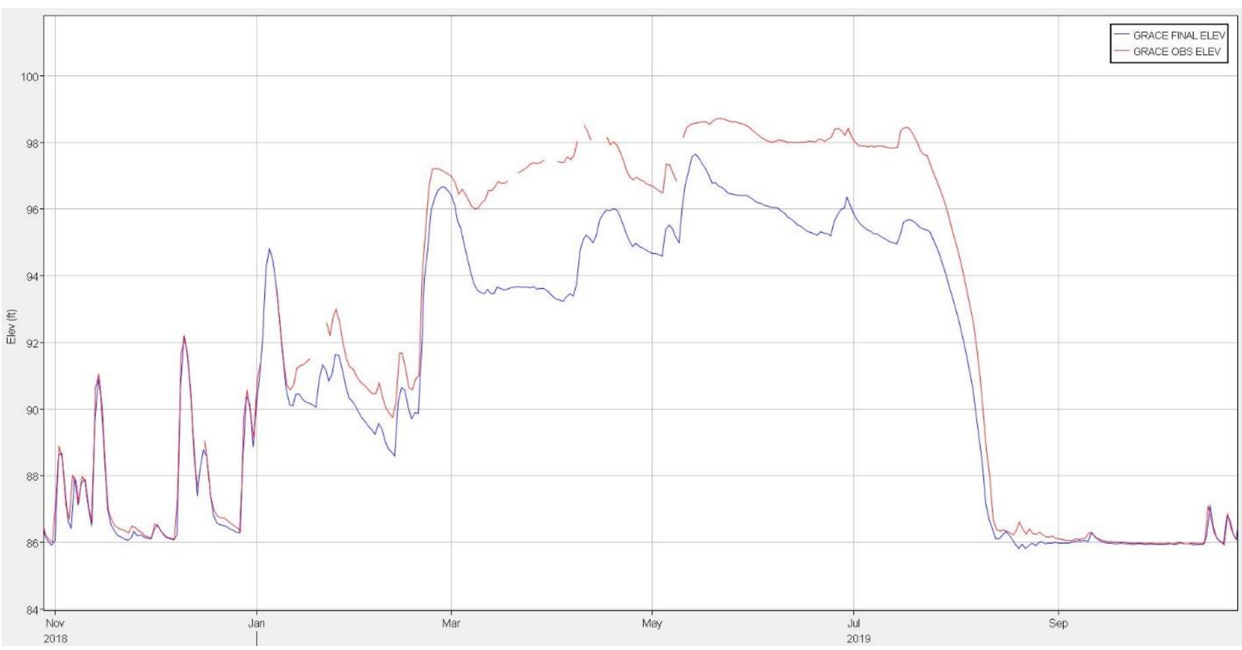

Figure 2-105. Steele Bayou at Grace 2019 Comparison.

FLOOD FREQUENCY ANALYSIS

154. Flood frequencies can be calculated with two different methods, annual and partial series. Both methods give similar results for the low frequency events like the 50- or 100-year floods, but the partial series give a much more accurate estimate of high frequency events like the 1- and 2-year floods. The annual method uses the single highest peak in the period of a year. The period can either be the calendar year or the water year. The method utilizing the water year is generally preferred. The partial series method utilizes the peaks over threshold method to filter

the POR to obtain all of the peaks which exceed the threshold requirements. The threshold values used is this study were: the minimum peak elevation was greater than or equal to the annual series 1.25 year elevation, a minimum of 14 days between the peaks, and a minimum change in elevation of three feet. This provided a partial series of 59 to 76 peaks and the top 42 (number of years in the POR) peaks to calculate the flood frequency elevations. The Hydrologic Engineering Center (HEC) Statistical Software Package (SSP) Version 2.2 was used to calculate the annual and partial series flood frequency elevations. SSP uses the methods outlined in Bulletin 17C, Guidelines for Determining Flood Flow Frequency, May, 2019. The annual stage frequencies were calculated with the General Frequency Analysis module; while the partial frequencies were calculated with the Distribution Fitting Analysis module after the POR stages were filtered using the threshold values listed above. Because the major environmental and economic impacts of this project result from high frequency events the partial series method was used in this study. The results of both the annual and partial series flood frequency analyses are available on the project website in an Excel file titled YBW_SSP_RAS_Final.xlsx. The base condition partial series flood frequency elevations are reported in Table 2-22.

Table 2-22. Base Condition Partial Frequency Elevations

Filtered using 1.25 year (Annual) as min and top 42 were used						
Flood Frequency	**Little Callao**	**Anguilla**	**Holly Bluff**	**Little Sunflower Landside**	**Grace**	**Steele Bayou Landside**
0.2	104.72	101.15	100.88	100.57	99.19	99.85
0.5	104.57	100.76	99.79	99.54	99.04	99.08
1	104.39	100.40	98.90	98.63	98.88	98.35
2	104.16	99.96	97.94	97.60	98.65	97.47
5	103.69	99.25	96.57	96.00	98.18	96.01
10	103.18	98.58	95.45	94.58	97.65	94.63
20	102.46	97.79	94.25	92.97	96.89	92.96
50	101.07	96.48	92.53	90.49	95.38	90.20
80	100.08	95.68	91.58	89.04	94.28	88.49
90	99.79	95.46	91.34	88.65	93.96	88.02
95	99.65	95.36	91.23	88.47	93.80	87.80
99	99.55	95.28	91.14	88.33	93.68	87.63

155. The partial frequencies for the with-project condition were calculated in a similar manner, and the results are presented in Table 2-23.

Table 2-23. With-project Partial Frequency Elevations

Filtered using 1.25 year (Annual) as min and top 42 were used						
Flood Frequency	**Little Callao**	**Anguilla**	**Holly Bluff**	**Little Sunflower Landside**	**Grace**	**Steele Bayou Landside**
0.2	104.75	100.50	98.05	98.64	98.90	97.48
0.5	104.58	100.18	97.36	97.09	98.74	96.23
1	104.39	99.87	96.76	95.90	98.56	95.23
2	104.14	99.50	96.11	94.70	98.30	94.17
5	103.65	98.87	95.12	93.09	97.81	92.67
10	103.11	98.27	94.29	91.87	97.26	91.46
20	102.37	97.54	93.36	90.63	96.50	90.18
50	100.96	96.33	91.97	88.97	95.01	88.39
80	99.98	95.57	91.18	88.11	93.95	87.41
90	99.69	95.36	90.98	87.90	93.64	87.16
95	99.56	95.27	90.88	87.80	93.49	87.04
99	99.45	95.19	90.81	87.72	93.38	86.96

RISK AND UNCERTAINTY

156. A risk-based analysis was performed on the computed stage-frequency curves developed at the Steele Bayou and the Little Sunflower structures as outlined in EC 1105-2-205. These two gages were used in period-of-record-routing analysis from which stage-frequency curves were developed and utilized in the Economic Analysis of the SEIS.

157. The General Frequency Analysis (GF) module of the SSP software that was used to calculate the stage frequencies allows the user to select either a graphical or an analytical fit. The analytical method was used in this study. The GF module calculates the 95 percent confidence interval for each frequency. The 95 percent confidence intervals for the base and with-project conditions at the Steele Bayou and Little Sunflower gages are provided in Table 2-24 and Table 2-25 respectively below. The confidence intervals for all gages and distribution fittings are provided in the Excel file named YBW_SSP_RAS_Final.xlsx, which can be found on the Project Website.

Table 2-24. Base Confidence Intervals for the Steele Bayou and Little Sunflower Gages

Probability	Return Period	Little Sunflower LS			Steele Bayou LS		
		Elevation	0.05	0.95	Elevation	0.05	0.95
0.2	500	100.57	104.88	96.22	99.85	102.79	95.85
0.5	200	99.54	102.63	95.96	99.08	101.66	95.65
1	100	98.63	100.81	95.63	98.35	100.18	95.41
2	50	97.6	99.51	95.12	97.47	99	94.98
5	20	96	97.39	94.21	96.01	97.2	94.21
10	10	94.58	95.74	93.18	94.63	95.76	93.35
20	5	92.97	94.03	91.92	92.96	94.13	91.92
50	2	90.49	91.18	89.91	90.2	91	89.64
80	1.25	89.04	89.39	88.69	88.49	88.92	88.08
90	1.12	88.65	88.97	88.27	88.02	88.42	87.58
95	1.06	88.47	88.81	88.04	87.8	88.22	87.35
99	1	88.33	88.65	87.85	87.63	88.04	87.13

Table 2-25. With-Project Confidence Intervals for the Steele Bayou and Little Sunflower Gages

Probability	Return Period	Little Sunflower LS			Steele Bayou LS		
		Elevation	0.05	0.95	Elevation	0.05	0.95
0.2	500	98.64	102.46	93.52	97.48	101.18	92.82
0.5	200	97.09	99.92	93.17	96.23	98.96	92.51
1	100	95.9	98.03	92.75	95.23	97.29	92.17
2	50	94.7	96.43	92.35	94.17	96.06	91.8
5	20	93.09	94.5	91.65	92.67	94.1	91.15
10	10	91.87	93.02	90.87	91.46	92.7	90.4
20	5	90.63	91.51	89.98	90.18	91.15	89.53
50	2	88.97	89.43	88.66	88.39	88.87	88.04
80	1.25	88.11	88.3	87.85	87.41	87.65	87.11
90	1.12	87.9	88.08	87.6	87.16	87.36	86.84
95	1.06	87.8	87.97	87.47	87.04	87.25	86.69
99	1	87.72	87.9	87.37	86.96	87.16	86.57

158. RISK program, and an HEC-DSS output file. The ASCII output data were provided to Economics and used in their risk analysis as described in Appendix R.

PROPOSED PLAN

159. Table 2-26 shows the reduction in stages for the proposed plan for the various flood frequency events. Table 2-27 shows the departures for the various frequency flood events for the proposed plan versus the Yazoo Backwater report recommended plan (14,000-cfs pump). While the proposed plan reduces the 100-year frequency flood from elevation 100.3 to 95.2 feet (NGVD 29), it also reduces the volume of water by 38 percent which is significant in a backwater area. The stage-frequency curves for the upper and lower ponding area stage-frequency curves for base conditions and the proposed plan are shown in Figure 2-106 through Figure 2-109. The FESM model was used to delineate the 1-, 2-, 5-, and 100-year frequency floods for base conditions and with-pump conditions as shown in Figure 2-110 through Figure 2-113. Figure 2-114 through Figure 2-117, respectively, shows the base condition 1-, 2-, 5-, and 100-year frequency land-use classifications. Figure 2-118 through Figure 2-121, respectively, show the proposed plan 1-, 2-, 5-, and 100-year frequency land-use classifications.

Table 2-26. Proposed Plan on Total Ponding Area Reductions

Flood Frequency	Reduction in Stage	Reduction in Area	Reduction in Volume	Days to Lower Flood to 87 Feet	Change in Water Surface per Day
1-Year	0.67	17.4%	14.2%	1.8	0.34
2-Year	1.81	39.7%	35.8%	12.9	0.25
5-Year	2.78	36.1%	45.8%	32.7	0.18
10-Year	3.17	34.7%	45.9%	48.2	0.16
25-Year	3.34	35.1%	45.4%	64.3	0.14
50-Year	3.3	34.0%	43.1%	85.4	0.12
100-Year	3.12	32.0%	40.7%	99.7	0.11

Table 2-27. The Difference in Upper and Lower Sump Elevations between 2007 and 2020 Reports

Little Sunflower - Upper Sump				Steele Bayou - Lower Sump			
Flood Frequency	2020 14,000 cfs Pump	2007 14,000 cfs Pump	Elevation Difference	Flood Frequency	2020 14,000 cfs Pump	2007 14,000 cfs Pump	Elevation Difference
100	95.9	96.4	0.5	100	95.2	95.7	0.5
50	94.7	95.1	0.4	50	94.2	94.4	0.2
20	93.1	93.5	0.4	20	92.7	92.7	0.0
10	91.9	92	0.1	10	91.5	91.2	-0.3
5	90.6	90.7	0.1	5	90.2	89.6	-0.6
2	89.0	88.9	-0.1	2	88.4	87.8	-0.6
1	87.7	87.8	0.1	1	87.0	87	0.0

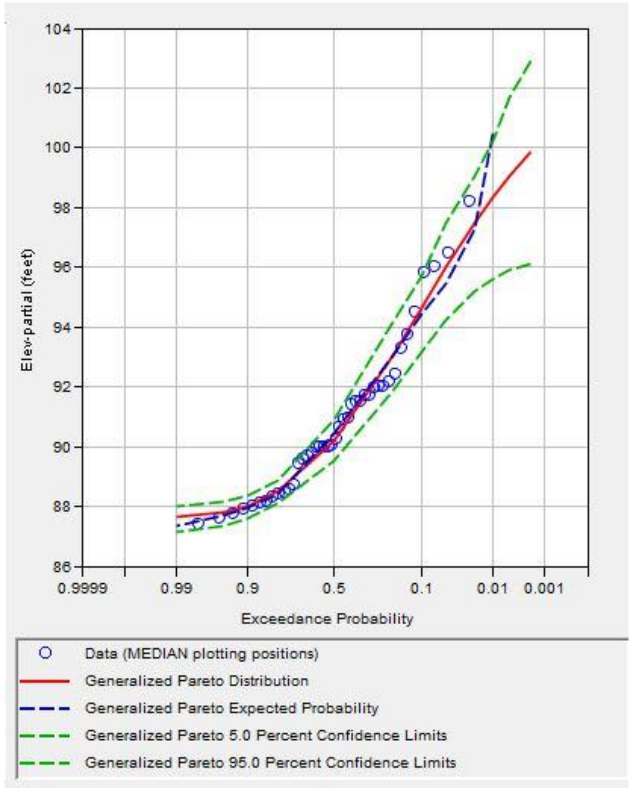

Figure 2-106. The stage-frequency curves for the lower ponding area for base conditions.

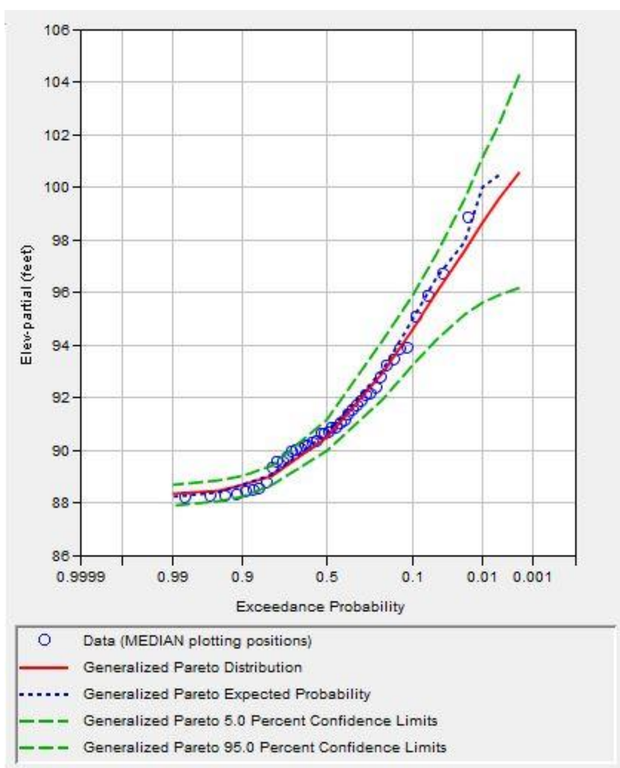

Figure 2-107. The stage-frequency curves for the upper ponding area for base conditions.

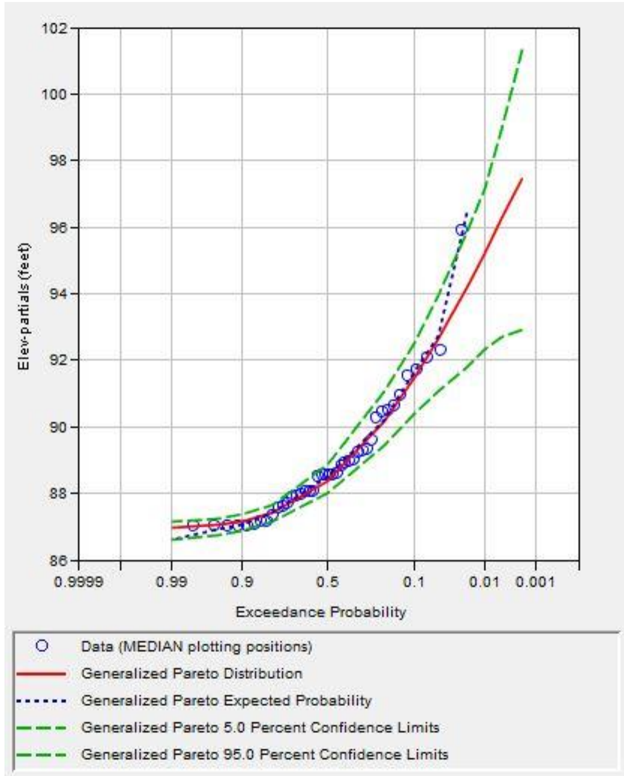

Figure 2-108. The stage-frequency curve for the lower ponding area for the proposed plan.

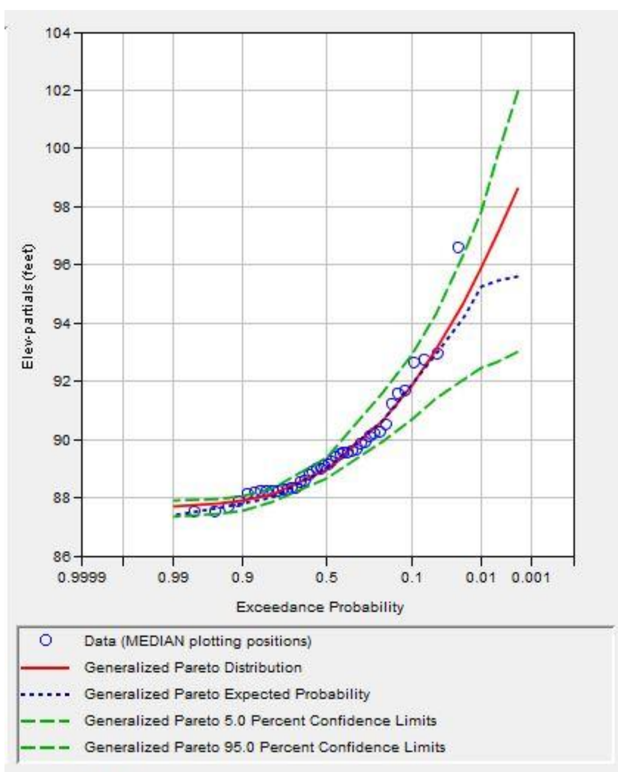

Figure 2-109. The stage-frequency curves for the upper ponding area for the proposed plan.

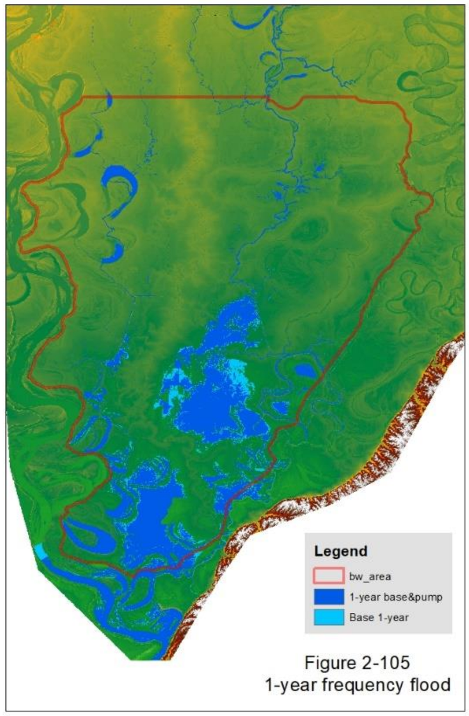

Figure 2-110. The FESM model 1-year frequency flood for base conditions and with-pump conditions.

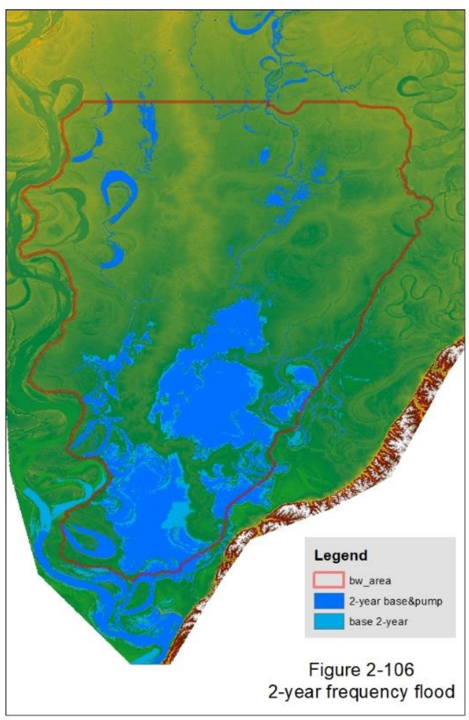

Figure 2-111. The FESM model 2-year frequency flood for base conditions and with-pump conditions.

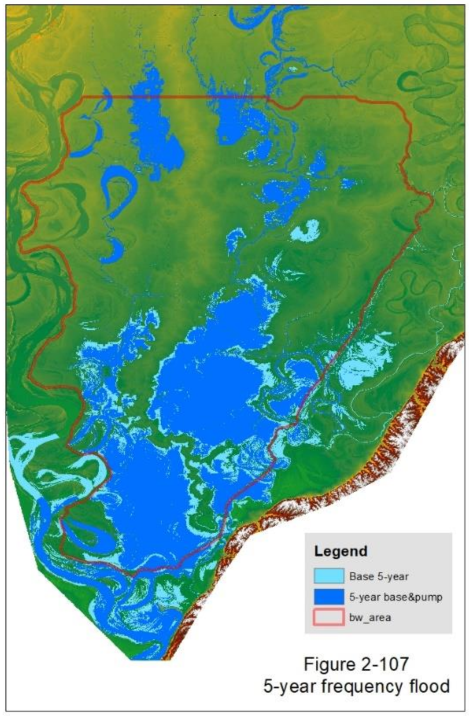

Figure 2-112. The FESM model 5-year frequency flood for base conditions and with-pump conditions.

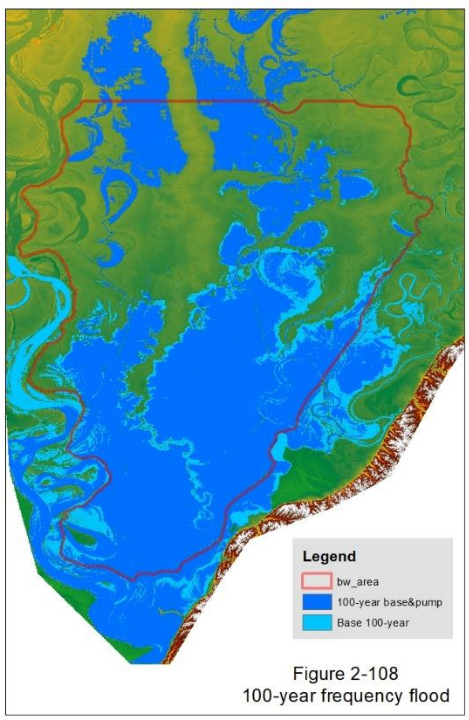

Figure 2-113. The FESM model 100-year frequency flood for base conditions and with-pump conditions.

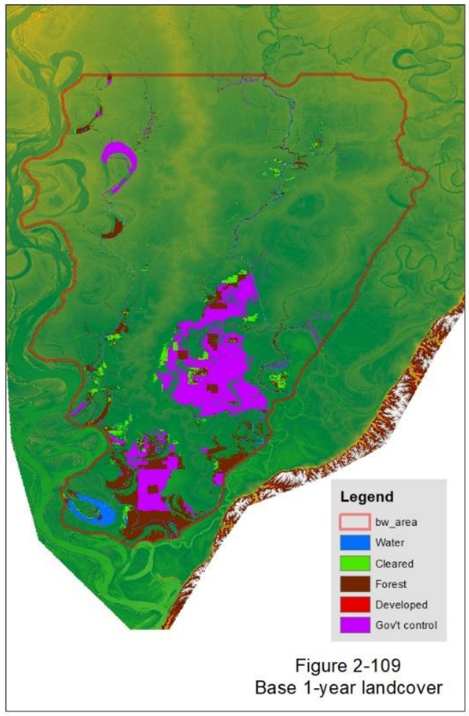

Figure 2-114. The base condition 1-year frequency land-use classification.

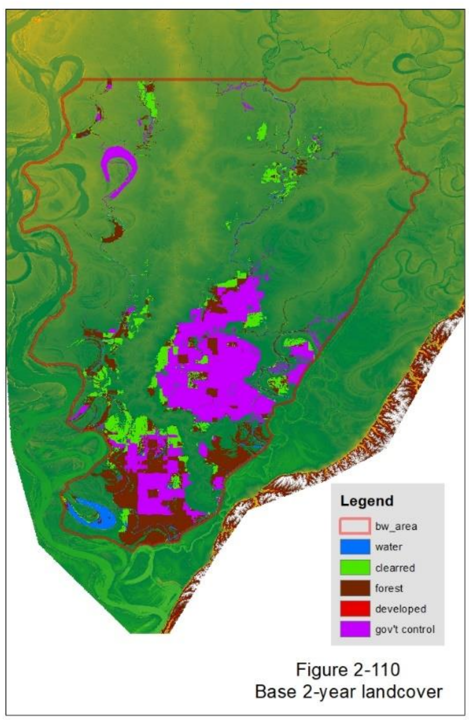

Figure 2-115. The base condition 2-year frequency land-use classification.

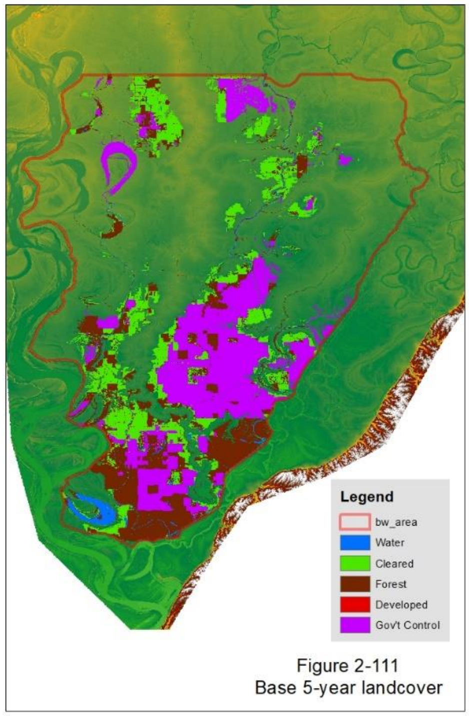

Figure 2-116. The base condition 5-year frequency land-use classification.

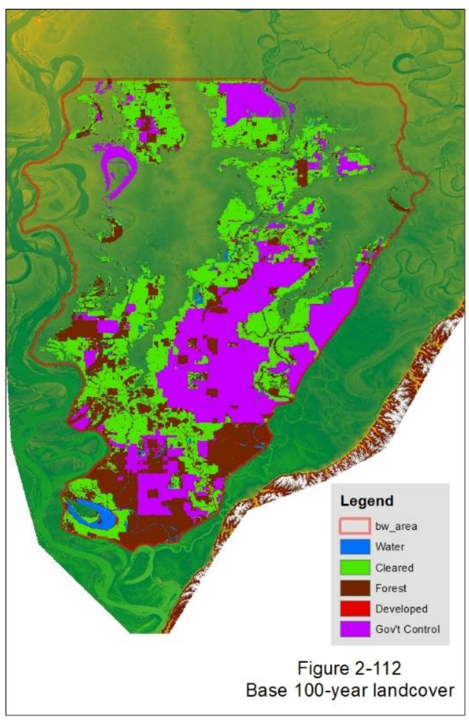

Figure 2-117. The base condition 100-year frequency land-use classification.

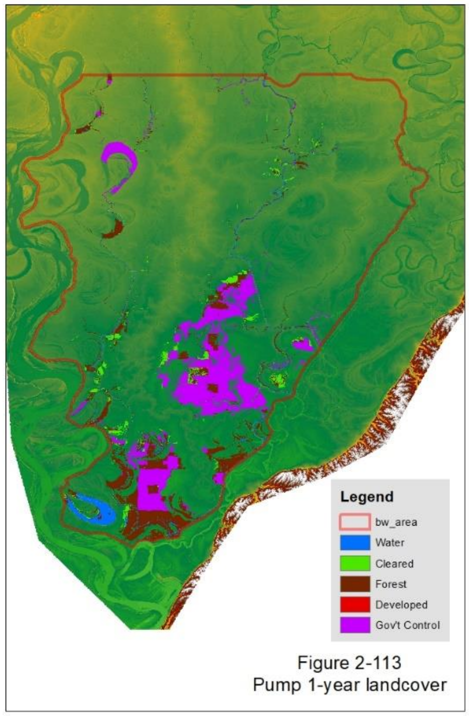

Figure 2-118. The proposed plan 1-year frequency land-use classifications.

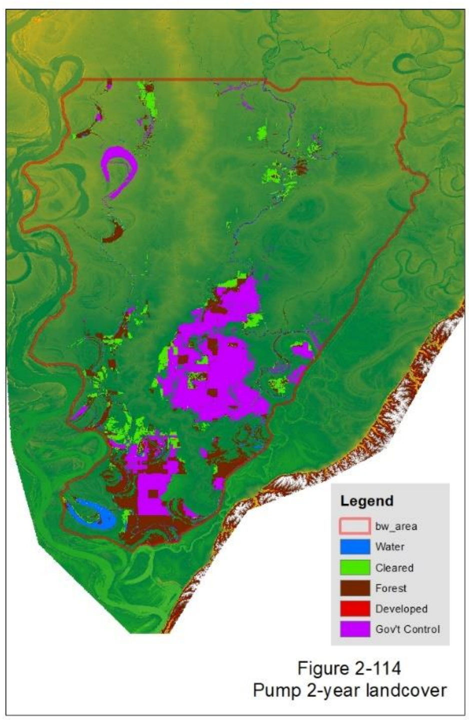

Figure 2-119. The proposed plan 2-year frequency land-use classifications.

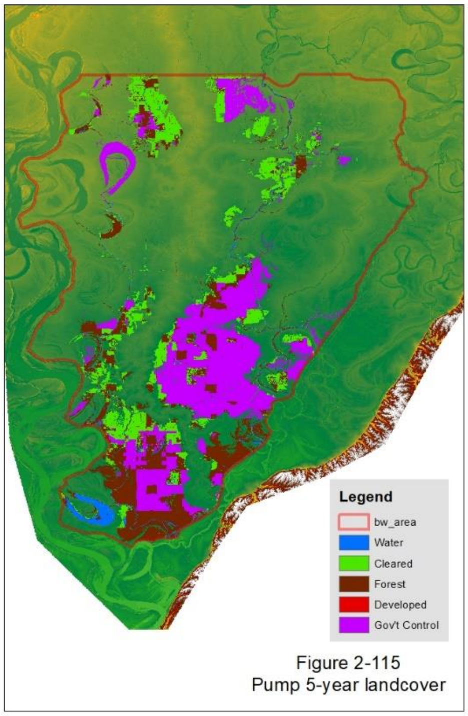

Figure 2-120. The proposed plan 5-year frequency land-use classifications.

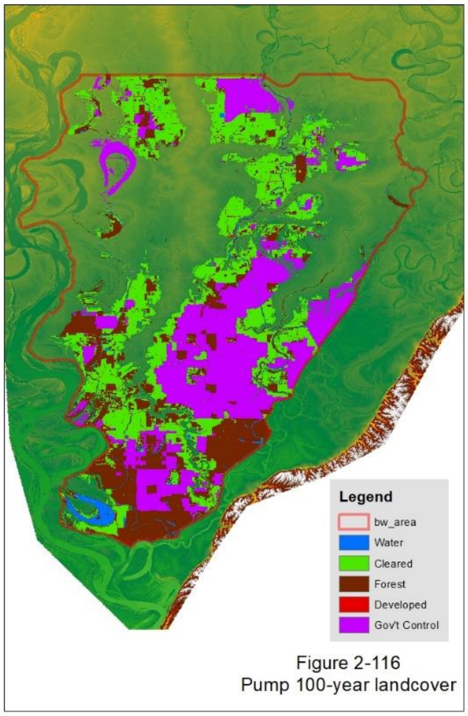

Figure 2-121. The proposed plan 100-year frequency land-use classifications.

160.This study used the 2018 National Agricultural Statistical Service (NASS) Crop Data Layer (CDL). The CDL has more than 50 land-use classes and is produced annually by NASS for each state. The land-use categories were simplified down to six broad categories, which are: unclassed, crop, cleared/non-crop, forest/wetlands, permanent water, and developed (cities and highways). Table 2-28 below has the land-use of the lands inundated by the one through the 100-year flood frequency events for the base and with-project conditions.

Table 2-28. Base condition cumulative land-use by frequency and Plan 5 Pump Frequency land-use

Base Condition Cumulative Land-use by Frequency							
Consolidated Landcover	**1-Year**	**2-Year**	**5-Year**	**10-Year**	**20-Year**	**50-Year**	**100-Year**
Unclassed	43.1	43.1	43.1	43.1	43.1	46.0	49.1
Crop	15100.5	44504.2	105969.7	147169.5	183500.2	223604.9	245949.8
cleared non-Crop	2975.6	5116.5	6995.1	7930.0	8880.5	9927.0	10321.8
Forest/Wetlands	101916.9	151424.0	194615.5	216707.5	231978.6	244224.9	249196.2
Developed	1154.6	2260.4	4180.5	5570.6	6780.2	8228.7	9205.6
Water	13950.7	14708.4	15709.0	16634.3	17300.8	18340.5	19071.7
Cumulative Total	135141.5	218056.7	327512.9	394055.1	448483.5	504372.1	533794.3
Plan 5 Pump Frequency Land-use							
Consolidated Landcover	**1-Year**	**2-Year**	**5-Year**	**10-Year**	**20-Year**	**50-Year**	**100-Year**
Unclassed	43.1	0.0	0.0	0.0	0.0	2.9	0.2
Crop	12114.0	19481.6	48882.4	32311.7	29813.6	32649.3	25193.0
cleared non-Crop	2675.6	1016.1	1978.4	1078.6	882.7	659.6	581.5
Forest/Wetlands	90519.8	37273.5	40160.3	20807.9	16779.8	17652.4	11351.2
Developed	982.5	719.2	1609.0	909.6	925.6	1172.2	834.9
Water	13578.9	662.1	1017.9	758.8	537.5	830.6	519.3
Total by frequency	119914.0	59152.4	93648.0	55866.6	48939.1	52967.1	38480.1
Cumulative Total	119914.0	179066.4	272714.4	328581.0	377520.1	430487.2	468967.4

161. The NASS land-use was also adjusted to remove Federally controlled lands. The lands within National Wildlife Refuges, Wildlife Management Areas, Wetland Reserve Program, and Conservation Reserve Program were lumped into a single category labeled as Federal. The adjusted land-use by flood frequency for the base and with-project conditions are presented in Table 2-29 below.

Table 2-29.The simplified federal lands – base flood frequency cumulative totals by frequency and the simplified federal lands – with-pump cumulative land-use by frequency

Simplified Federal Lands - Base Flood Frequency Cumulative Totals by Frequency							
	1-Year	**2-Year**	**5-Year**	**10-Year**	**20-Year**	**50-Year**	**100-Year**
water	10245.7	10561.7	11076.0	11442.6	11879.1	12556.6	13150.7
cleared	10553.2	35525.8	85430.5	122456.6	155884.4	193991.0	214929.5
forest	46713.2	70754.6	97745.1	111864.4	120618.3	127979.7	131350.2
developed	634.2	1314.1	2737.5	3910.0	4912.8	6174.7	6993.4
Federal	67268.4	100066.1	130868.7	144652.5	155404.8	163849.3	167399.7
Total	135414.8	218222.2	327857.8	394326.1	448699.4	504551.3	533823.6
Simplified Federal Lands - With-Pump Cumulative Land-use by Frequency							
	1-Year	**2-Year**	**5-Year**	**10-Year**	**20-Year**	**50-Year**	**100-Year**
Water	9977.4	10257.2	10684.8	11050.7	11307.3	11785.2	12057.5
Cleared	8859.9	24859.0	62341.8	91293.0	118797.4	148103.4	171326.1
Forest	42181.0	57810.3	80141.7	93600.1	104671.2	115603.0	122058.4
Developed	464.9	875.8	1935.2	2640.7	3388.6	4379.7	5101.4
Federal	58707.8	85409.9	117827.3	130182.2	139510.9	150796.6	158473.0
Sum	120191.0	179212.2	272930.7	328766.7	377675.3	430668.0	469016.4

PUMP AND FLOODGATE OPERATION DATA

162.The period-of-record-routing results were used to develop the data required to determine the pump energy requirements. The data used to calculate the energy requirements included average head, average annual number of days of pump operation, and discharge duration. The recommended plan yearly pumping data which show the periods of continuous flood event, number of days pumped per year, and some pumping statistics are found in Table 2-30. Figure 2-122 shows the number of days pumped per year. Figure 2-123 shows the number of days pumped by month. Figure 2-124 shows the proposed plan pump and floodgate operation by month for days the gates are closed and opened and when the pumps are on (pumps do not operate when gates are open). Based on these data, the recommended pump based on energy requirements was a natural gas-driven pump. Further refinements to the pumping station will be evaluated in depth following the approval of the proposed plan.

Table 2-30. Proposed Plan Yearly Pumping Data

YEAR	TOTAL DAYS PUMPED	CONTINUOUS FLOOD EVENTS PUMPED															DAYS ABOVE 87.0 FEET, NGVD, W/O PUMPING	YEAR
		PERIOD PUMPED	PUMP ON	PUMP OFF	PERIOD PUMPED	PUMP ON	PUMP OFF	PERIOD PUMPED	PUMP ON	PUMP OFF	PERIOD PUMPED	PUMP ON	PUMP OFF	PERIOD PUMPED	PUMP ON	PUMP OFF		
1978	0																0	1978
1979	47	3/11 - 5/2	87.2	96.3													81	1979
1980	0																33	1980
1981	0																0	1981
1982	9	12/16-12/25															17	1982
1983	55	4/17 - 6/11	88	95.8													91	1983
1984	43	4/7 - 4/24	87.3	90	5/10 - 6/4	90.8	91.9										66	1984
1985	1	12/16	87.0	87.0													3	1985
1986	0																0	1986
1987	0																0	1987
1988	0																0	1988
1989	1	3/18	88.7	88.6													25	1989
1990	14	6/2 - 6/15	87.1	89.4													43	1990
1991	23	1/5 - 1/27	87.4	91.7													87	1991
1992	0																0	1992
1993	10	4/10 - 4/16	87.5	90.1													54	1993
1994	36	4/13 - 5/18	89.2	90.9													90	1994
1995	22	6/6 - 6/27	87.1	87.9													23	1995
1996	17	6/8 - 6/24	87.1	88.1													20	1996
1997	35	3/8 - 4/11	88.9	93.3													50	1997
1998	24	5/2 -5/25	87	88.1													26	1998
1999	0																21	1999
2000	0																0	2000
2001	0																12	2001

Table 2-28 (Cont.) Proposed Plan Yearly Pumping Data

YEAR	TOTAL DAYS PUMPED	CONTINUOUS FLOOD EVENTS PUMPED															DAYS ABOVE 87.0 FEET, NGVD, W/O PUMPING	YEAR
		PERIOD PUMPED	PUMP ON	PUMP OFF	PERIOD PUMPED	PUMP ON	PUMP OFF	PERIOD PUMPED	PUMP ON	PUMP OFF	PERIOD PUMPED	PUMP ON	PUMP OFF	PERIOD PUMPED	PUMP ON	PUMP OFF		
2002	26	4/5	88.76		5/18-6/11	87.09	89.01										46	2002
2003	12	5/24 - 6/5	87.3	88.4													18	2003
2004	0																10	2004
2005	22	1/16 - 2/6	87.4	90													26	2005
2006	0																0	2006
2007	0																0	2007
2008	65	4/3 - 6/5	87.2	87.2													65	2008
2009	45	5/8 - 6/13	87.6	87.5	11/7 - 11/14	87	87.9										47	2009
2010	19	2/4 - 2/22	87.6	87.2													19	2010
2011	58	4/26 - 6/22	87.1	87.4													58	2011
2012	0																0	2012
2013	31	5/4 - 5/28	87.2	90.9	6/17 - 6/22	87.1	87.7										41	2013
2014	0																0	2014
2015	46	3/23 - 4/4	88.5	90.6	4/20 – 5/1	87.6	89.9	7/18 - 8/7	87	87.4							64	2015
2016	32	1/2 - 1/28	87.3	91.2	3/11 – 3/15	87.6	90.6										59	2016
2017	20	5/27 - 6/15	87.1	88.5													22	2017
2018	26	2/27 - 3/24	87.6	94.5													74	2018
2019	78	1/13- 1/19	89.8	91.5	2/17 – 3/31	89.8	96.8	5/11 - 5/21	97	98.1	6/6 - 6/18	97.7	97.7	7/4 - 7/9	97.6	97.6	217	2019
42YRS	817																1508	42 YRS

AVERAGE # DAYS PUMPED PER YEAR= 20 DAYS TOTAL
DAYS PUMPED = 812 DAYS
TOTAL # CONTINUOUS PERIODS PUMPED = 32 PERIODS
AVERAGE PUMP ON ELEVATION = 94.1 feet NGVD
AVERAGE PUMP OFF ELEVATION = 96.6 feet NGVD
MINIMUM PUMP ON ELEVATION = 87.0 feet NGVD
MAXIMUM PUMP ON ELEVATION = 97.7 feet NGVD
MINIMUM PUMP OFF ELEVATION = 88.0 feet NGVD
MAXIMUM PUMP OFF ELEVATION = 97.7 feet NGVD

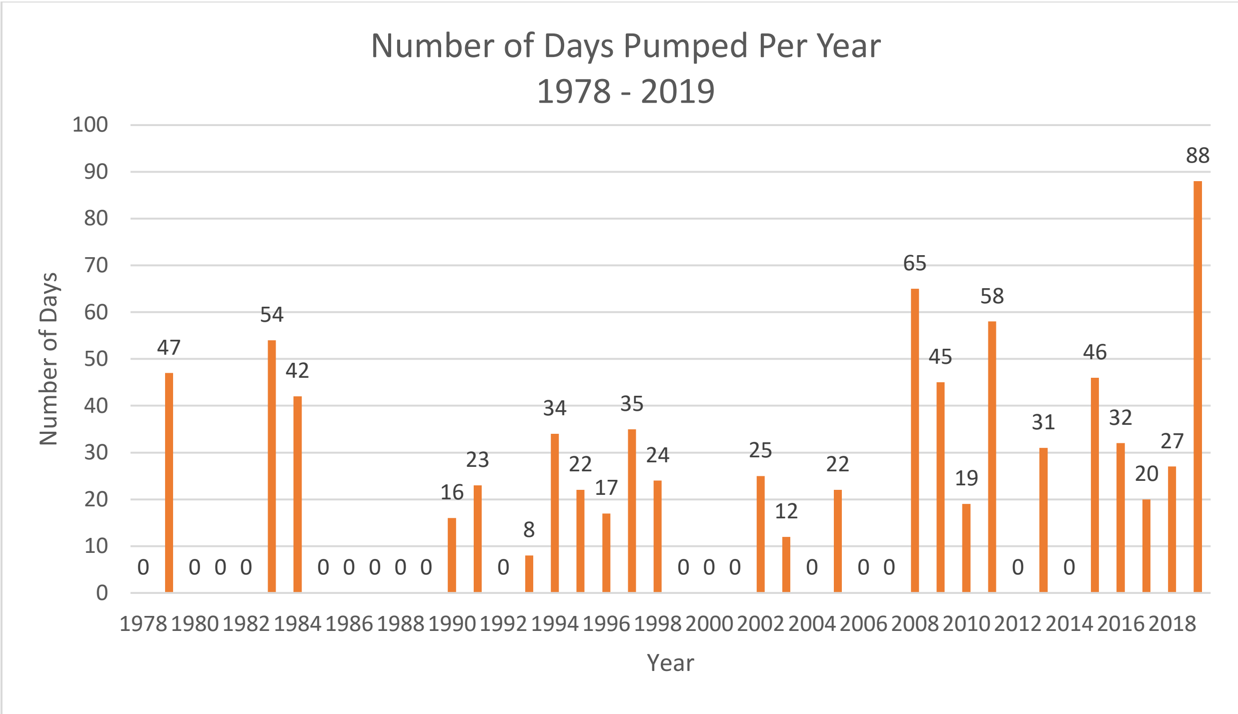

Figure 2-122. The number of days pumped per year.

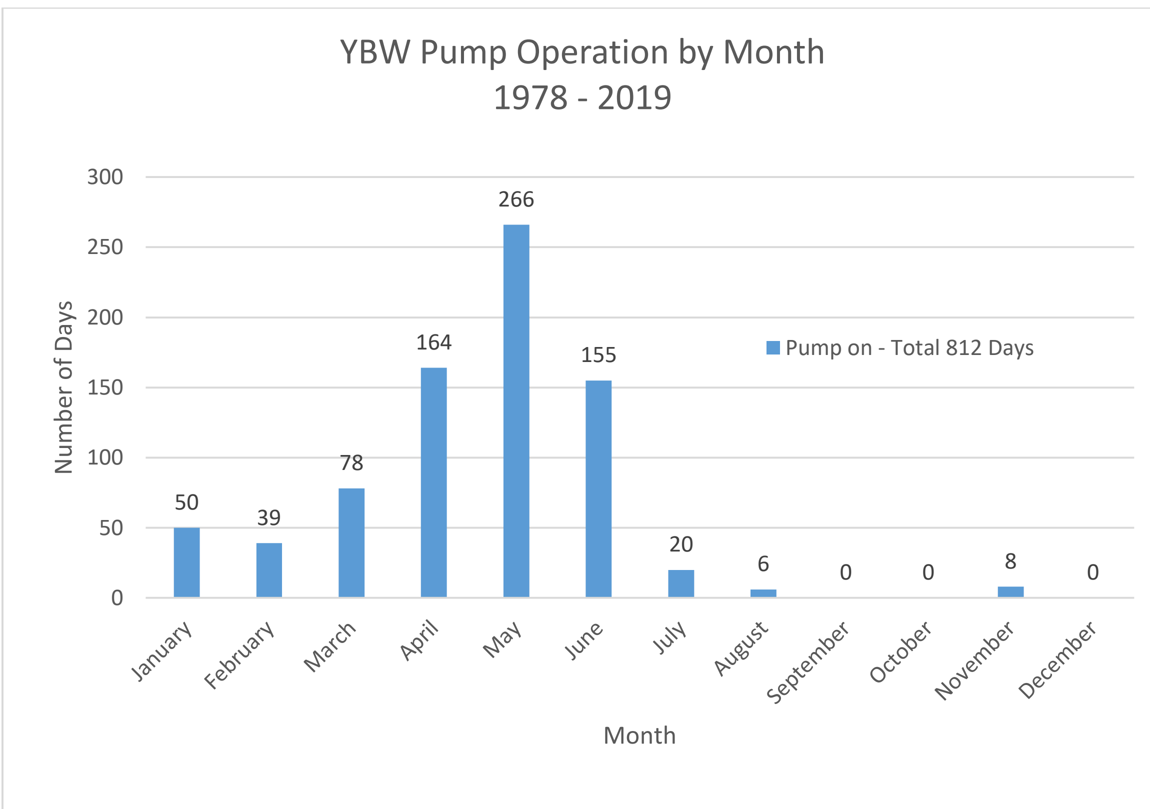

Figure 2-123. The number of days pumped per month.

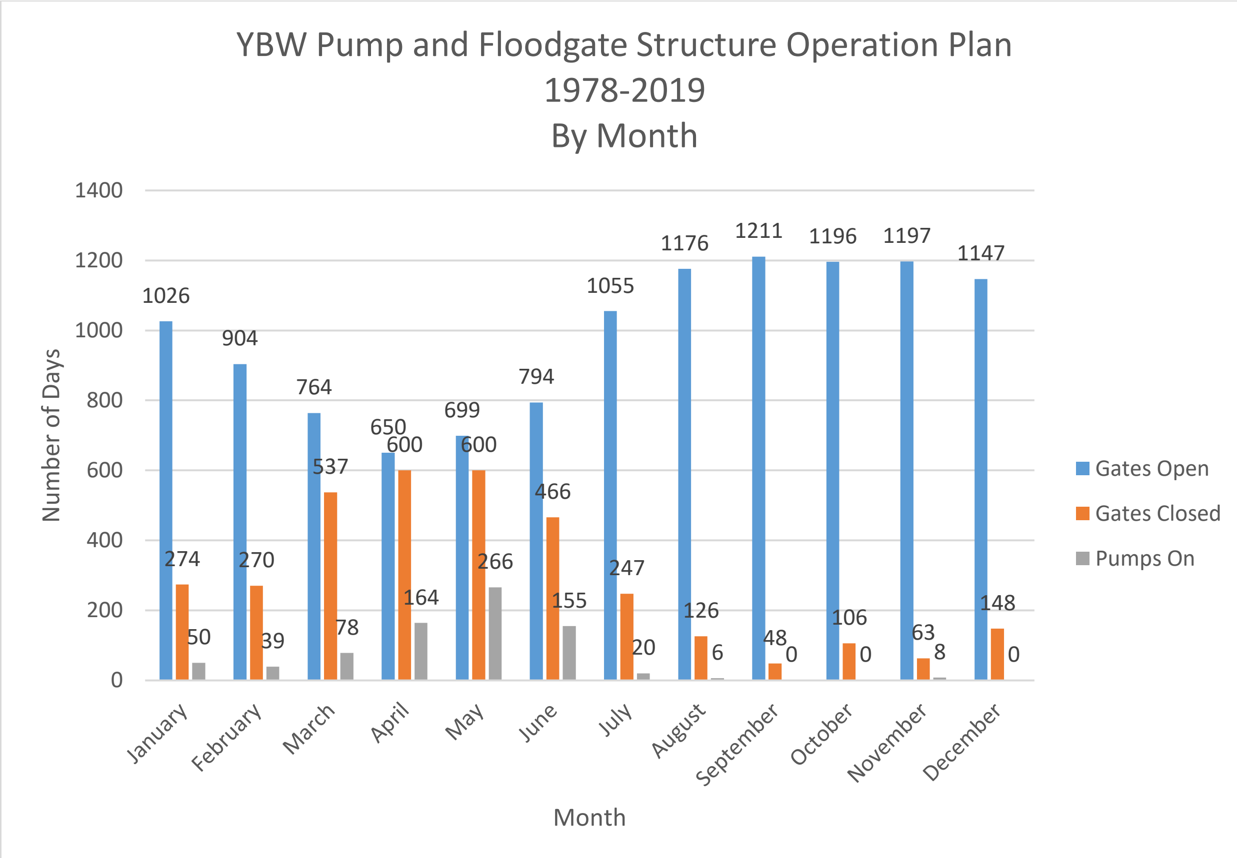

Figure 2-124. YBW Pump and Floodgate Structure operation plan from 1978 through 2019, by month.

PROPOSED PLAN PUMP OPERATION

163. For the proposed plan, the period-of-record-routing models pump operation included 12 pumps at 1,167 cfs each with a pump on/off elevation of 87.0 feet (NGVD 29). The model operated the number of pumps based on the available storage above elevation 87.0 feet (NGVD 29); e.g., if the inflow was such that it required ten pumps, the model would turn ten pumps on automatically. The real time pump operation would use a forecast of Mississippi River stages, forecasts of inflows from the Steele Bayou and Sunflower River, and consideration of interior runoff conditions to determine requirements for pumping. Since the natural gas-driven pumps cannot be instantaneously turned on at the same time, a pump operation scheme will be developed to achieve a pumping capability and flood control benefits commensurate with the benefits projected in the flood routings and benefit analysis. Specific refinements to the pump operation sequence will be developed as part of the water control plan for the project. The proposed plan pumping units and pump station layout are designed for a nominal pump on elevation of 87.0 feet (NGVD 29). To provide for a margin of safety, the discharge pipe maximum elevation was set at 106.0 feet (NGVD 29). This design allows for the pumps to operate efficiently and without damage down to elevation 86.0 feet (NGVD 29). Operation below 86.0 feet (NGVD 29) is outside of the design requirements for the pumping units and could damage the natural gas engines and/or pumps.

STANDARD PROJECT FLOOD

164. The Standard Project Flood (SPF) represents the flood that can be expected from the most severe combination of meteorologic and hydrologic conditions that are considered reasonably characteristic of the geographic region involved, excluding extremely rare combinations. Procedures for estimating the SPF involve a single storm event – the Standard Project Storm (SPS). However, with base conditions, flooding in the Yazoo Study Area generally results from a number of storm events occurring over a period of several months.

165. Assuming a condition when the floodgates are closed and the SPF event occurs over the Yazoo Study Area, the inflows are of such magnitude that the 14,000-cfs pumping station capacity is greatly exceeded and the interior ponding area would rise significantly where the floodgates would likely be operated for an extended period of time to evacuate the interior ponding for this headwater-type event. A similar but smaller event by comparison was the 1991 flood event, which was a headwater-type event with a low tailwater condition on the Mississippi River.

166. Should this condition occur with a high Mississippi River tailwater and an SPF event over the Yazoo Area, the pump would shorten the duration of the rising leg of the hydrograph and slightly reduce the peak stage. The extent and magnitude of flooding with the SPF would not be greatly affected by the 14,000-cfs pumping station because the storm was a very intense, short duration event with inflow rates much in excess of the pump capacity.

HYDRAULIC DESIGN

INLET AND OUTLET CHANNELS

167. The inlet channel will carry water from the existing diversion canal to the pumping plant. The inlet channel construction will require a section of the existing backwater levee to be removed. A new precast concrete bridge will be constructed over the inlet channel to provide access up and down the existing backwater levee. The 1,025-foot long inlet channel will have a bottom elevation of 65 feet (NGVD 29). The flared inlet channel entrance will have a 100-foot radius on both the north and south banks entering into the 300-foot wide inlet channel from the diversion canal. The next 375 feet of inlet channel will be 300 feet wide followed by a 450-foot transition to a channel width of 346.3 feet. The last 100 feet of inlet channel will be 346.3 feet wide as it arrives at the pumping plant.

168. The outlet channel will carry water from the pumping plant to the Yazoo River. The 1,915-foot long outlet channel will have a bottom elevation of 76 feet (NGVD 29). The outlet channel for the first 200 feet, as it leaves the pumping plant, will be 346.3 feet wide followed by a 450-foot transition to a channel width of 300 feet. The remaining outlet channel will be 300 feet wide with a flared outlet into the Yazoo River. The north bank will have a 50-foot radius and the south bank will have a 150-foot radius.

169. Both the inlet channel and outlet channel bottoms will be lined with R-650 riprap. The inlet channel will have 1V:4H side slopes lined with R-2 I,k00 riprap extending from the channel bottom to an elevation of 80 feet (NGVD 29). The outlet channel will have 1V:4H side slopes

lined with R-200 riprap extending from the channel bottom to an elevation of 86 feet (NGVD 29).

PUMP DESIGN

170.The pumping station was designed and modeled prior to the cancellation of the project in 1986. Reference Technical Report HL-88-2, "Pumping Station Inflow-Discharge Hydraulics, Generalized Pump Sump Research Study," ERDC, February 1988.

ENVIRONMENTAL ANALYSIS

WATERFOWL

171.Waterfowl feeding habitat is defined as areas that are inundated by up to 18 inches of water. The Yazoo Backwater stages generally increase during the waterfowl season of 01 November to 28 February. Mean monthly stages increase by 10 or more feet at most gaging locations during this period. The maximum and minimum stages during the winter waterfowl season were determined by the computer program ENVIRO-DUCK. The ENVIRO-DUCK program was initially developed by the Vicksburg District with the cooperation of the U.S. Fish and Wildlife Service (USFWS). It was based on a food energy model developed by the USFWS. ENVIRO-DUCK was later updated and modified by Dr. Mickey Heitmeyer for the Memphis District. For input, the program requires the beginning and ending dates of the waterfowl season and the period-of-record to be used in the analysis. The program also requires a stage-area curve, which it uses to calculate the daily acres inundated (resting) and the daily acres of feeding habitat. Using this information, the program calculates the daily resting and feeding acres available, sums these for each year, and calculates the average acres available during each year. The program also calculates the annual mean, minimum, and maximum stages during the waterfowl season. Finally, it calculates the mean, minimum, and maximum stages during the entire period-of-record during the waterfowl season.

172.The areal extent of available waterfowl habitat was determined with the FESM flood mapping tool. Water surface profiles for the minimum and maximum stages were used to map the upper and lower bounds of the waterfowl habitat. The NASS crop cover for 2018 for the seven states in the study area were merged into a single coverage, and clipped to the project area. The FESM tool produces a TIFF file. The maximum extent TIFF file was converted to a polygon file, which was then used to clip the NASS crop layer to produce the land-use of available waterfowl habitat.

FISHERIES

173.The computer program ENVIRO-FISH was used to analyze fisheries habitat. The program was initially developed in the late 1980s by the Vicksburg District with the assistance of the USFWS. The program considers two important life cycle stages of fish, which are the spawning and rearing stages. The input parameters for the program are: minimum and maximum depths for spawning and rearing, spawning days, and season (beginning and ending dates). ENVIRO-FISH also requires a DSS file with daily stages and a stage area curve for each gage. The program produces two reports, a detailed daily report and a summary report by year. The daily report provides the date, stage, total rearing, restricted rearing, and spawning acres. The annual report provides the following fields: year, average stage, average total rearing, average restricted rearing, average spawning, maximum stage, maximum total rearing, maximum restricted rearing, maximum spawning, minimum stage, minimum total rearing, minimum restricted rearing, and minimum spawning acres. The program finishes by providing the average seasonal stage, the average minimum stage, and the average maximum stage. The summary provides the same three statistics for each of the three habitat types.

TERRESTRIAL

174. To identify areas for terrestrial and aquatic evaluation, the elevation equal to or exceeding five, 10, 25, 50, 75, 90, and 95 percent of the time annually (annual exceedence duration) for the period-of-record was computed. The elevations were determined by the SAS UNIVARIATE program. The five, 10, 25, 50, 75, 90 and 95 percent elevations were determined for each gage. The SAS UNIVARIATE program computed the duration intervals for each year, each decade, each month, and each season.

175. Mink require terrestrial environments near water. The 50 percent exceedence elevation was used to represent areas inundated for 180 days during the year. Thus, the 50 percent duration for the period-of-record at each gage was used to represent available mink habitat. The FESM tool was used to determine the areal extent of the 180-day duration flood. The 180-day duration elevation was less than the minimum elevation at most gage locations; therefore, the minimum elevation in the DEM at each gage was used instead of the 180-day duration. This FESM output provided the minimum water surface of the major rivers in the Yazoo Study Area, but the mink inhabit areas adjacent to the rivers. The FESM output from the mink model run was incorporated into ArcMap and converted into a polygon coverage. A 100-foot buffer area was computed around the polygon to produce a new coverage that represents the available habitat for mink. There was very little difference between the base and with-pump 180-day durations. Both elevations were less than the minimum water surface elevation in the DEM, and therefore the base and with-pump mink habitat is the same.

176. The wood duck breeding season occurs during the spring. Wood duck survival is best if the chicks are close to water when they leave the nest. Wood duck habitat was modeled as the 46-day duration (50 percent exceedence elevation) elevation during the spring (March through May). The median spring duration was calculated for each of the six reaches for each year in the period-of-record. During Wood Duck rearing, the primary source of food is invertebrates. They can feed in shallow water that is less than or equal to12 inches or on the forest floor. They need to have dense brush nearby for refuge and nesting during the night. The 2018 NASS Crop Data Layer was condensed into four land-use categories, cleared, forested, permanent water, and developed. The condensed Crop Data Layer was then added to the Stage-Area layer to produce a new coverage that contained the available acres of the four categories from elevations 75 through 108 feet (NGVD 29). The stage-area curve was developed in one foot intervals. The area in each interval was sub-divided into 0.1 foot intervals by linear interpolation. The revised forest stage-area curve, in 0.1 foot intervals, was used to calculate the median annual acres for Wood Duck rearing. The impacted acres were determined by subtracting the with-pump rearing acres from the base rearing acres.

REFORESTATION

177.The reforestation of private developed lands in the lower Delta below the 1-year frequency flood elevation is the nonstructural feature in the final array of alternatives. The FESM model was used to determine and delineate the privately owned cleared lands below 87 feet (NGVD 29), which is the pump-on elevation. Using the updated 2018 land-use maps, the FESM model determined that there were 2,100 privately owned cleared acres were at or below this elevation. These acres include all cleared acres and catfish ponds without CRP, WRP, and WMAs. Figure 2-125 shows the delineation for the nonstructural unprotected areas below the 1-year event and the structurally protected areas above the 1-year event. More detailed information regarding reforestation can be found in the Mitigation Appendix O.

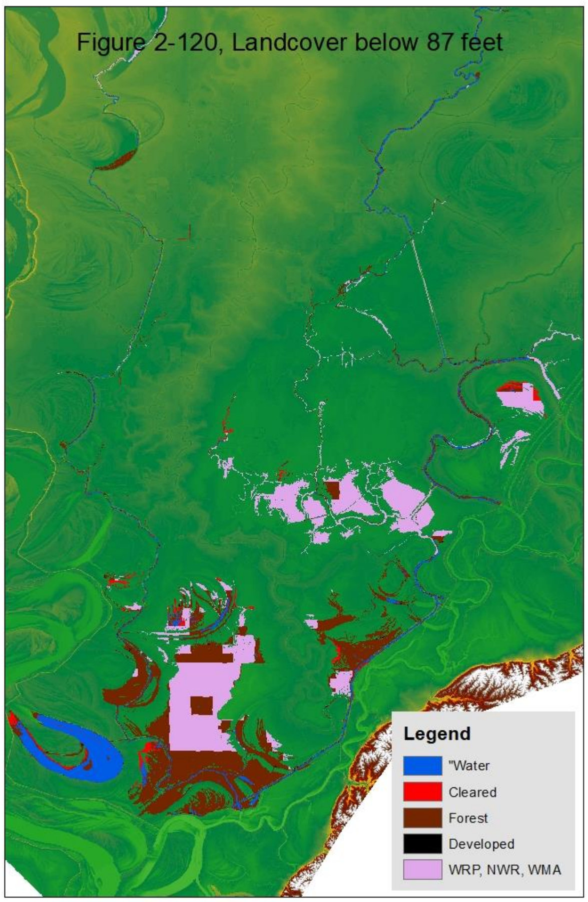

Figure 2-125. The delineation for the nonstructural unprotected areas below the 1-year event and the structurally protected areas above the 1-year event

WETLAND HYDROLOGY

178.One criterion in the determination of wetlands is the degree of continuous inundation or saturation during the growing season. Areas that are inundated for 14 consecutive days, or are saturated in the top 12 inches for 21 consecutive days, are wetlands. Inundation generally comes from riverine flooding, while saturation comes from precipitation. However, depressional areas can also experience inundation from precipitation. In the Yazoo Study Area, flood inundation will be the dominant source of inundation, and the impacts on this project will be made based on changes in flood inundation. However, it should be noted that flood inundation is not the primary source of moisture that sustains wetlands in the Yazoo Study Area, but precipitation is the dominant source of moisture which sustains the wetlands. The proposed plan will not alter soil saturation which results from precipitation. In Wetlands (Mitsch and Gosselink 2015) the hydrology of bottomland hardwood wetlands is discussed at length. According to the authors during the winter leaf-off period precipitation greatly exceeds evapotranspiration and the excess moisture accumulates in the surficial soils. The soils remain saturated until after the trees leaf-out and evapotranspiration can dry the soils in the root zone (top 30 centimeters). In 2009 the Vicksburg District established 24 shallow groundwater monitoring wells in Delta National Forest and the Twin Oaks Wildlife Management Area. The wells were approximately 90 cm deep and extended 60 cm above the ground surface. In May and June of 2010, the wells were fitted with pressure transducers to measure water depth (Ott, Omniprobe). Data was collected at each site for two to nine years. The failure to collect data at several sites was due to vandalism, logging operations, or equipment failure. When the equipment failed, the transducers were returned to the manufacturer in an attempt to retrieve additional data. The transducer recorded depth every six hours. The total days of saturation per year in the top 30 cm for each well is presented in Table 2-31 below. The wells were located based on flood frequency and flood duration. Two wells were located in the 2-year frequency, 14-day duration zone. "These wells collected 10 station years of data and experienced an average of 167 days of saturation from 2011 to 2016 (2010 was excluded due to the short period of operation during the early growing season). Four wells were placed in the 2-year frequency 7-day duration zone. These wells collected 20 station years of data and experienced an average of 161 days of saturation per year from 2011 through 2017. Eight wells were located in the 2-year frequency less than seven day duration zone. These wells collected 32 station years of data, and experienced an average of 122 days of saturation from 2011 through 2018. Based on a minimum of 21 consecutive days of saturation in the root zone (top 30 cm), six of these sites were classed as wetlands and two sites were classed non-wetland. The six wetland sites average 96 days of saturation per year, while the two non-wetland sites averaged 56 days of saturation per year. There were nine wells located in the 5-year flood frequency zone. These wells collected 47 station years of data and averaged 51 days of saturation per year from 2011 through 2016. Three of these wells were classed as wetlands based on more than 21 days of continuous saturation in the root zone. The average days of saturation per year for these three wells was 88. In contrast, the six wells that were classed as non-wet only experienced an average of 23 days of saturation per year. One well was located in the 10-year flood frequency zone, and it experienced and average of 47 days of saturation per year for the three years it was in operation. As shown in Table 2-31, the average days of saturation varied from four to 193 days per year. Three sites averaged less than 10 days, while ten sites averaged more than 100 days per year. The average days of saturation due to precipitation was more than 10 times the expected days of flooding in the three flood duration zones with wells in the 2-year floodplain. The 2004 three tiered EMAP wetland sampling study identified wetlands in every flood frequency zone (one to 100-year). It is clear that all sites above the 2-year flood plain could not meet the minimum days of flooding per year to be classed as wetlands, and therefore the only source of moisture available to sustain those sites is precipitation. Wetland scientists at ERDC that were monitoring the success of wetland restoration projects for the Vicksburg District established an additional 42 monitoring wells in restoration sites. The results from all of these wells is discussed in the Wetland Appendix, Appendix I. Additional information from the monitoring wells can be found in the Excel file named Ott_summary.xlsx, which can be found on the District's Yazoo Backwater Webpage.

Table 2-31. Annual Days of Saturation in the Top 30 cm by Well and Year

City Name	Site Name	2010	2011	2012	2013	2014	2015	2016	2017	2018	Sum 2011+	Annual Average
Atlanta	DNF-A	0	0	0	126	51	109	97			383	63.8
Aberdeen	DNF-A2	0	23				4	114			141	70.5
Baton Rouge	DNF-B	0	6	10	126	69	136	107			454	75.7
Baltimore	DNF-B2	0	23								23	23.0
Chicago	DNF-C	0	1	1	2	3	10	18			35	5.8
Dallas	DNF-D	0	6	26	158	89	144	124			547	91.2
Eldorado	DNF-E	2	58	23	82	52	71	67			353	58.8
Fort Worth	DNF-F	0	1	2	8	6	7	17			41	6.8
Jackson	DNF-J	1	81	136	150	123					490	122.5
Kansas City	DNF-K	0	104								104	104.0
Los Angeles	DNF-L	17	41	133	128	153	179	136			770	128.3
Memphis	DNF-M	0	52	76							128	64.0
New Orleans	DNF-N	21	159	192	238	179					768	153.6
Philadelphia	DNF-P	2	132	141	155	164	194	125			911	151.8
Tallahassee	DNF-T	0	0	5	7						12	4.0
Utica	DNF-U	0	60	25	76	23	78	81			343	57.2
Vail	DNF-V	0	6	25	92	37					160	40.0
Waterloo	DNF-W	0	6	19	61	24					110	27.5
Yankee	DNF-Y	1	109	138	132	37	203	123			742	123.7
Zealand	DNF-Z	2	108	107	39	22	38	21			335	55.8
Gainesville	TO-G	1	152	113							265	132.5
Houston	TO-H	22	160	182	231	221	226	157	180		1357	193.9
Raleigh	TO-R	1	94	90	165	120	126	127	89	101	912	114.0
San Francisco	TO-S	0	156	152	167						475	158.3

WETLANDS IN THE 2- AND 5-YEAR FLOOD FREQUENCY ZONES

179. The US Army Corps of Engineers Wetland Delineation Manual (Corps Manual) defined wetlands as "areas that are inundated or saturated by surface or ground water at a frequency and duration sufficient to support, and that under normal circumstances do support, a prevalence of vegetation typically adapted for life in saturated soil conditions" (Environmental Laboratory 1987). Operationally the Corps Manual described wetlands as areas that exhibit wetland hydrology [inundation or saturation for a minimum continuous period of ≥5% of the growing season in most years (50% probability of recurrence)], hydric soils, and hydrophytic vegetation

(see the table below, which was adapted from Table 5 of the Corps Manual). Within the project area, ≥5% of the growing season corresponds to approximately 14 days. Additionally, subsequent guidance outlined in the Technical Standard for Wetland Hydrology replaced the ≥5% of the growing season criteria with a threshold of ≥14 consecutive day for identifying wetland hydrology (USACE 2005; 2010).

Table 2-32. Description of hydrologic zones based upon the information provided in Table 5 of Environmental Laboratory (1987) and subsequent analysis of hydrology to establish the minimum wetland hydroperiod (days/year) within the Yazoo Backwater Area

Zone	Name	Duration[2]	Comments	Days/year[3]
I[4]	Permanently inundated	100 percent	Inundation >6.6 ft mean water depth	
II	Semipermanently to nearly permanently inundated of saturated	>75 - <100 percent	Inundation defined as <= 6.6 ft mean water depth	203-365
III	Regularly inundated or saturated	>25 - 75 percent		67-203
IV	Seasonally inundated or saturated	>12.5 - 25 percent		34-67
V	Irregularly inundated or saturated	>=5 - 12.5 percent	Many areas having these hydrologic characteristics are not wetlands	14-34
VI	Intermittently or never inundated or saturated	<5 percent	Areas with these hydrologic characteristics are not wetlands	<14
[1]Zones adapted from Clark and Beniforado (1981).				
[2]Refers to duration of inundation and/or soil saturation during the growing season.				
[3]Using the 270 day growing season for the Yazoo Backwater Area				
[4]This defines an aquatic habitat zone.				

180. The FSEIS examined potential impacts to wetlands associated with 1) the direct impact area where some wetlands will be converted to non-wetlands due to land use changes associated with the physical footprint of the pumping plant and other infrastructure and 2) the indirect impact area where some wetlands will exhibit a shift in the duration of surface water inundation following project implementation. In order to assess potential project impacts, the subset of wetlands exhibiting a minimum of 14 days duration of flood inundation at a frequency of five years in 10 were selected to determine wetlands that may be altered by the project. This determination was made in accordance with the guidance above which establishes the 14-day minimum criteria for wetland hydrology. Areas that experience less than 14 days of flood inundation in at least five years in 10 would not meet the wetland criteria as a result of flooding. Thus, only the subset of lands that are inundated by flooding for ≥14 days (i.e., the minimum wetland hydrology duration threshold) occurring within the 2-year floodplain (i.e., those with a flood frequency return interval of five years in 10) were considered during the assessment of potential impacts to wetland resources.

181. Notably, the Vicksburg District acknowledges the presence of wetlands outside of the 2-year floodplain elevation and in areas that experience <14 days of flood inundation, but those wetlands are sustained by precipitation. The project will not have any impact on precipitation or the wetland functions provided by wetlands outside the area of influence of the project. The following paragraphs provide data and a discussion to help readers understand the interplay between different sources of wetland hydrology (i.e., flooding vs precipitation) and the limited influence of flooding on the observed patterns of soil saturation in the project area.

SOIL SATURATION AT GROUNDWATER MONITORING WELLS

182. Evaluating the duration of soil saturation is important to understanding the sources of wetland hydrology in the study area and can help determine the subset of wetlands that should be considered during the assessment of potential impacts to wetlands. As mentioned in paragraph 178 above, the Vicksburg District established shallow groundwater monitoring wells (roughly three feet in depth) at 23 sites in Delta National Forest (DNF) and Twin Oaks Wildlife Management Area. In May and June of 2010 depth transducers were installed at each site. The wells were maintained from one to nine years from 2010 to 2018.

183. Table 2-31 provides the days of soil saturation per year for each of the well locations. The annual average days of saturation ranged from a low of 4 days per year to a maximum of 193.9 days per year. Using the mapped flood frequency zones from this study, eight wells were located in the 1-year flood frequency zone, four wells were located in the 2-year zone, two were located in the 5-year zone, four were located in the 10-year zone, four were located in the 20-year zone, and one was located in the 50-year flood frequency zone. Fifteen of the wells had wetland hydrology (>14 days of saturation in the top 30 cm in one year of two), and eight did not have wetland hydrology. Table 2-33 below shows the total days of soil saturation during the operation of each well, the number of years of operation, and the average annual days of soil saturation for these shallow groundwater monitoring wells. Because the transducers were not deployed until May or June of 2010, the total days of saturation and the average days of saturation in this paragraph are based on the years from 2011 and after. Table 2-31 lists the days of saturation by year, and the table includes 2010, but Table 2-33 does not include the days of saturation in 2010 in either the totals or the averages.

Table 2-33. Duration of Soil Saturation

Site identifier	**Mapped flood frequency zone**	**Wetland hydrology determination**	**Total days of saturation**	**Years with available data**	**Average annual period of stauration**	**Average annual period of saturation with the flood frequency zone (all locations)**	**Average annual period of saturation with the flood frequency zone (wetland locations)**	**Average annual period of saturation with the flood frequency (non-wetland locations)**
Houston	1	Wet	1199	8	193.9			
New Orleans	1	Wet	789	5	192			
Philadelphia	1	Wet	913	7	151.8			
Raleigh	1	Wet	913	9	114.1			
San Francisco	1	Wet	475	4	158.3			
Kansas City	1	Wet	102	2	104			
Memphis	1	Wet	128	3	64			
Jackson	1	Wet	491	5	122.5	137.6	137.6	
Gainesville	2	Wet	266	3	132.5			
Los Angeles	2	Wet	787	7	128.3			
Utica	2	Wet	343	7	57.2			
Baltimore	2	Not wet	23	2	23	82.3	106.0	46.8
Tallahassee	5	Not wet	12	4	4			
Waterloo	5	Not wet	110	5	27.5	15.8		
Dallas	10	Wet	547	7	91.2			
El Dorado	10	Not wet	353	7	58.8			
Fort Worth	10	Not wet	41	7	6.8			
Vail	10	Not wet	160	5	40	49.2	91.2	35.2
Atlanta	20	Wet	383	7	63.8			
Baton Rouge	20	Wet	454	7	75.7			
Chicago	20	Not wet	35	7	5.8			
Zealand	20	Not wet	337	7	55.8	50.3	69.75	30.8
Yankee	50	Wet	743	7	123.7	123.7	123.7	

184.The wells are sorted by flood frequency zones. The eight wells in the 1-year flood zone average 130.6 days of saturation per year, with the days of saturation ranging from 64 to 171.3. All of the wells in the 1-year flood zone had wetland hydrology. There were four wells in the 2-year flood zone. Three had wetland hydrology and one did not. The average duration of saturation for all wells was 85.3 days per year. The three with wetland hydrology averaged 107.1 days of saturation per year, while the non-wet site averaged 23.0 days per year. There are two wells in the 5-year flood zone, neither had wetland hydrology. The average days of saturation for these two wells is 15.8 days per year. There were four wells in the 10-year flood zone. Three were non-wet and one was wet. The average length of annual saturation was 49.3 days. The single wet site had an average length of saturation of 91.2 days, while the three non-wet sites averaged 33 days per year. There were four wells in the 20-year flood zone. Two had wetland hydrology and two did not. The average length of saturation for the four wells was 50.4 days per year. The two wet sites had an average length of saturation of 69.8 days, while the non-wet sites averaged 33 days of saturation per year. There was a single well in the 50-year flood zone, it had wetland hydrology with an average length of saturation of 123.8 days per year.

185. In 2010-2011 ERDC installed additional shallow groundwater monitoring wells at wetland mitigation sites in the Yazoo Basin, some of which were in the Backwater Project Area. The days of saturation at those locations are presented in Table 2-34. These wells are located in the 1-year through the 5-year flood zones. The days of saturation by flood zones is similar to what is described in the long-term data set.

Table 2-34. Duration and Frequency at ERDC Wells

Flood Frequency			**Flood Duration**		
Site identifier	Mapped flood frequency zone	Days of saturation	Site identifier	Mapped flood frequency zone	Days of saturation
FM-94	1	109	DAR-21	<7	109
FM-98	1	31	PO-5	<7	57
GT-86	1	32	FM-97	<7	98
DAR-21	2	109	LG-101	<7	111
PO-5	2	57	LG-28	<7	122
FM-97	2	98	LG-16	7 to 13	120
LG-1	2	157	FM-94	14 to 20	109
LG-101	2	111	FM-98	14 to 20	31
LG-106	2	165	LG-1	21 to 27	157
LG-108	2	153	LG-106	21 to 27	165
LG-16	2	120	LG-21	21 to 27	169
LG-21	2	169	LG-22	21 to 27	160
LG-22	2	160	GT-86	21 to 27	32
LG-28	2	122	LG-108	28 to 34	153
DAR-01	5	114			
DAR-05	5	111			
DAR-20	5	129			
DAR-22	5	120			
GT-84	5	43			
FM-93	10	29			

186. These data demonstrate that the majority of locations examined exhibited extensive periods of wetland hydrology and that wetland hydrology was observed across a range of mapped flood frequency zones, including area that very rarely experience flooding (e.g., >20-year flood frequency zones). The annual length of soil saturation varies significantly across different flood frequency zones, but that soil saturation is an annual event and the length of soil saturation during each year greatly exceeds the minimum number of days needed to meet the wetland hydrology criteria. Although many of the sites at the 5-year flood frequency and above have long periods of soil saturation and exhibit wetland hydrology (soil saturation in the top 30 cm for 14 consecutive days), these sites are only wetlands due to the soil saturation resulting from precipitation. The potential for infrequent flood events (e.g. 1 year in five) will not affect their status as wetlands. Because the frequency of flooding is insufficient to establish these sites as wetlands, they were excluded from the analysis of the impacts of this project to wetlands.

SOIL SATURATION COMPARED TO FLOOD INUNDATION

187. Evaluating the role of flooding in the observed patterns of wetland hydrology is useful for estimating how the proposed project, which will alter flood durations, may impact wetland resources. As a result, the following describes the duration of flooding within the project area from 2011-2018.

188. The total observed days of flood inundation for the 1, 2 and 5-year flood frequencies at three gages in the Big Sunflower Ponding Area are shown in Table 2-35. There were three 2-year events at the Anguilla gage, two at the Holly Bluff gage, and five at the Little Sunflower gage. There were 64 days of flooding above the 2-year elevation at Anguilla, 91 days at Holly Bluff, and 179 days at Little Sunflower. There were no 5-year events at Anguilla, two at Holly Bluff, and one at Little Sunflower. Only one of the groundwater monitoring wells was operational in 2018 (Raleigh). The 2018 flood exceeded the 2-year elevation at all three gages, and the 5-year elevation at two gages.

Table 2-35. Annual duration of flooding that exceeded the 14-day wetland hydrology criteria, the 2-year flood frequency elevation, and the 5-year flood elevation at three gages (Ang = Anguilla; HB = Holly Bluff; LS = Little Sunflower)

Year	Ang 14d	Ang 2yr	Ang 5yr	HB 14d	HB 2yr	HB 5yr	LS 14d	LS 2yr	LS 5yr
2010	0	0	0	0	0		0	0	0
2011	6	0	0	62	0		88	42	0
2012	0	0	0	2	0		3	0	0
2013	16	9	0	48	0		62	21	0
2014	6	0	0	8	0		0	0	0
2015	4	0	0	34	0		60	6	0
2016	39	27	0	75	32	17	82	40	0
2017	0	0	0	0	0		38	0	0
2018	64	28	0	80	59	28	94	70	26
Sum	**135**	**64**	**0**	**309**	**91**	**45**	**427**	**179**	**26**

189. Using the data from 2011 through 2016, the average total days of soil saturation for the twelve shallow wells in the 1 and 2-year flood zones (the wells flooded by a 2-year event) is 535.8, with an annual average length of soil saturation of 120.2 days per year. The average total days of flooding above the 2-year elevation is 59 (Table 2-35, (36+32+109)/3). Thus, the total days of soil saturation in the 12 wells inside the 2-year flood zone is nearly 10x greater (538.5:59) than the observed days of flooding during the same six years. The three gages averaged 6, 5.3, and 18.2 days of flooding at the 2-year elevation during the six-year period (2011-2016). Over the same period, the twelve wells averaged 120.2 days of saturation per year. Again, the soil saturation exceeds the flood inundation by a factor of more than 10. Performing the same analysis of the fourteen wells in the 5-year flood zone yields similar results. The wells averaged a total of 467.9 days of soil saturation compared to 17 days of flood inundation at only one of the three gages. There were no flood events which exceeded the 10-year flood frequency elevation at any gage during the period the groundwater wells were in operation. All saturation for wells in the 10-year flood zone and above could only be saturated due to precipitation.

190. Table 2-33 also shows that the average days of saturation observed at these wells is much greater than the minimum 14-days required to qualify for wetland hydrology. Table 2-34, shows the days of saturation during 2010-2011 at 20 shallow groundwater monitoring sites. This data supports the length of saturation data presented above, and coupled with the relatively small contribution of flooding to the period of soil saturation, highlights the dominant role that precipitation plays in sustaining wetland hydrology in the study area. In summary, soil saturation at the 43 shallow groundwater monitoring gages greatly exceeds flood inundation. Removing or reducing the days of flood inundation, would not cause these sites to be converted from wetlands to non-wetlands. All sites that show wetland hydrology, would continue to show wetland hydrology, because soil saturation due to precipitation is the dominant source of moisture to sustain bottomland hardwood wetland systems, as described in Mitsch and Gosselink (2015).

SATURATION VERSUS INUNDATION WITH TABLE 2-30 HYDROLOGIC ZONES

191. If the 23 shallow groundwater monitoring wells in Table 2-31 and Table 2-33 would be sorted based on the annual days of saturation using the hydrologic zones II through VI in Table 2-32 (i.e., Table 5 from the Corps Manual). Three of the study locations wells would fall into Zone VI, non-wetlands. Two wells fall into Zone V (5-12.5% duration, 14-34 days of saturation), with an average annual duration of saturation of 25.3 days. Neither of these wells had median saturation of 14 days and were therefore non-wetlands. Six wells would fall into Zone IV, with annual saturations between 34 to 68 days per year. They have an average annual duration of saturation of 56.7 days. Three of these wells exhibited wetland hydrology, and three did not. Twelve wells had annual duration of saturations between 25 and 75% of the growing season (67.5 to 202.5 days per year), with an average duration of 132.4 days. All twelve of these wells had wetland hydrology.

192. The two wells in Zone V (probable wetlands) did not have wetland hydrology even though the duration of saturation exceeded 25 days per year. Only half of the well sites in Zone IV had wetland hydrology, although all sites with more than 34 days of saturation every two years should theoretically be wetlands. Part of the discrepancy here is that the wells recorded total days of saturation, and wetland status is based on periods of continuous saturation. The daily

water surface elevations were used to determine if the sites met the requirement for wetland hydrology. Table 5 in the WDM is likely based more on flood inundation than soil saturation. Reliable transducers were not widely available in 1981. Most studies which use groundwater monitoring wells find that the total days of saturation at wetland sites exceeds 100 days per year. More studies using groundwater monitoring wells are needed, so wetland scientists will have a better understanding of the duration of saturation required to create wetland conditions.

HGM FLOOD DURATION ZONES

193. The Yazoo Basin HGM Manual was used to assess the impacts of this project on wetland resources. In order to conduct the assessment, the 2-year floodplain was divided into the six flood duration zones used in the HGM analysis of wetland impacts. The six resulting durations were 1 to 6, 7 to 13, 14 to 20, 21 to 27, 28 to 35, and greater than 35 days. Twelve of the shallow groundwater monitoring wells were located within the three shorter duration zones. Table 2-36 below shows the data from these twelve wells. Five wells were located within the 14 to 20-day duration zone (Probable wetlands). The five wells had an average length of saturation of 126.9 days. Three wells were located in the 7 to 13-day flood duration zone. These wells had an annual average length of saturation of 160.2 days. Finally, an additional four wells were located in the 1 to 6-day duration zone. These wells had an average annual saturation duration of 81.7 days.

Table 2-36. Duration by Duration Zone

Well Name	**GridV**	**Dura zone**	**Wetland Hydrology**	**Ave days of saturation**	**Ave by Duration zone**
Gainesville	0	<7	Wet	132.5	
Raleigh	0	<7	wet	114.1	
Utica	0	<7	wet	57.2	
Baltimore	0	<7	nw	23.0	79.46
Houston	10	7	wet	193.9	
San Francisco	10	7	wet	158.3	
Los Angeles	10	7	wet	128.3	160.2
New Orleans	20	14	wet	192.0	
Philadelphia	20	14	wet	151.8	
Kansas City	20	14	wet	104.0	
Memphis	20	14	wet	64.0	
Jackson	20	14	wet	122.5	126.9

194. These data demonstrate that the period of wetland hydrology far exceeds the period of flood inundation across all flood duration intervals. The duration of soil saturation from precipitation is nearly ten times the duration of flooding, and the saturation occurs on an annual basis not on a biannual basis. Conversely flooding contributes to the wetland hydrology of some wetlands in

some years as outlined in a recent publication by Berkowitz et al. (2019). The data clearly shows that wetland hydrology in the basin is dominated by soil saturation due to precipitation and not from flood inundation.

195. The HGM model developed for application in the Yazoo Study Area addresses a number of wetland subclasses. For the purpose of the current assessment, all wetlands are assumed to occur within the Riverine Backwater subclass. This selection was made because 1) the wetlands examined occur within the 2-year flood frequency interval and 2) the Riverine Backwater subclass encompasses the full suite of wetland functions described in Smith and Klimas (2002). Notably, the selection and application of other wetland subclasses that occur in portions of the Yazoo Study Area, such as Flats or Depressions, would decreases the estimated impacts to wetland resources (and associated mitigation requirements) because those wetland subclasses only provide a subset of the wetland functions provided by River Backwater wetlands. As a result, the assumption that all of the wetlands included in the assessment are Riverine Backwater wetlands represents the most conservative approach possible for selecting wetland subclasses.

196. Smith and Klimas (2002) define the Riverine Backwater wetland subclass as "those wetlands subject to backwater flooding from streams at frequencies of 5 years or less". As stated elsewhere, the current assessment only included the subset of Riverine Backwater wetlands subject to flood inundation at flood frequencies of 2 years or less. This determination was made because these are the wetlands areas that experience flooding at a frequency to sustain wetland hydrology as described in the operational definition of wetlands as discussed above. This approach is consistent with the wetland assessment procedures outlined in Smith and Klimas (2002), which instruct the user to take the following steps: a. Define assessment objectives, b. Identify regional wetland subclasses, c. Characterize the project area, d. Screen for red flags, e. Define the wetland assessment areas, f. Collect field data, g. Analyze field data, and h. Apply the assessment results. When conducting the wetland assessment, the regional wetland subclass was identified (step b; i.e., Riverine Backwater) and the wetland assessment area was defined (step e; i.e., those areas containing wetlands that derive their sustaining hydrology from floodwater inundation at a frequency of 2 years or less). This approach demonstrates that the assessment of wetland areas flooded at frequencies outside of the 2-year floodplain is not required simply because those areas occur within Riverine Backwater subclass.

197. The completed project is expected to alter flood durations in the project area, but it will not alter soil saturation due to precipitation. The Hydrogeomorphic (HGM) method was used to evaluate wetland functional values for both the base and with-project conditions. The HGM method uses five different duration intervals to evaluate functional values. Those duration intervals are: seven days, 14 days, 21 days, 28 days, and 35 days. The 87 Manual used five percent (14-days) and 12.5 percent (35 days) of the growing season to determine the upper and lower bounds of possible wetlands. The additional duration intervals were used to more accurately determine the wetland functional values. The results of the HGM assessment are presented in the Wetland Appendix.

HEADWATER FLOODS COMPARED TO BACKWATER FLOODS

198. The Yazoo Backwater Area is affected by two types of flood events. All flood events are due to precipitation events. They can be single events or multiple events. These events can be

internal or external in origin. The external events can occur anywhere within the Mississippi River Basin above Vicksburg. Internal events unaffected by external factors generally have rapid rises and rapid falls, while external events have much slower rises and falls. The historic 2011 flood is an example of a flood which was primarily external in nature. The Steele Bayou RS gage rose to 80 feet on 10Mar and did not drop below 80 until 19Jul, a period of 131 days. The riverside gage was higher than the landside gage for the entire period. Although long in days, the internal flood was relatively small due to below normal precipitation and the maximum elevation was only 90.0 feet. The common perception of the Yazoo Backwater Project is that the pump will eliminate all flooding within the basin. This is far from the truth, because the Project will only address backwater flood events, and it will not even be put into operation during headwater flood events. The reason behind this is that the Steele Bayou structure can release a maximum of 55,000 cfs, and at one foot of head (LS > RS), the gates can release 19,000 cfs which exceeds the pump capacity by a factor of 1.36. This difference does not include the capacity of the gates at the Little Sunflower Control Structure.

199.As mentioned above there are two types of flood events, headwater and backwater, but how many of each have occurred in the period-of-record (POR)? Table 2-25 lists all of the backwater flood events. Backwater flood events (Steele Bayou gates closed) occurred in 27 of 42 years during the POR, including 14 of the last 22 years and ten of the last 12 years. The National Climate Assessment predicts the region will have more frequent and more intense storm events, which will result in more frequent flooding. A different method called peaks over threshold is used to calculate the total number of flood peaks during the POR. The method uses three threshold values, time between peaks, minimum peak elevation, and minimum change in water surface elevation between two peaks. The peaks over threshold filtering is accomplished with the Statistical Software Package (SSP Version 2.2) by the USACE Hydrologic Engineering Center (HEC). The following threshold values were used for the filtering, 14 days separation, the minimum elevation was equal to the annual 1-year frequency flood, and a minimum rise and fall of three feet. This produced from 114 to 190 peaks above the annual 1-year frequency flood elevation per site for the six gage location used in this study. Peaks per site are Little Callao, 190; Anguilla, 139; Holly Bluff, 151; Little Sunflower LS, 143; Grace, 114; and Steele Bayou LS, 117. The two gage locations in the Steele Bayou sub-basin likely had fewer peaks due to the smaller size of the basin. Using Table 2-25 and Figures 2-8 through 2-31, there were 69 instances where the gates were closed. These 69 closures resulted from 48 separate flood peaks. Dividing 48 peaks by the total peaks per station yields a ratio of backwater to headwater peaks. This ratio ranged from 0.25 (Little Callao) to 0.42 (Grace). Thus, at the Little Callao gage one out of four peaks is a backwater peak, but three of four are headwater peaks. At Grace four peaks in ten are backwater, while six others are strictly headwater flood events. All of the basin will continue to receive many more headwater floods than backwater floods over the years to come. These headwater events help fill wetland areas and area ditches providing wildlife habitat and wetland benefits from flood pulses.

WETLAND ELEVATION DEVELOPMENT

200.The computer program WETSORT was used to perform the statistical analyses for determination of wetland profiles. For each year of the period-of-record evaluated, WETSORT identified the span of consecutive days, within the growing season, having the highest mean stage and reported the lowest water surface elevation within that span of days.

201. WETSORT ranks the elevations for each year in descending order. The median elevation for the period-of-record is the resultant value for the gage. The WETSORT program provided the median elevation for the years 1962 to 2018 for the five duration intervals listed above.

WETLAND MAPPING

202. The GIS flood mapping tool Flood Event Simulation Model (FESM) used the five profiles to determine the areal extent of each of the duration intervals. The FESM tool uses three GIS data layers. The first layer is a point file with the gage locations and their respective water surface elevations for the five duration intervals. The second layer is a polyline file, which connects the 25 gage locations. The last data layer is a digital elevation model (DEM). A 10-meter DEM was used in this study. The FESM tool was run five times, once for each duration interval, and the five resultant files were merged to form a composite wetland zone map.

WETLAND IMPACTS DETERMINATION

203. Wetland impacts were developed using the HGM method and utilized the HGM Yazoo Basin Handbook by Smith and Klimas (2002). The wetland impacts are presented in the Wetland Appendix (Appendix I).

EFFECTS OF THE PROPOSED PLAN

MISSISSIPPI RIVER AND YAZOO BACKWATER FLOOD STAGES

204. In the 1982 analysis and subsequent design analysis, the impact of a large pump station (25,000 cfs) on Mississippi River stages was evaluated by use of the Mississippi Basin Model, which was calibrated to 1973 conditions. Flood hydrographs for the 1973 and 1975 floods were introduced and stage hydrographs were recorded at stations on the Lower Yazoo and Mississippi Rivers for various conditions including pre-project (no backwater levees), existing (levees and floodgates only), and the recommended 25,000-cfs pump station. The tests indicated a maximum increase of about 0.4 foot in riverside stages with the 25,000-cfs station in continuous operation. With the recommended 14,000-cfs pump station, the increase would be much smaller than with the 25,000-cfs station as tested.

205. From the routing results and rating curves, it is estimated that the maximum increase in peak stages, with the 14,000-cfs pumps on the riverside of the pump station, would be about 0.25 foot for riverside conditions near the initial pump start-up elevation of 87.0 feet (NGVD 29). At 87.0 feet (NGVD 29), the water levels are below major damage levels for developed areas downstream of the pump station along the Yazoo and Mississippi Rivers. For example, for the start pump elevation of 87.0 feet (NGVD 29) on the riverside of the pump station and a comparable stage of 40.77 feet on the Mississippi River at Vicksburg gage (gage zero is equal to 46.23 feet [NGVD 29]) the flow is approximately 1.1 million cfs. The maximum discharge of 14,000-cfs from the pump station is approximately one percent of the total flow in the Mississippi River at the pump start elevation of 87.0 feet (NGVD 29). The 2019 event was ran in HEC-RAS and compared the with and without pump results at the Steele Bayou Riverside and Vicksburg gages. The peak stage differences for the event showed an increase of 0.2 feet at Vicksburg, and 0.3 feet at Steele Bayou Riverside with a 14,000-cfs pumping station.

NAVIGATION

206.The Recommended Plan will not impact any stages on the Yazoo River for river stages below 87.0 feet (NGVD 29). Therefore, the navigation depth under low-flow conditions would not be impacted. The pump outlet channel was designed to minimize crosscurrents in the navigation channel when the pumping station would be operating. Reference Technical Report HL-90-4, "Yazoo Backwater Pumping Station Discharge Outlet," ERDC, May 1990.

SEDIMENTATION

207.During certain prolonged periods when the pumps are not in operation and river stages are at moderate levels (80 to 87 feet), some minor sedimentation is expected to occur in the approach to the inlet channel of the pumps and in the outlet channel near the confluence with the Yazoo River. While sedimentation is not expected to be of any major concern, the control of vegetation in the deposited areas will need to be pursued possibly on an annual basis. It is likely after the project is complete, that removal of sediment accumulations (averaging about 1 foot in depth over the extent of the channels which is approximately 80,000 cubic yards) once or twice in the life of the project may be necessary depending upon the sequence of hydrologic events which could result in deposition in the channels as described above. Material deposited in the outlet channel by the secondary currents of the Yazoo River may be returned to the Yazoo River without any significant impacts. That material deposited in the inlet channel will likely be disposed in upland areas available within the pumping station property.

CHANNEL STABILITY

208.With the proposed plan, the water surface slope in the existing connecting channel will be slightly steeper than base conditions. However, during the most severe conditions indicated by the period-of-record-routings, the channel velocity would be less than 4 feet per second, and no channel stability problems are anticipated.

ENDANGERED SPECIES

209.Possible impacts to habitat of endangered species, such as pondberry, were analyzed using hydrologic data and the FESM model. Endangered species analysis is found in the Threatened and Endangered Species section of the SEIS.

YAZOO BACKWATER PUMP ENTRAINMENT AND IMPINGEMENT

210.The proposed project would install and operate twelve pumps with an overall capacity of approximately 14,000 cubic feet per second in the Yazoo Basin to reduce seasonal flood elevations above 87 feet National Geodetic Vertical Datum (NGVD). Fish approaching the intakes are susceptible to entrainment by the pumps, which have axial flow impellers operate at 145 to 151 RPM's creating intake velocities of 1.7 feet per second increasing to 2.3 feet per second at the trash rack, and 5.8 feet per second at the formed suction intake. The trash racks are spaced approximately 5.5 inches apart preventing larger fish from entering the intakes, although adult fish could become trapped against the racks (i.e., impingement). Small-bodied fish could be entrained and are susceptible to physical strike of the impeller and can be subjected to rapid changes in shear stress, pressure, acceleration, and turbulence.

211. To evaluate species composition of potentially entrained fish, the outlet below Steele Bayou Structure was sampled with paired "bongo" nets (0.75-meter diameter, 4.5-meter long, 505-micrometer mesh) during August 2019 and May through June 2020 after the Steele Bayou gates were open following impoundment. The Yazoo River above the Steele Bayou outlet and the outlet of Forest Home Chute, a natural backwater draining into the Yazoo River, were also sampled for comparison. Net samples were taken below the water surface and each sample was of 5-minute duration fished from a stationary boat. A General Oceanics Model 2035-B flow meter was mounted in the mouth of each net to measure velocity of water passing through the net. Meter readings and duration of sampling were converted to an estimate of water volume filtered for each sample. Samples were fixed and preserved in five percent buffered formalin. In the laboratory, fishes were identified to the lowest practical taxon and enumerated. Catch was expressed as density (e.g., number of larval fishes per 100 cubic meters of water filtered) and used to describe temporal patterns in occurrence and relative abundance.

212. USACE acknowledges that entrainment may occur during operation of the pumps, but does not anticipate significant impacts to fish populations in the study area based on the following reasons:

a. Over 98 percent of the fishes collected with bongo nets were either Gizzard or Threadfin Shad, and of these individuals, 99 percent were larvae or juveniles (Table 2-37). Gizzard and Threadfin Shad are ubiquitous throughout the lower Mississippi Valley and are often the most abundant fish species in lakes and rivers. No protected or rare species were collected.

Table 2-37. Abundance of fish species collected in bongo nets during summer 2019 and spring-summer 2020 after the Steele Bayou structure was opened following impoundment. Abundance is expressed as number of fish/100 cubic meters of water filtered.

Scientific Name	Common Name	Frequency	Percent	Cumulative Frequency	Cumulative Percent
Clupeidae	Shad	1643.0	47.4	1643.0	47.4
Dorosoma sp.	Shad (either Gizzard or Threadfin)	1101.6	31.8	2744.6	79.1
Dorosoma cepedianum	Gizzard Shad	673.6	19.4	3418.2	98.6
Pomoxis nigromaculatus	Black Crappie	19.0	0.6	3437.2	99.1
Lepisosteus oculatus	Spotted Gar	10.8	0.3	3448.0	99.4
Hypophthalmichthys molitrix	Silver Carp	4.7	0.1	3452.7	99.6
Pomoxis annularis	White Crappie	3.8	0.1	3456.4	99.7
Ictiobus sp.	Buffalo	3.7	0.1	3460.2	99.8
Micropterus salmoides	Largemouth Bass	2.2	0.1	3462.4	99.8
Centrarchus macropterus	Flier	1.7	0.1	3464.1	99.9
Morone chrysops	White Bass	1.4	0.0	3465.5	99.9
Gambusia affinis	Mosquitofish	1.2	0.0	3466.7	100.0
Aphredoderus sayanus	Pirate Perch	0.7	0.0	3467.5	100.0
Lepomis sp.	Sunfish	0.7	0.0	3468.2	100.0

b. The pump station will draw water near the bottom of the inlet channel, which is approximately 27 feet in total depth. Based on the Water Quality and Aquatic Appendix, deeper water during impoundment is hypoxic (less than three milligrams per liter of dissolved oxygen) and avoided by fish.

c. Most adult fish, including minnows, have burst speeds of three feet per second or greater that can be maintained for at least 30 seconds, which exceeds the water velocity at the trash intake but not the formed intake. Most fish avoid moving backwards in a current (at the point of entrainment) and will exhibit burst swimming speeds to move out of the intake area if possible. Fish entrained and not injured would move through the outlet into the Yazoo River where access to floodplain and riverine habitat is widely available.

d. Most studies of fish entrainment through power plant turbines concluded that overall mortality is less than five percent (Cada 1990).

FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURACE MAPPING

213. The base flood mapping for the entire project area was compared to the FESM model 100-year frequency base conditions flood delineation. The FESM model delineation produced very similar results. Coordination meetings with FEMA and the Mississippi Emergency Management Agency were held to address issues pertaining to flood insurance issues. The Corps is required by law to update any FEMA Flood Insurance Study mapping impacted by a

project. The project will require updating of the current FEMA Flood Insurance mapping through the Letter of Map Amendment Revision (LOMAR) process. The LOMAR study typically will be performed when construction is over fifty percent complete.

LOW FLOW IN DELTA STREAMS

214. Rivers and streams in most of the country are in equilibrium with the surficial aquifer. During periods of heavy rainfall, water moves from the rivers into storage in the surficial aquifer. On the other hand, water moves from the aquifer into the stream during periods of less rainfall. The water that moves into streams from the aquifer is called base flow. Base flow is essential to maintaining good aquatic life communities in streams and rivers. However, when the surficial aquifer is heavily utilized for irrigation or some other consumptive use, the water level in the aquifer can fall below the stream bed, inhibiting the stream from receiving base flow from the aquifer. Figure 2-126 shows the flow duration profiles of the Big Sunflower River at Sunflower, Mississippi. The period-of-record flows have been divided into five periods in order to illustrate how the flow has changed over time. More insight into this problem can be obtained from the USGS Circular 1376, "Streamflow Depletion by Wells—Understanding and Managing the Effects of Groundwater Pumping on Streamflow." Figure 2-126 shows that the minimum flow was around 200 cfs in the 1930s through the 1940s, but, during the next three decades, the minimum flow diminished to just under 100 cfs. By the 1980s and 1990s, the minimum flow (one percent duration) had diminished to around 20 cfs, which is a 90 percent reduction from when it was first measured in the mid-1930s.

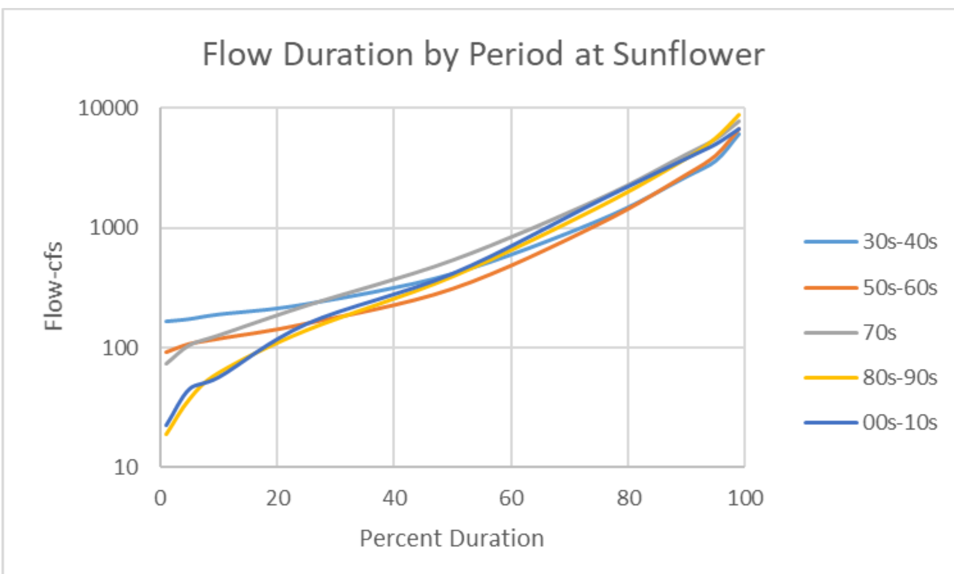

Figure 2-126. Flow Duration by period in the Big Sunflower River at Sunflower, Mississippi.

215. The observed flow depletion is most severe during the fall months, which historically receive less rainfall. Figure 2-127, Figure 2-128, and Figure 2-129 show the flow duration by period for the spring, fall, and summer months respectively. The flow data was sorted by periods, where a single period represents two decades. The exception to this is the 1970s, which are treated as one period. The 1970s was the period where flows were changing from pre-irrigation to full irrigation. In addition, the 1970s represent a very high flow decade. The 1970s

experienced four major flood years, which were 1973, 1974, 1975, and 1979. The two highest floods in the POR occurred in 1973 and 1979. From Figure 2-127 and Figure 2-128, it is evident the spring and fall flow duration profiles were nearly identical, but flows were much lower during the fall months. The spring and fall profiles show that the two most recent periods (1980 to 1999, and 2000 to 2020) have lower profiles from the one percent through the 50 percent duration. Although, the median value for spring in the most recent period (826 cfs) is only slightly less than the median for the period from 1950 to 1969 (866 cfs).

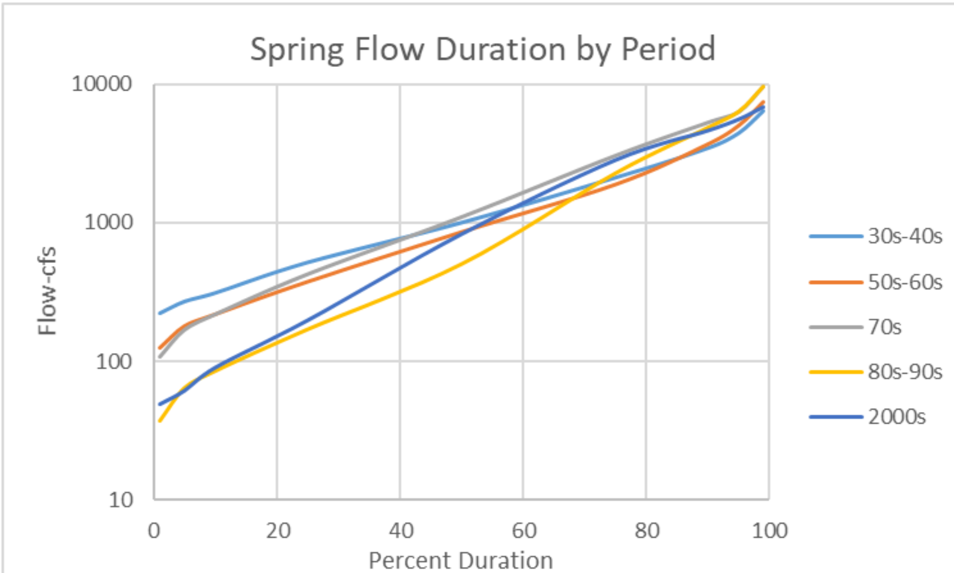

Figure 2-127. Flow duration profile for the spring months (March, April, and May).

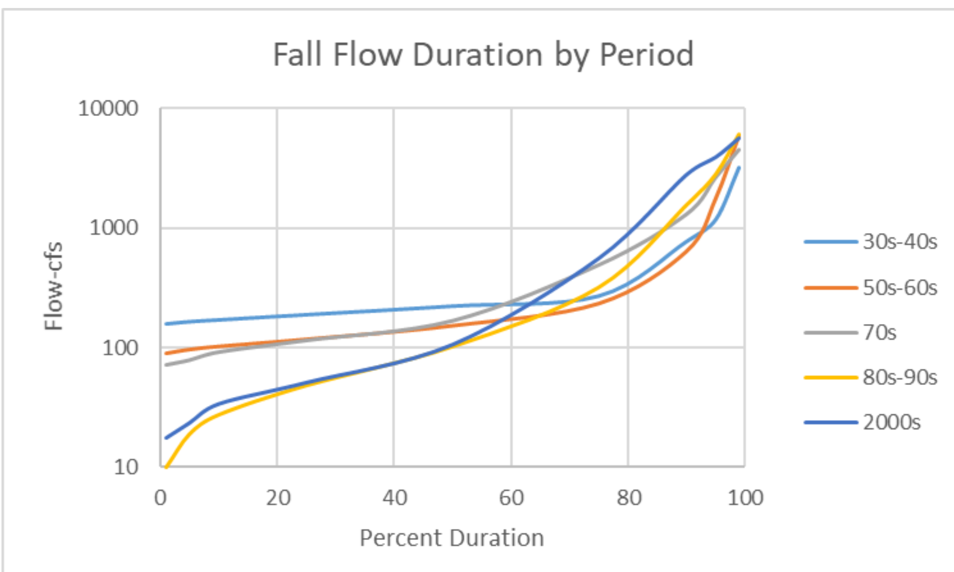

Figure 2-128. Flow Duration for the fall months (September, October, November).

216.The median flows in the two most recent fall periods are 102 and 106 cfs and are substantially less than the previous three periods, which had median fall flows ranging from 153 to 225 cfs. As low as the median flows have become, it is the one percent fall flow, which has seen the most significant declines. The one percent flow in the 1980s and 1990s was only 10 cfs. This increased slightly during the last period (2000 to 2020) to 18 cfs. In the first period (1930 to 1949) the one percent flow was 160 cfs, but this declined to 90 cfs in the next period (1950-1969) then to 71 cfs during the 1970s. The summer flow duration profile is quite different. During the summer, the more recent periods showed increased flow instead of decreased flow (Figure 2-129). This increase is due to irrigation return flow. The median flows for the five periods are respectively: 287, 202, 458, 440, and 370 cfs. Although there was small amounts of irrigation in the late 1960s, irrigation became widespread in the 1970s and has been steadily increasing since then. The entire flow profile during the summer period for the last three periods lies above the profiles for the first two periods, except for the one percent duration. These observed changes in flow are not restricted to the Big Sunflower River. Figure 2-130 shows the annual flow duration profile by decade for Bogue Phalia. It should be noted that Bogue Phalia only has six decades of flow data, which is displayed by decade instead of by period. As was observed in the Big Sunflower, the low flow end of the profiles declined by decade, with the exception of the 1970s. Again the fall flow duration profiles for the last 40 years lie below the profiles for the 1960s and 1970s from the median (50 percent duration) to the one percent duration (Figure 2-131). The fall one percent duration by decade in Bogue Phalia were 35, 53, 7, 6.4, 4.9, and 0.3 cfs respectively. These low flows represent a 90 percent reduction in fall low flow for Bogue Phalia.

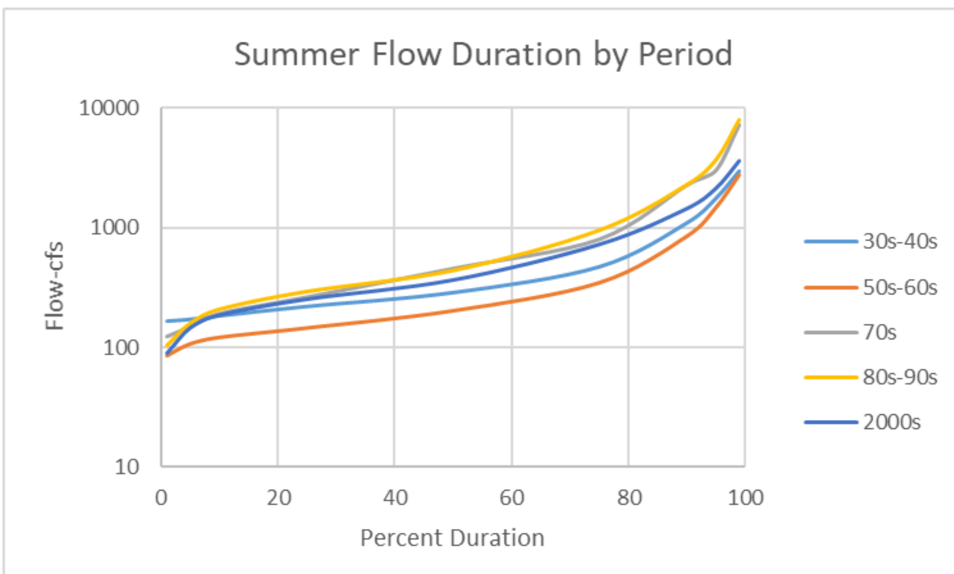

Figure 2-129. Flow duration profile for the summer months (June, July, and August).

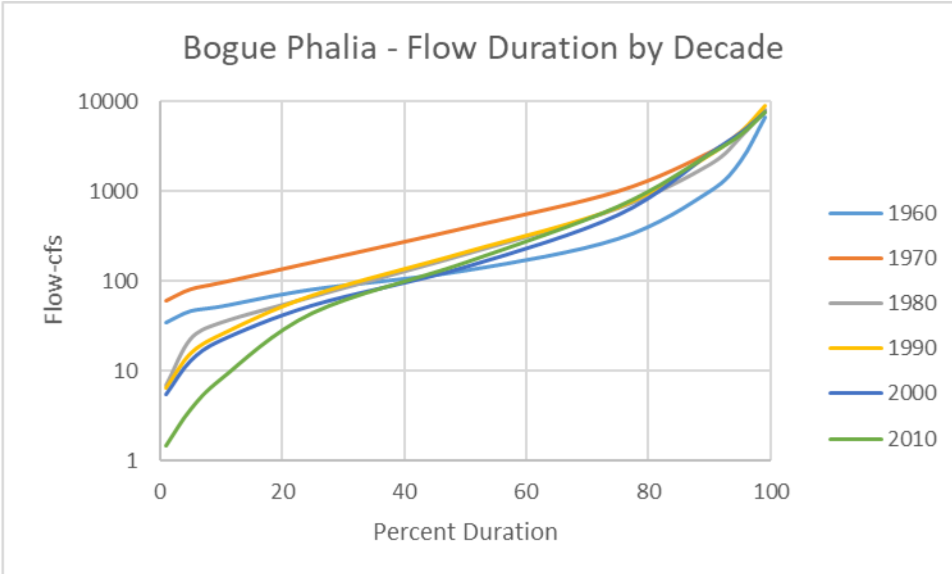

Figure 2-130. Annual flow duration profile for Bogue Phalia.

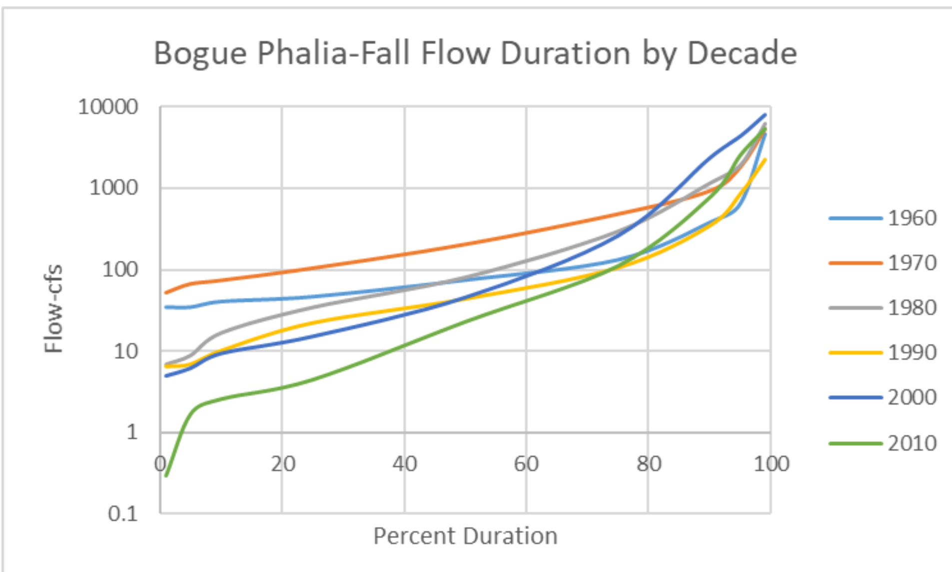

Figure 2-131. Fall flow duration for Bogue Phalia by decade.

217.Two of the goals of the Clean Water Act were to make America's surface waters swimmable and fishable. It is hard to imagine how this goal can be accomplished, when a fifth order river has less than a foot of water in the channel.

HYDROLOGIC ALTERATION

218.The previous paragraphs have described the hydrologic alterations that have occurred in Delta streams over the past forty to fifty years. These alterations are not limited to Bogue Phalia and the Big Sunflower River. These streams were highlighted because long term flow data is available with which to describe the alterations. Many smaller streams have been adversely affected by flow alteration, such that once perennial streams have become ephemeral or intermittent. The EPA has identified hydrologic alteration as a major water quality problem.

The EPA's Watershed Academy Web series has a good introduction to flow alteration entitled "How much water does a river need?" This article was provided by Brian Richter of the Nature Conservancy and is a condensed version of an article published in Freshwater Biology (Richter et al. 1997) by the same name. The second section of the Web Academy paper is essential for the understanding of the low flow problem in the Big Sunflower Basin and is include verbatim:

219."Water Quality and Water Quantity"

220."Watershed management focuses mostly on **water quality** issues, but **water quantity** is extremely important in its own right. Writing for the U.S. Supreme Court in the case *Jefferson City Public Utility District v. Ecology Dept. of Washington*, Justice Sandra Day O'Conner said that the separation of water quality from water quantity was an artificial distinction that had no place in a law intended to give broad protection to the physical and biological integrity of water. Further, she claimed that reducing water quantity (or flow) was capable of destroying all designated uses for a given body of water, and that the Clean Water Act's definition of pollution was broad enough to encompass the effects of reduced water flow. This Supreme Court decision upheld the State of Washington's right to require a minimum water flow necessary to protect salmon and steelhead and to disapprove a hydroelectric plant application that would have diminished the existing flow."

221.The EPA recognizes the essential need for minimum flows, as illustrated by the many reports published on the subject. A recent study which was conducted with the USGS was jointly published by the agencies in 2016. The report is the "Final EPA-USGS Technical Report: Protecting Aquatic Life from Effects of Hydrologic Alteration, EPA Report 822-R-16-007 or USGS Scientific Investigation Report 2016-5164 (Novak, et al. 2016). There are many activities that alter the flow in streams including: impoundments, channelization, diversions, groundwater pumping, wastewater discharges, urban development, thermoelectric power generation, and agricultural practices (EPA-USGS Technical Report: Protecting Aquatic Life from Effects of Hydrologic Alteration). Although the direct withdrawal of water for irrigation may have been the original source of flow alteration in the basin, the withdrawal of groundwater for irrigation is the primary cause of flow alteration in the Big Sunflower Basin. Since 1970, the Mississippi Department of Environmental Quality has approved the installation of more than 20,000 irrigation wells in the Mississippi Delta. The withdrawal of irrigation water over the last forty years has created a cone of depression in the groundwater centered in Sunflower and Leflore counties (Barlow and Clark 2011). The report observes, "Water-level declines also have resulted in decreases in base flow in many Delta streams to the extent that in the absence of rainfall of irrigation return flow, some stream reaches are dry during the summer months." The impact of streamflow depletion due to wells is documented in the report: "Streamflow Depletion by Wells—Understanding and Managing the Effects of Groundwater Pumping of Streamflow" (Barlow and Leake 2012). The problem of low flow or flow alteration is not new to the Mississippi Delta. The USGS first reported on the problem in a report published in 1964 (Low-Flow Characteristics of Streams in the Mississippi Embayment in Mississippi and Alabama; Speer et al. 1964). In this report, the USGS compared the low flows in Delta Streams before and after the initiation of surface withdrawals for irrigation. The report found that the 20 recurrence annual low flow for the Big Sunflower River at Sunflower dropped from 138 cfs to 89 cfs after only a few years of irrigation withdrawals. In order to compare the low flows in streams with widely differing drainage areas the low flows were normalized by dividing the observed flows in cfs by the drainage area in square miles, which yields a unit of cfs/mi^2. The baseline 90 percent exceedence flows for several locations in cfs/mi^2 were: Big Sunflower River (BS) at Sunflower, 0.24; BS at Little Callao, 0.22; BS at Holly Bluff, 0.25; Bogue Phalia at Leland, 0.17. The 90 percent exceedence flow after irrigation started yielded these flows (cfs/mi^2.): BS at Sunflower, 0.16; BS at Little Callao, 0.14; BS at Holly Bluff, 0.16; and BP at Leland, 0.11.

222. There are four major natural sources of water entering streams. They are direct precipitation falling on the stream (relatively small component), overland flow from runoff, interflow from runoff (or subsurface storm flow), and discharge from groundwater (base flow). During wet periods overland flow and interflow are the major contributors to streamflow, but during dry periods, base flow will be dominant or the only source to supply flow to a stream. Most streams are in a case of dynamic equilibrium with the groundwater. During wet periods the water level in the stream is high, and the water surface will be higher than the groundwater. During these periods water will move from the stream into the aquifer (Figure 2-132, losing stream). During dry periods, the process is reversed. The water level in the water table will be higher than the stream's surface, and water will move from the aquifer into the stream (Figure 2-133, gaining stream). In some instances, the water table can drop below the bottom of the stream, and stream is now disconnected from the aquifer and it will lose flow the aquifer all of the time (Figure 2-134, disconnected stream). When a disconnected stream has no flow, it becomes an ephemeral stream. Many of the smaller tributary streams in the Big Sunflower Basin have become ephemeral streams during the fall due to lack of rainfall (these three conditions are described in USGS Circular 1376; Barlow and Leake 2012). These three are simplified examples of the interaction of groundwater and surface water. For a more complete understanding, the reader is directed to read the three reports cited in the previous section. Figure 2-135 is from the USGS Report 2011-5019 (Simulation of Water-use Conservation Scenarios for the Mississippi Delta Using an Existing Regional Groundwater Flow Model; Barlow and Clark 2011). The figure illustrates the more complex conditions that are observed in the Big Sunflower Basin. The groundwater table is fully charged on both the left and the right of the figure. On the left, the aquifer is in direct connection with the Mississippi River, while on the right side, the aquifer receives inflow from the Bluff Hills to the East and from the Tallahatchie River. The Tallahatchie River receives discharge from the four Corps reservoirs in the Bluff Hills, and generally has ample flows throughout the year. The figure shows examples of both connected and disconnected streams. The center of the zone of depression in the aquifer lies between the Big Sunflower and Quiver Rivers. This area has a thick layer of clays which extend fifty to sixty feet below the surface. The subsurface geology of the area was mapped by Fisk, et al. 1944 and later by Saucier, 1997. Due to the thick layers of clay on the surface the area is dominated by rice and catfish production. Both use much more water than normal crops. Rice uses 36 to 42 inches per acre per year, while catfish uses more than five feet per acre per year. The combination of high water use and low infiltration rates has resulted in a severe drawdown of the alluvial aquifer in that region.

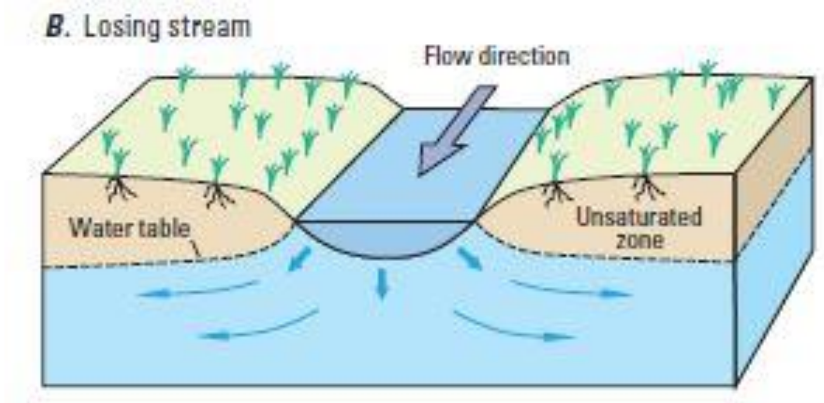

Figure 2-132. Losing Streams, (USGS, Circular 1376).

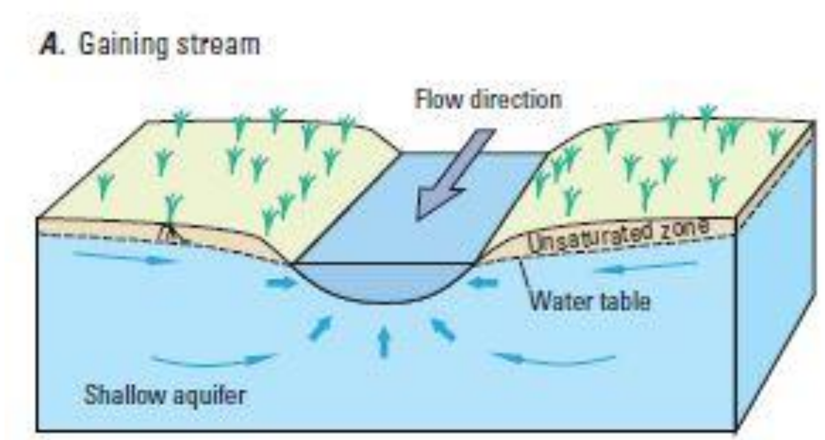

Figure 2-133. Gaining Streams, (USGS, Circular 1376).

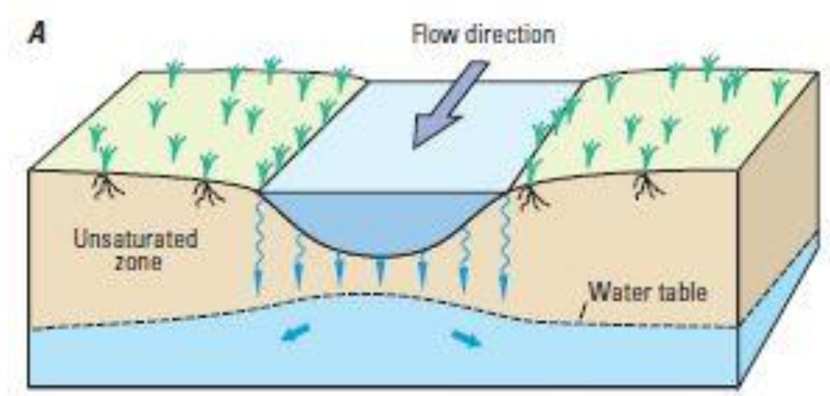

Figure 2-134. Disconnected Streams (USGS, Circular 1376).

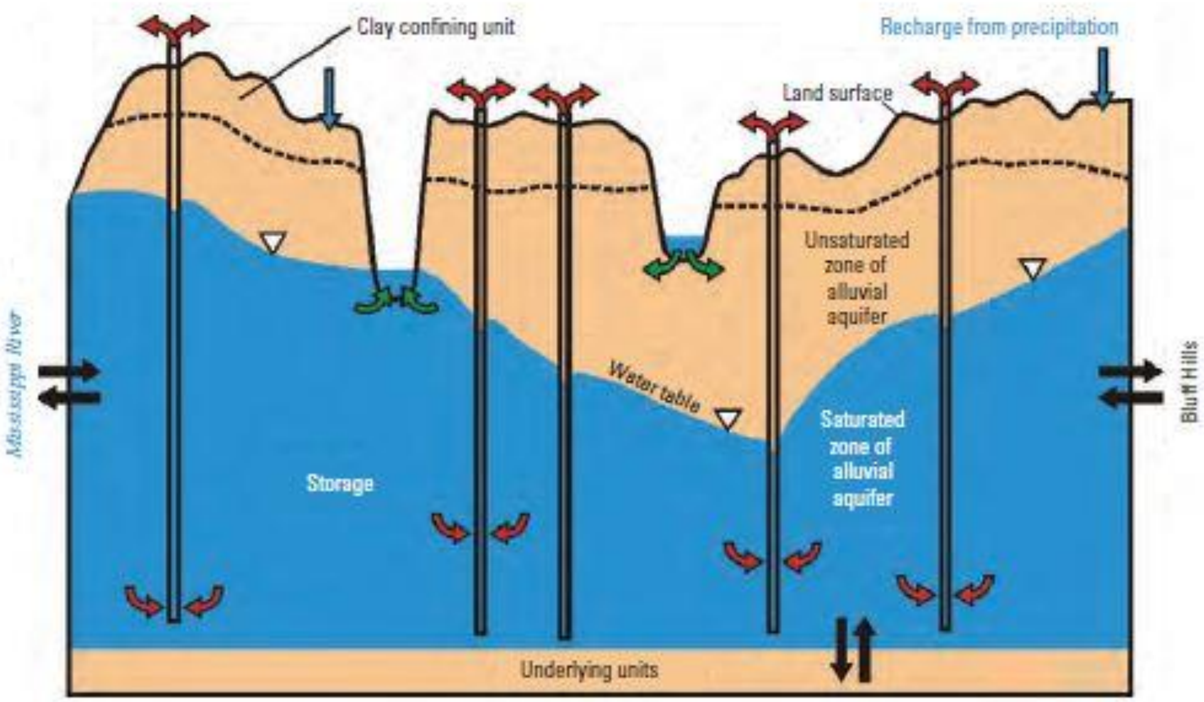

Figure 2-135. Profile of the Mississippi Alluvial Aquifer in the Mississippi Delta (USGS, SIR 2011-5019).

223. Several years ago, the USGS and Corps entered into a cooperative agreement to maintain several paired groundwater-surface water gages. These paired gage locations have greatly extended our knowledge of the interactions between the groundwater and surface water in the basin. Seven of these paired gages are located in the Big Sunflower and Steele Bayou Basins. Four gages are located at Big Sunflower River locations, which are from north to south: Clarksdale, Merigold, Sunflower and Anguilla. A fifth gage is located on Bogue Phalia at Leland. Groundwater data from the upper most (Clarksdale, Figure 2-136) and the lower most (Anguilla, Figure 2-137) show that the groundwater and surface water are fully connected. When the surface water level increases the groundwater table also rises. During the summer the ground water levels are above the stream levels and the groundwater is discharging into the river maintaining base flow. The paired gages at Sunflower show that the aquifer is below the level of the surface gage, but that it does show increases in the water surface level during periods of high stages. However the Sunflower and Merigold gages (Figure 2-138 and Figure 2-139) show an aquifer completely disconnected from the surface stream. The groundwater at these two gages do show increases, when stages are high, but the water surface stays well below the surface of the river. Bogue Phalia is west of the Big Sunflower River and outside of the zone of depression in the alluvial aquifer.

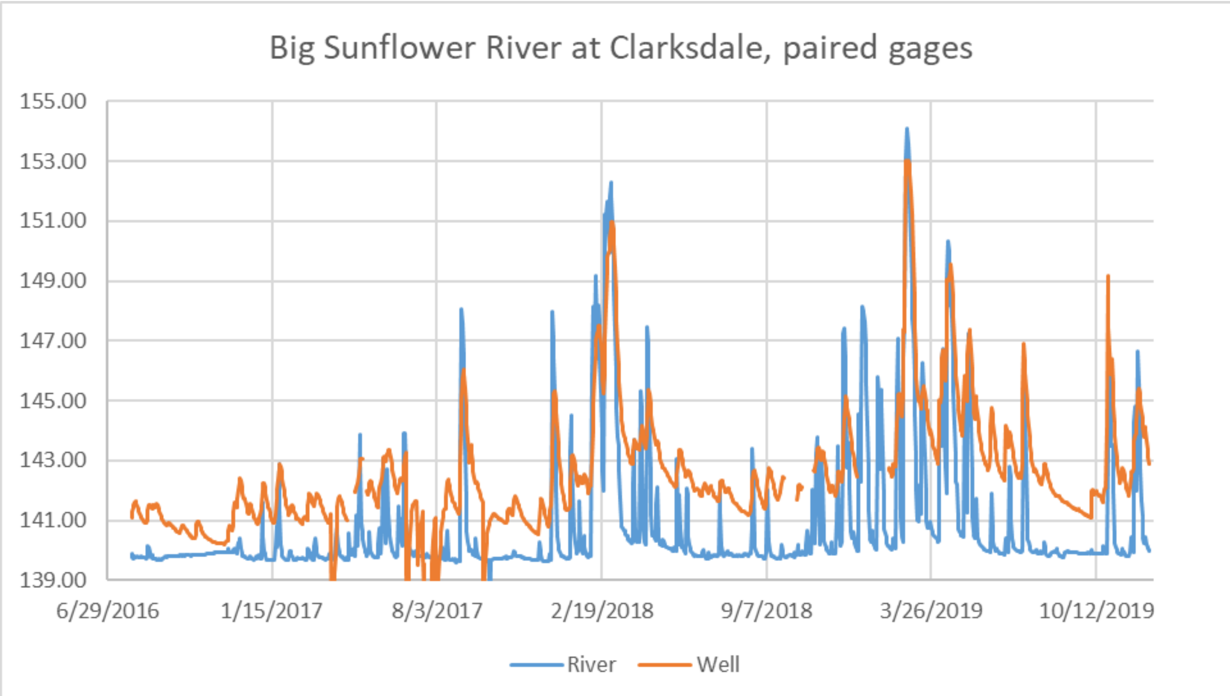

Figure 2-136. Paired gages for the Big Sunflower River at Clarksdale.

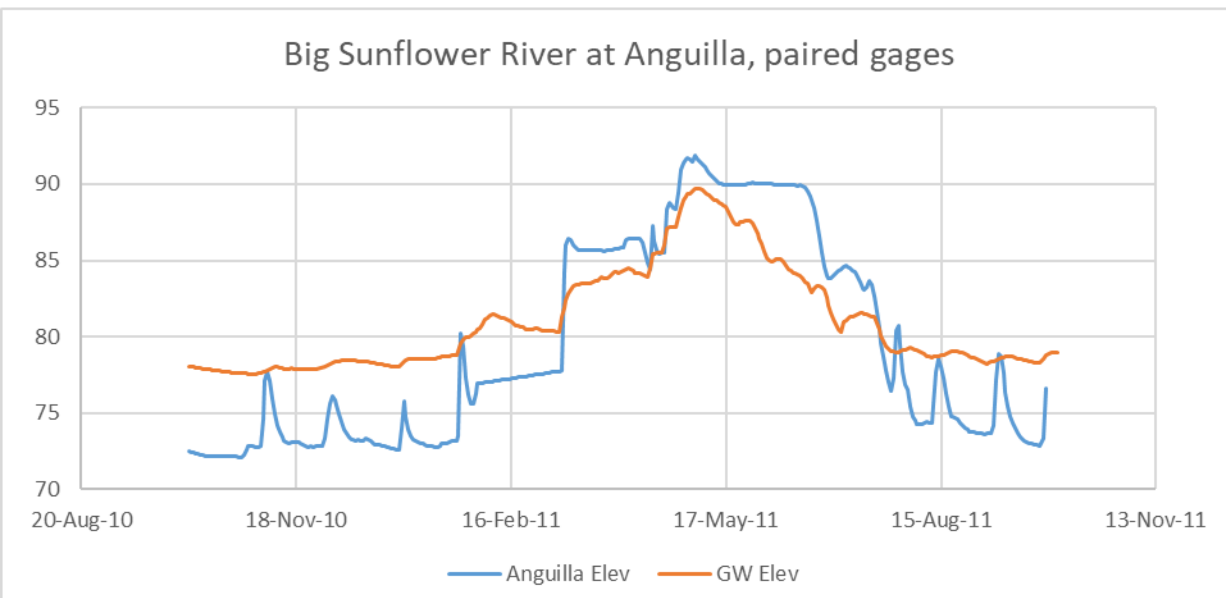

Figure 2-137. Paired gages for the Big Sunflower at Anguilla.

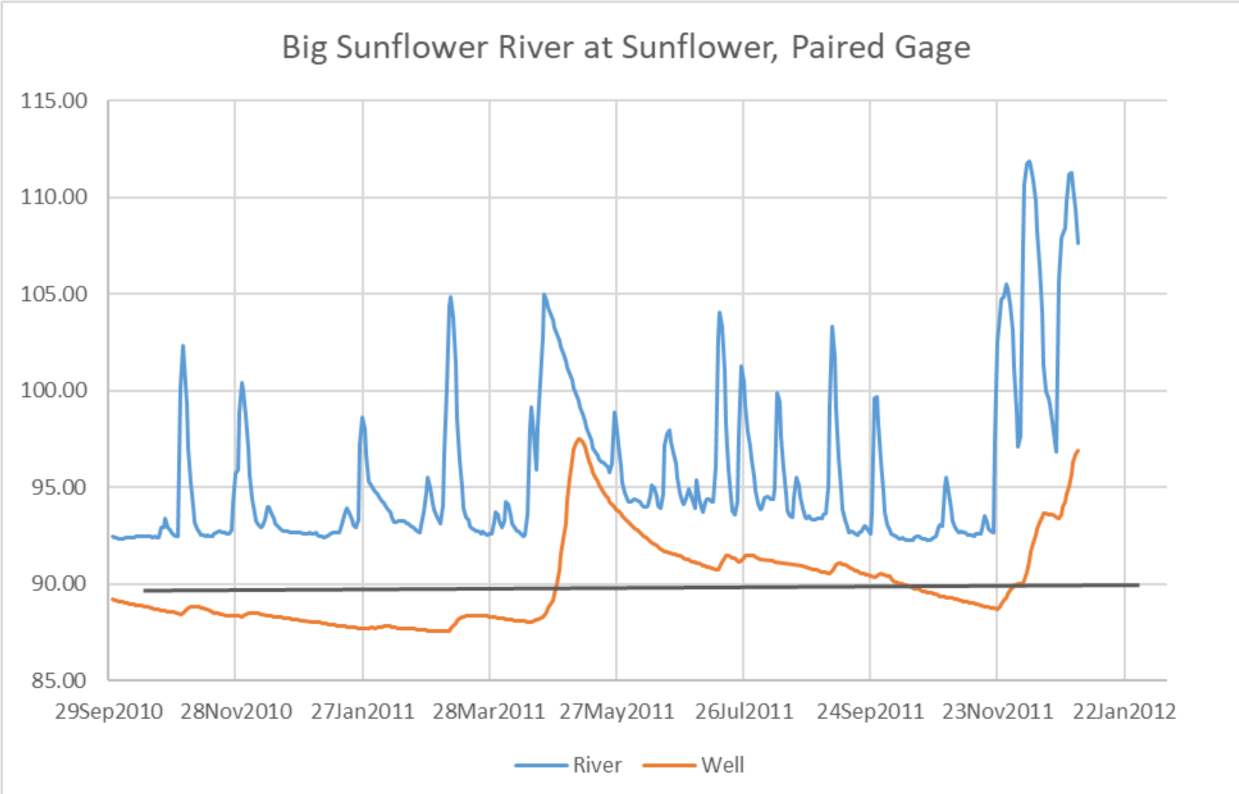

Figure 2-138. Paired gages for the Big Sunflower River at Sunflower.

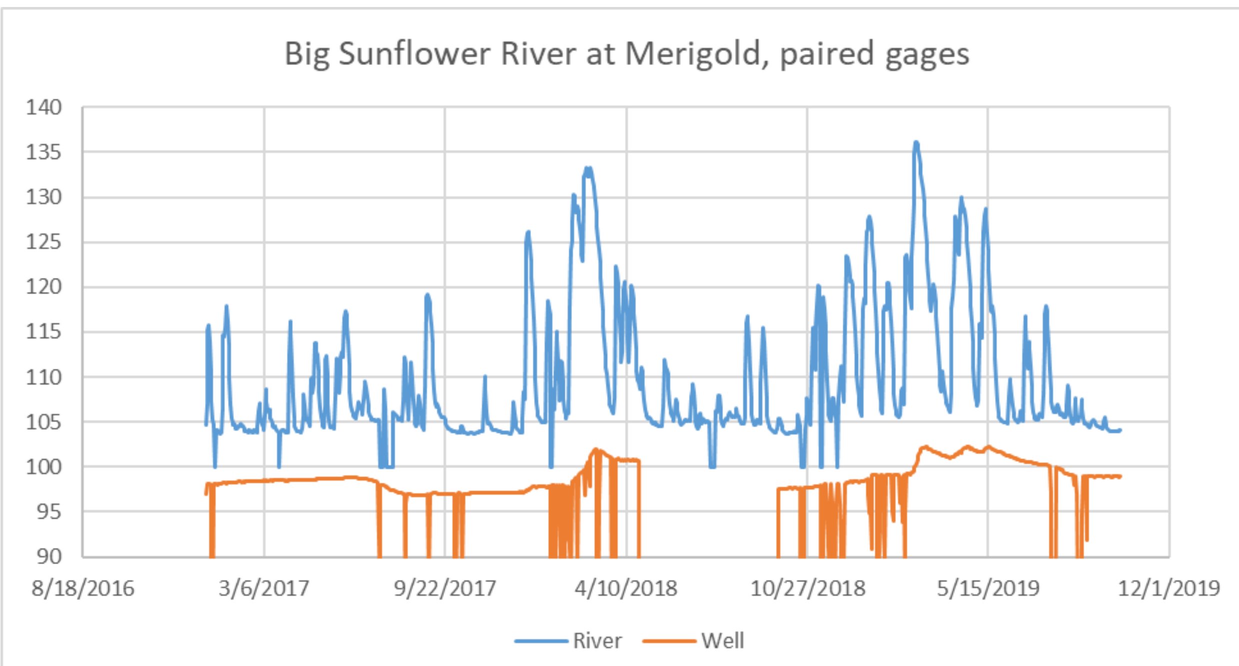

Figure 2-139. Paired gages for the Big Sunflower River at Merigold.

224.Figure 2-140 displays a hydrograph for Bogue Phalia at Leland. It shows that the water surface of the groundwater is above that of the river during summer and fall, which means that Bogue Phalia is both a losing and gaining stream at some period of each year. These figures illustrate that the conditions within the Big Sunflower Basin are variable. In some locations the rivers and the aquifers are connected, while in other locations they are clearly disconnected.

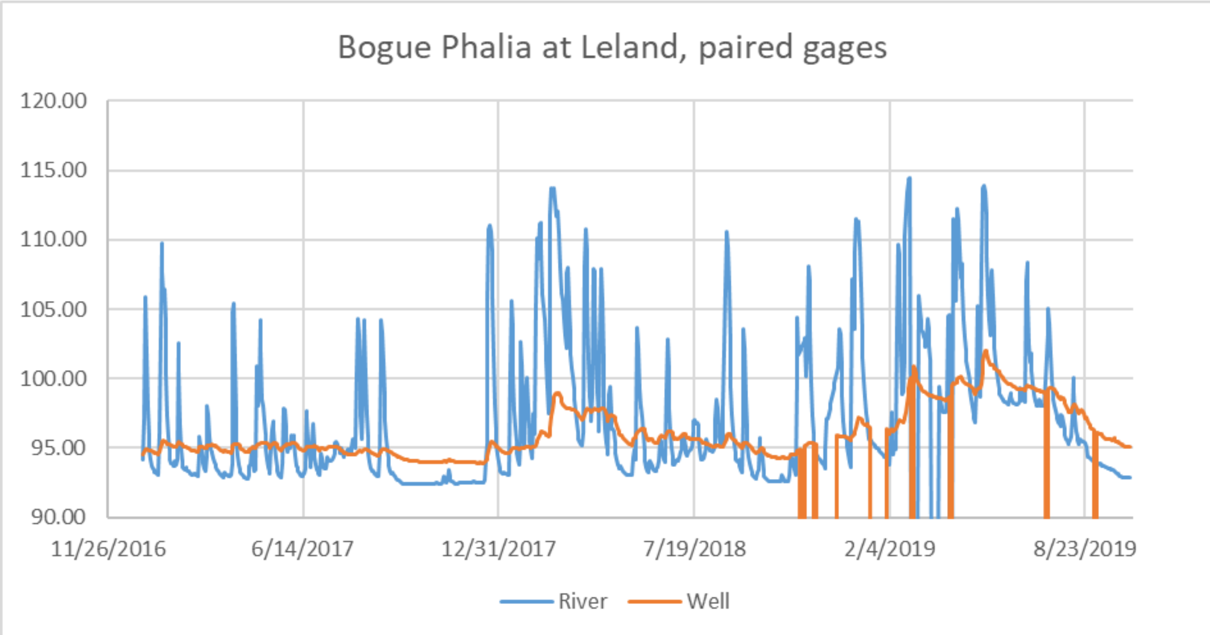

Figure 2-140. Paired gages for Bogue Phalia at Leland.

225.The final figure (Figure 2-141) illustrates the effect that disconnecting the aquifer from the surface stream has impacted flows during the fall low flow season. The median flow has dropped from over 220 cfs in the 1930s and 1940s to around 100 cfs today. The decline in the 90 percent exceedence flow (10-percent duration) is even starker (Note, SAS sorts flows from highest to lowest, thus the percent exceedence flow is obtained by subtracting the percent duration from 100). Initially, the 10 percent duration was around 200 cfs, but it has fallen to between 20 and 30 cfs during the last 40 years.

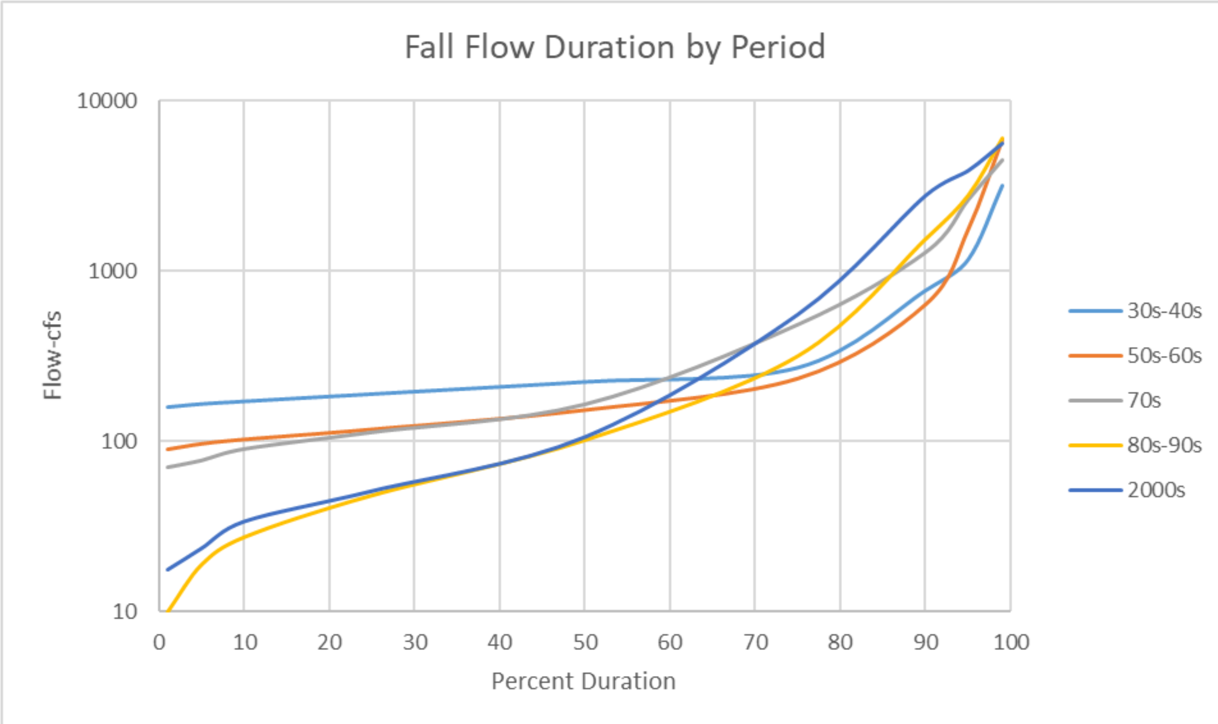

Figure 2-141. Fall flow duration for the Big Sunflower River at Sunflower.

FLOW AUGMENTATION

226.Early uses of flow augmentation were to improve water quality or to improve water quantity to ensure the water quality was maintained. The Federal Water Pollution Control Agency, in Atlanta, GA contracted with the University of Florida (Final Report to Southeast Region, FWPCA, Sep 1969, A Model For Quantifying Flow Augmentation Benefits; Pyatt et al. 1969) to examine the cost benefit of augmenting flow compared to the increased costs of waste water treatment. One of the EPA first reports dealt with flow augmentation, "Water Quality Control Though Flow Augmentation" (Heidelberg College, Biology Department 1971). Again, the emphasis of the study was improving water quality.

227.The Corps has implemented several programs over the years to try and improve fisheries habitat in the basin, but none have shown any significant improvements. In 1968 the Big Sunflower Lock and Dam upstream of the Little Callao gage on the Big Sunflower River was converted into a weir. The weir increased the minimum water surface by about seven feet. In the early 1980s, the Corps started holding the minimum elevation at the Steele Bayou structure to between 68.5 and 70 feet. This change increased the minimum water level by up to 15 feet in the lower basin. Prior to this change some channels used to go dry during extreme low flow conditions brought on by low flow in the Yazoo and Mississippi Rivers. The Steele Bayou side of the basin has three weirs in the Steele Bayou channel to provide minimum water depths during low flow periods. Finally, the Upper Steele Bayou Basin has seven additional low flow weirs to improve fisheries habitat and reduce channel maintenance. These weirs have provided some benefit to fisheries in the upper Steele Bayou Basin. There was a measured increase in species richness after project completion. The greatest increase over time occurred in Steele Bayou where species richness was over 50 percent higher post-project. The pre-project fish community consisted of 20 species, whereas 30 species occurred post-project. Increase in richness was due principally to pre-project absence and post-project colonization by intolerant species: threadfin shad, golden topminnow, bantam sunfish, ghost shiner, and speckled chub. Large numbers of inland silverside and threadfin shad indicate substantial zooplankton populations, golden topminnows and bantam sunfish, the availability of structurally complex habitats (vegetation, woody debris) and persistent slack water, and ghost shiner and speckled chub, moderate water velocities. In addition, benthic species such as slough darter were collected for the first time indicating firmer, more stable substrates. Commercial fishes were documented in the system (buffalo) and nest-building sunfishes increased (warmouth, bluegill, dollar sunfish). Largemouth bass were collected only post-project. Largemouth bass are rarely collected in Yazoo delta streams, so their presence in USBS, along with other intolerant species, suggests beneficial effects of increased water levels and more stable substrates. These improvements are presented in a Technical Note (Kilgore et al. 2008). However, weirs do not help solve low DO problems above the weir, but they generally improve DO downstream of the weir. Increased channel depths don't increase DO levels either. Because all of the past attempts to improve fisheries habitat have only led to marginal success, other restoration techniques should be considered, targeting the limiting factor suppressing fisheries improvements – environmental flows. Flow augmentation has been successful in many streams, but flow augmentation is usually done downstream of dams. As there are no dams available, we are suggesting that a series of wells be installed to provide an improved low flow. The Yazoo Mississippi Delta Water Management District (YMD) experiment with flow augmentation during the fall of 1993. That experiment is documented in an article titled, "Augmentation of Low Flows of The Upper Sunflower River," by Dean Pennington (Pennington 1993). YMD later started paying landowners to discharge water from irrigation wells into the upper Big Sunflower River to augment low flows. In 2005, YMD installed eleven wells in the upper Big Sunflower Basin and operated them for many years to augment low flows. They used these wells to augment fall low flow (Sunflower River Low Flow Well Field Project, Pennington, YMD Website). They used these wells for over fifteen years, and they are still using these wells now. These wells increased the base flow to between 35 and 45 cfs during the fall low flow period. Although the increase in base flow at Sunflower is often less due to evaporation and infiltration losses. As mentioned above, many flow augmentation projects have been done downstream of dams. The Upper Snake River watershed in Idaho has several dams operated by the U.S. Bureau of Reclamation due to a court ruling the Bureau of Reclamation has to provide 487,000 acre-feet of water for flow augmentation each year. This water either comes from storage in reservoirs or from landowners from wells. The program was mandated by the Court to offset the incidental take of salmon and steelhead due to low flow. This low flow augmentation program is documented in the report: 2010 Salmon Flow Augmentation Program and Other Activities Associated with the NOAA Fisheries Service 2008 Biological Opinion and Incidental Take Statement for Operations and Maintenance of Bureau of Reclamation Projects in the Snake River Basin above Brownlee Reservoir, Annual Progress Report (U.S. Department of the Interior 2010). A similar report is available for the Russian River in California (Stream Flow Augmentation Agreements to Benefit Salmonids-A Collaborative Drought Response in the Russian River; National Marine Fisheries Service 2015). Like the Snake River study, this study documented the use of several different methods of flow augmentation, which included flow from reservoirs, flow from wells, and reduced use of water by adjacent vineyards. The actions in this study were initiated during a drought to protect juvenile salmon and steelheads. Another example is in the Spring Creek sub-basin of the Flint River in Georgia. Prolonged droughts and increased water demand were adversely affecting low flow in Spring Creek. The prolonged low flow was affecting mussel populations, and in 2011 a demonstration project was initiated which used flow augmentation from wells to maintain minimum flow in Spring Creek to prevent mussel die off. This project is documented in: "An Evaluation of Streamflow Augmentation as a Short-term Freshwater Mussel Conservation Strategy" (Wisniewski et al. 2015). The internet has hundreds of similar studies, and more can be obtained by querying 'flow augmentation for fish.'

WELL FIELD AUGMENTATION

228. In order to improve habitat for fish and mussel, the Corps plans to augment flows in the Big Sunflower and Steele Bayou Basin by withdrawing water from the alluvial aquifer using wells located near the Mississippi River Mainline Levee. The plan would install up to 34 wells in five sub-basins. Figure 2-142 shows the potential locations of the wells. The final locations cannot be determined until after the project is approved and funds are provided by Congress. Well locations will then be negotiated with the individual landowners. The wells will be sited as close as practicable to the preliminary locations shown in this document. Locations could change depending on cultural and HTRW investigations, minimizing environmental impacts, lack of adequate electrical power at the site, or to facilitate construction. The sub-basins are Harris Bayou, Hushpuckena River, Bogue Phalia, Deer Creek, and Steele Bayou. The wells in the Harris Bayou and Hushpuckena River watersheds would supplement low flows in the upper Big Sunflower River from below Clarksdale to below Indianola. The wells in the Bogue Phalia Basin would augment flows in the middle Big Sunflower River from just above the Little Callao gage to below the Anguilla gage. The wells in the Deer Creek sub-basin would augment flows in the lower Big Sunflower Basin through Rolling Fork Creek. Finally, the wells in the upper Steele Bayou Basin would augment flows in Main Canal, Black Bayou, and Steele Bayou. The wells would only be operated during the fall low flow period after irrigation return flows cease. Depth transducers will be installed in each sub-basin, and pumping would be started and stopped based on observed water surface elevations. The wells will not be operated during medium or high flow events, and they definitely will not be operated during flood events. Minimum flow targets will be established for downstream locations, and the number of wells operated will vary so that the target flows are achieved. The minimum flows will be established through the Adaptive Management Program for this project. The wells will be located in areas near the Mississippi River levee to minimize possible impacts to the alluvial aquifer. The groundwater elevation will be monitored at all sites to evaluate the impact of well usage to the aquifer. All wells will be located outside of the current zone of depression in the groundwater table. Figure 2-143 and Figure 2-144 shows the fluctuations in the groundwater elevation at three wells near Greenville, MS, with a hydrograph of the Mississippi River at Greenville for the same period of time. The figure shows that the water surface in the wells goes up and down with the Mississippi River. The water surface of the Mississippi River fluctuates by about 40 feet annually, but the wells water surfaces only change about 10 feet each year. Figure 2-145, shows the annual fluctuations in the groundwater depth at wells with increasing distance from the Mississippi River. The annual fluctuation decreases with increasing distance from the Mississippi River. The plan places most wells within five miles of the Mississippi River so that the aquifer will be recharged at those locations each year. The planned peak flows for each sub-basin will amount to approximately one to two percent of peak flows. Water depth will be one to two feet at each

site, but the ultimate minimum flows and depths will be determined by the Adaptive Management Program.

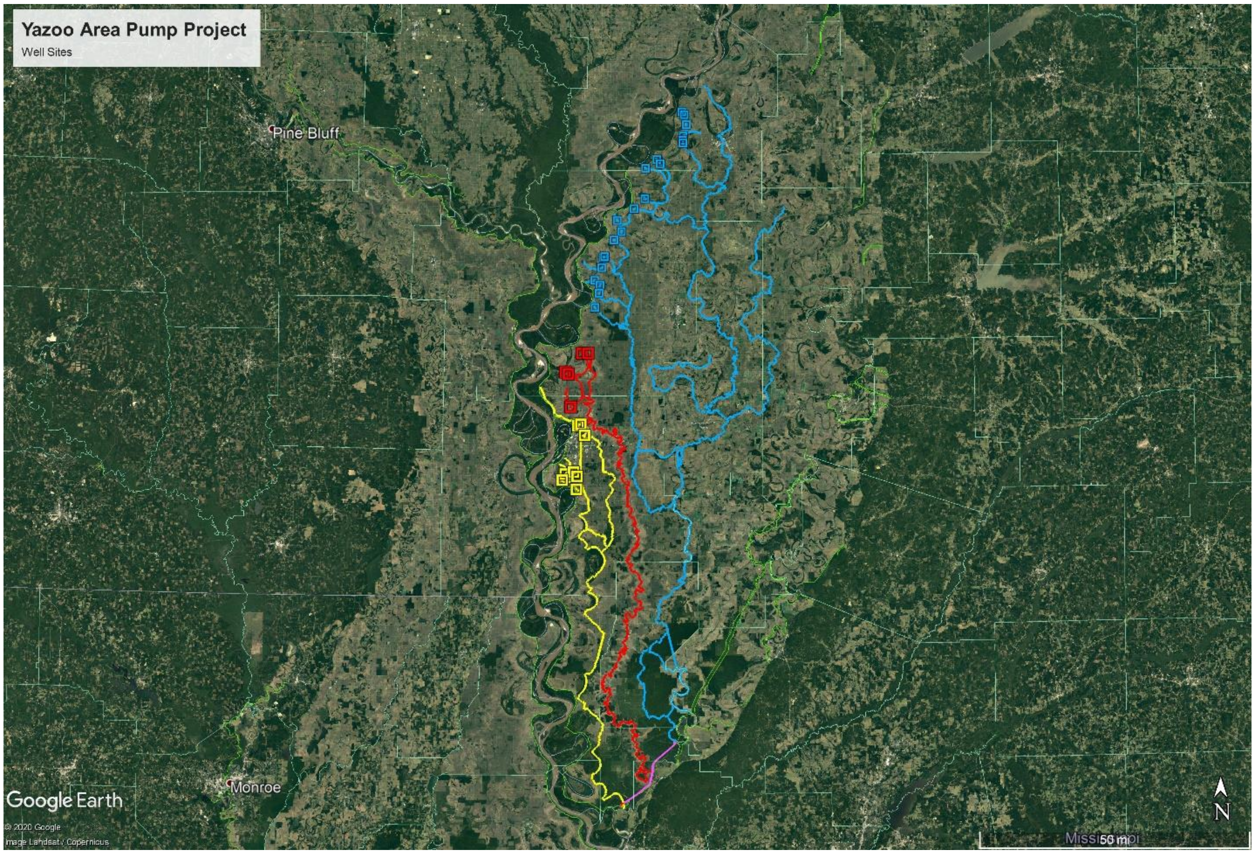

Figure 2-142. The potential locations of the wells.

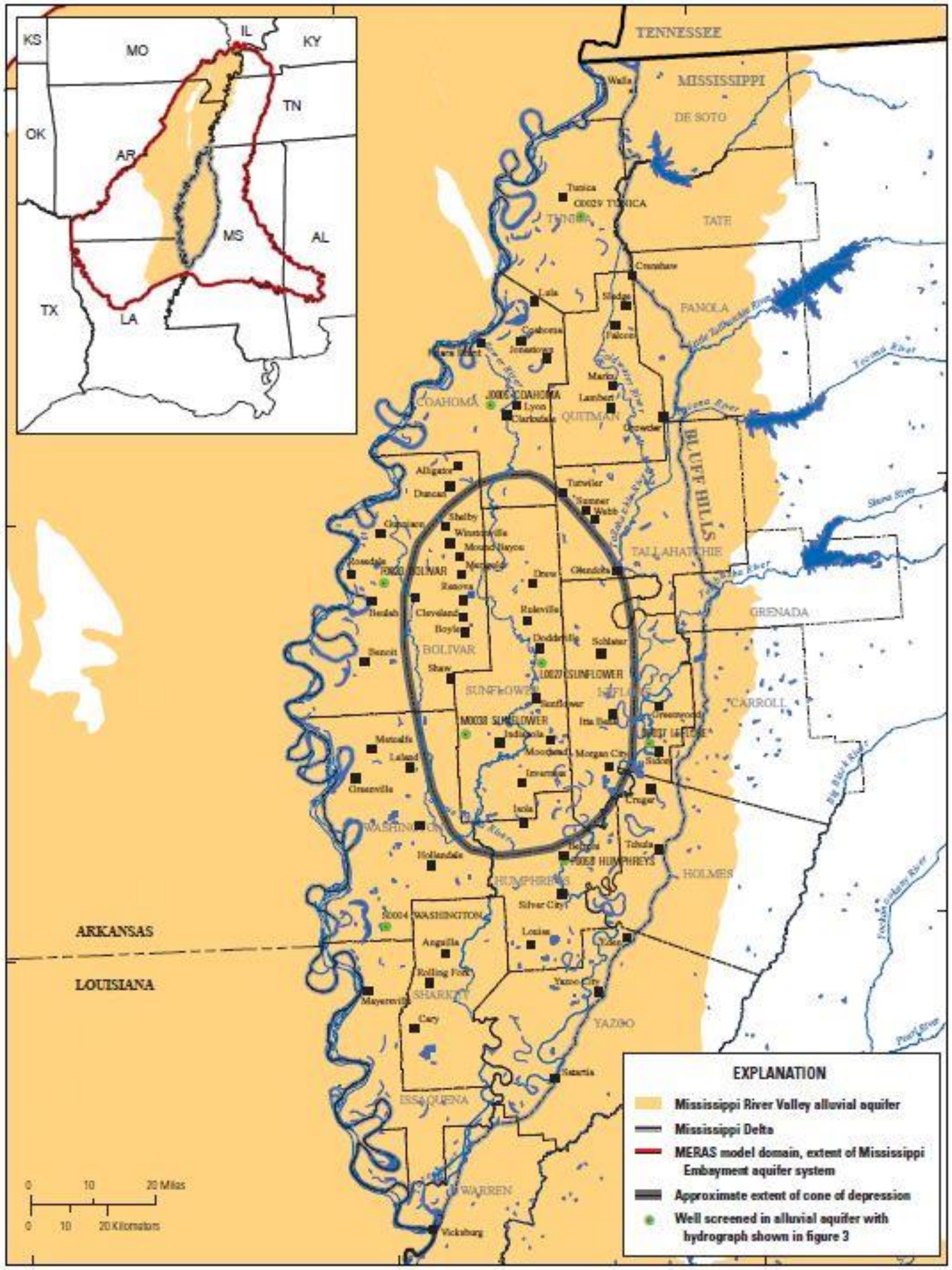

Figure 2-143. Location of the zone of depression in the alluvial aquifer. From "Simulation of Water-Use Conservation Scenarios for the Mississippi Delta Using an Existing Regional Groundwater Flow Model, USGS Scientific Investigations Report 2011-5019.

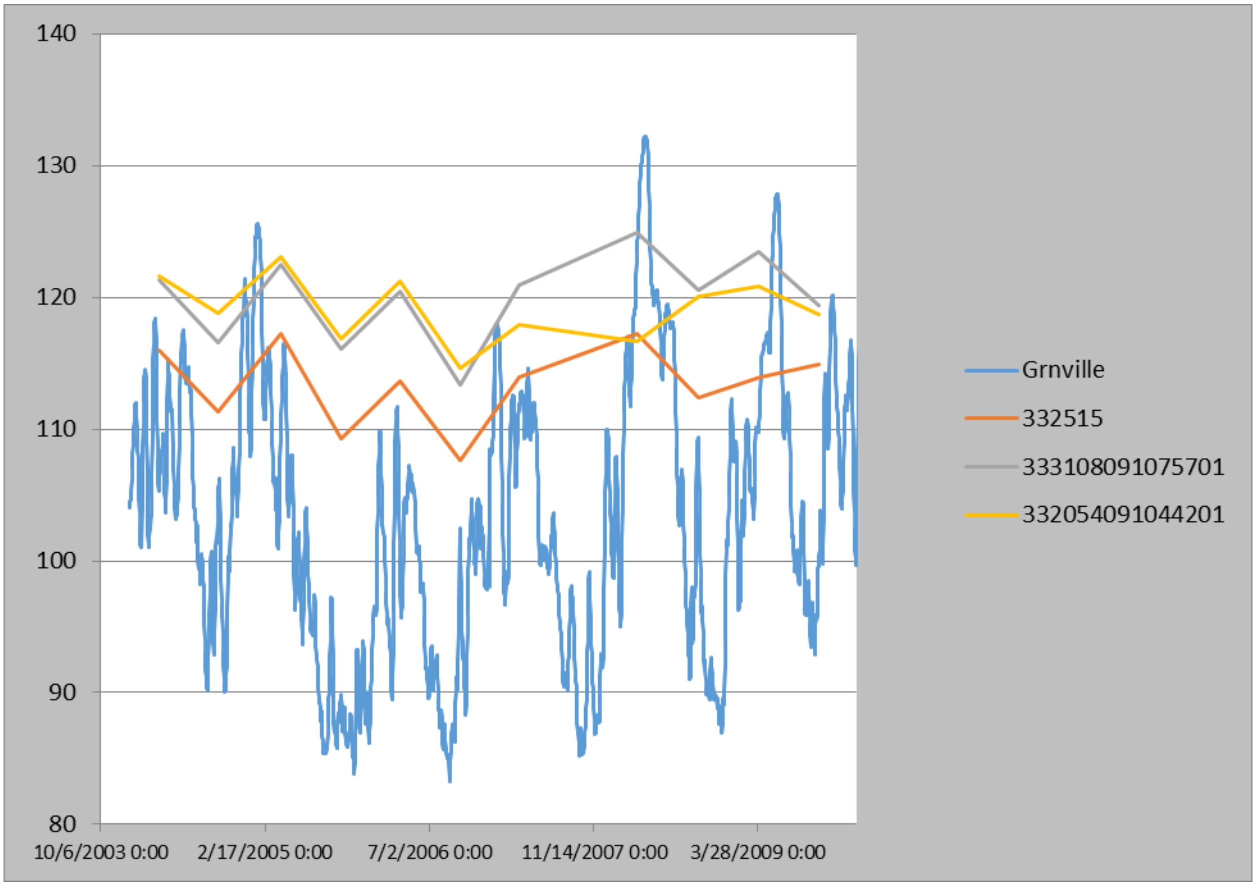

Figure 2-144. Groundwater elevation compared to the Mississippi River water surface elevation at Greenville, MS.

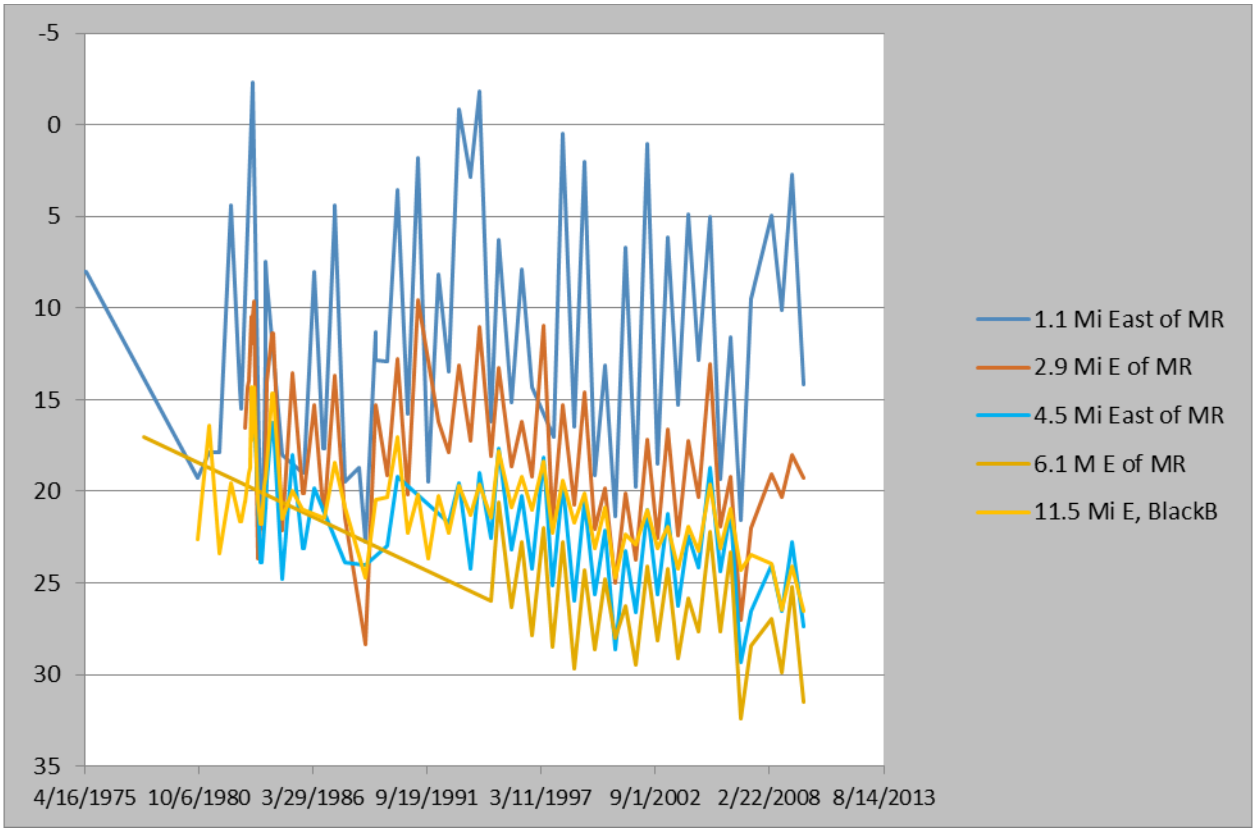

Figure 2-145. Fluctuations in groundwater surface with distance from the Mississippi River.

229. Supplemental flows from groundwater wells during low flow conditions would improve water quality, mussel survival, and fish recruitment. Changes from an intermittent condition to perennial flows will increase dissolved oxygen concentrations, biochemical processing, and carbon export. Increases in wetted perimeter due to establishment of environmental flows will provide adequate water to avoid desiccation of established mussel beds and reduced mortality associated with elevated water temperature during low water conditions. Mussels are widespread and abundant in the Big Sunflower-Steele Bayou drainage, and include regional and federally protected species. Elevated flows will facilitate periodic fish passage flows over weirs for spawning movements, recolonization of fish, and an overall increase in fish species richness. Infected mussel host fish could also access new areas of suitable habitat for mussel colonization leading to population expansion. Improvement in water quality and macroinvertebrate production in summer and fall may improve the condition factor of fishes increasing survivorship. Improved health and condition would transfer to the spring spawning period and positively benefit annual recruitment cycles. This approach offsets the high mortality of larvae and juvenile fishes occurring in the spring during hypoxic events with increased survival rates of juvenile and adult fishes during autumn and fall.

SECTION 3 - ENGINEERING AND CONSTRUCTION

PURPOSE

230. The purpose of this Engineering and Construction Section is to provide a site description and document engineering studies performed on the design, operation, and maintenance of the pump station located in Warren County, Mississippi.

PROJECT DESCRIPTION

231. The Yazoo Study Area is located in west-central Mississippi and is bordered by the left descending bank of the mainline Mississippi River levee on the west, the west bank levees of the Whittington Auxiliary Channel, the connecting channel, on the east, and the Yazoo River on the south. The area, which includes portion of Humphreys, Issaquena, Sharkey, Warren, Washington, and Yazoo counties, Mississippi and part of Madison Parish, Louisiana, contains approximately 926,000 acres. In addition, this area is subject to headwater flooding from the Yazoo and Sunflower Rivers and backwater flooding from Steele Bayou that is induced from high stages on the Mississippi River. The proposed location of the pump station is located in Warren County, Mississippi. The site lies between the Yazoo River and the Yazoo Diversion Canal along the Yazoo Backwater Levee, approximately three miles northeast from the intersection of Highway 465 and Highway 61.

REGIONAL GEOLOGY

PHYSIOGRAPHY - TOPOGRAPHY

232. The pump station is located near the southern limits of the Yazoo Basin, a subprovince of the Mississippi Alluvial Valley. The Yazoo Basin is bounded by the Mississippi River on the west and the Bluff Hills on the east. The surface of the Yazoo Basin consists mainly of an intricate network of meander belt (point bar, abandoned channel, abandoned course, and natural levee) deposits. The point bar deposits, which form the ground surface of almost all of the proposed sites, exhibit an undulating surface of ridges and swales partially covered by remnant natural levees. Natural ground surface elevation in the vicinity of the proposed sites ranges from 85 to 95 feet (NGVD 29).

STRATIGRAPHY

233. The geologic formations present at the proposed project site consist of Quaternary alluvium, underlain by Eocene Yazoo Clay. The alluvium is divided into topstratum deposits, which overlay substratum deposits. The topstratum consists of fine-grained silts, clays, sandy silts, and silty sands, which were deposited by vertical accretion of sediments. In general, the topstratum deposits average approximately 30 to 40 feet in thickness. The clays within the topstratum are generally tan, brown or gray in color, and soft. The silts and silty sands are generally tan, loose to dense, and contain minor traces of organic matter. The substratum is comprised of a thick deposit of fine sand that grades downward into coarse sands and sandy gravels. There are lenses of silty sands and perhaps silty clays that are occasionally encountered in the substratum. The contact between the topstratum and substratum is highly irregular and reveals channels of topstratum incised into the substratum. The substratum overlies an eroded surface of Tertiary

formations within the entrenched Mississippi Alluvial Valley. At the proposed site, the substratum is underlain by the Yazoo Formation. The Yazoo Formation consists of gray to grayish-green, highly plastic, virtually impervious clay (CH) with thin zones of silty clay irregularly dispersed throughout the section. The Yazoo Formation is generally considered to be a regional aquiclude.

STRUCTURE

234. The proposed project site is situated about 15 miles west of the structural axis of the Mississippi Embayment. Much of the Mississippi Embayment is underlain by extensions of the Ouachita Mountain fold belt of Paleozoic age. There are numerous major structures, such as fault systems, basins, uplifts, salt domes, etc., of various ages within the Mississippi Embayment. However, no major structures lie within the proposed project area. The established trace of the Pickens-Gilbertown Fault System extends from Gilbertown, Alabama, through Pickens, Mississippi and terminates near the axis of the Mississippi Embayment, approximately 20 miles northeast of the study area. The study area is situated a few miles south of the Monroe Uplift-Sharkey Platform, along the west limb of the structural embayment, where the formational dip resides to the southeast. Surficial evidence of a northwesterly trending fault exists along Bluff Creek in the Bluff Hills, approximately four miles north of Vicksburg, and is referred to as the Bliss Creek Fault. The Bliss Creek Fault is reportedly Tertiary in age, meaning that only the Tertiary deposits have been disturbed, whereas the overlying surficial deposits have not been disturbed. This observation indicates that movement along the fault has not occurred since Tertiary time. The northwesterly extent of the Bliss Creek fault is not known because the Tertiary surface is covered by more than 100 feet of alluvium. A straight line northwesterly projection of the fault from Bliss Creek places the fault trace a few miles southwest of the proposed project site. The questionable extent of the fault; the apparent inactivity of the fault since Tertiary time; and the fact that the Tertiary surface is covered by more than 100 feet of alluvium in the area of the site are considered sufficient reasons for dismissing the Bliss Creek Fault as a threat to the project.

TECTONICS AND SEISMOLOGY

235. The New Madrid earthquakes of 1811 and 1812 are generally considered to be the most powerful earthquakes in United States history; were rated approximately XI on the Modified Mercalli (MM) scale; and had a body-wave magnitude of approximately 7.2. Subsequent record keeping and more recent seismic monitoring shows that the New Madrid area continues to be an active earthquake area. During the 1950's, more than ten earthquakes were recorded in the New Madrid area, with intensities of MM of V or VI. The numbers and intensities were similar during the 1960's and 1970's. Record keeping and seismic monitoring led to the development of earthquake zones across the United States, relative to occurrences and intensities of the earthquakes. The generally accepted southern limit of the New Madrid earthquake zone lies near Marked Tree, Arkansas, northwest of Memphis, Tennessee (about 225 miles from the project site). In the area of the project site, earthquakes should be infrequent and of low intensity, if they occur.

HYDROGEOLOGY

236. The proposed project area is ultimately drained into the Mississippi River through numerous rivers and streams. The Yazoo River traverses the area from the northeast to southwest and enters the Mississippi River at Vicksburg. Deer Creek, Big Sunflower, and Yazoo Rivers drain most of the area to the north and west of the site. The fine-grained topstratum overlies the more permeable sands and gravels of the substratum. The hydraulic connectivity of topstratum and substratum is dependent on the thickness, lenticularity, and permeability of the topstratum material. Permeable sand lenses that are overlain and underlain by clay should be considered as hydraulically connected to the substratum during high water events and may develop perched water table conditions at low water stages. Pressure head in the alluvial aquifer will fluctuate considerably and is primarily controlled by the stages on the Yazoo River. It is anticipated that a water table elevation above 100 feet (NGVD 29) may exist when the Mississippi River and Yazoo River stages are at Project Design Flowline.

SITE GEOLOGY

GENERAL

237. An interpretation of the local geology is presented in ERDC (1979). The general descriptions of the geology at the proposed location is provided in the following sections. The descriptions are primarily based on information contained in ERDC (1979) and available boring data associated with the construction of the Yazoo Backwater Levee. Additional soil boring data will be necessary to provide site specific geologic data, along with the vertical and lateral extent of the deposits.

TOPSTRATUM

238. The proposed Deer Creek site is underlain by alluvial natural levee, point bar, and abandoned course topstratum deposits. The natural levee deposits, which are thickest (highest) near Deer Creek, extend approximately 1,000 feet from either side of Deer Creek and become progressively thinner with distance. Along the banks of Deer Creek, the natural levee deposits attain a maximum thickness of approximately 10 to 15 feet. The natural levee deposits are composed of fine, sandy silt (ML) and soft to stiff, gray, clay strata (CH-CL). Near the center of this proposed site, Deer Creek flows through an abandoned course of a larger ancestral meandering stream. Deer Creek is a unique stream in the Yazoo Basin. It has angular bends and a deep, narrow channel, but it has developed natural levees that are almost as wide and high as those of the Mississippi River. Deer Creek carried floodwaters from the Mound Crevasse as recently as the 1927 flood; developed its own meander belt; and eventually merged with a former abandoned stream, approximately three miles northwest of the proposed site. Data and control points regarding the nature of the abandoned course deposits associated with the abandoned stream are sparse and inconclusive. However, the abandonment of the stream appears to have been rapid, as the data suggests that the abandoned course is predominantly filled with a wedge of silt and clay. The abandoned course is 1,500 to 2,000 feet wide (north to south) and extends to a depth of approximately 40 to 50 feet. To the north and south of the abandoned course material, the proposed site is underlain with point bar deposits. The point bar deposits are composed primarily of silt (ML) and silty sand (SM, SP-SM) with subordinate amounts of clay

(CH-CL). The silt (ML) is generally gray with sand, silty sand, and clay strata. The silty sands (SM, SP-SM) are brown, fine-grained, and contain occasional clay strata. The clays are gray and brown, range from medium to hard in consistency, and contain silt strata, sand strata, and roots.

SUBSTRATUM

239.The substratum at the proposed Deer Creek site is expected to be relatively uniform in thickness and is expected to extend to an average elevation of -70 feet (NGVD 29). The average thickness of the substratum is 125 feet. The substratum is composed of gray sand (SP) with minor amounts of silty sand (SM) and silty fine sand (SP-SM). The sand is fine to medium and contains occasional silt strata, lignite, silty sand strata, and a trace of gravel. This unit may form the foundation for the proposed structure and will require dewatering prior to excavation.

TERTIARY LITHOLOGY

240.The bedrock deposits forming the floor of the Mississippi entrenched valley system consist of massive clay beds of the Yazoo Clay Formation of the Jackson Group. The Yazoo Formation is present as a narrow northeastward trending belt beneath the Yazoo Basin and consists of gray to grayish-green, heavy clay (CH), with silt strata or lenses throughout the section. This formation is a barrier to ground-water migration and is considered to be a regional aquiclude.

SECTION 4 - DESCRIPTION OF THE PROPOSED PLAN DESIGN

GENERAL

241. The Vicksburg District Design Branch has prepared updated planning-level plans and quantities with calculations in order to develop an accurate certified cost estimate for the project. The new plans and quantities include the new pump station located at the Deer Creek site and all appurtenances, the 34 supplemental low flow groundwater well fields, all required utility connections, and development of the borrow area near the Steele Bayou pump site. The Vicksburg District Design Branch also prepared right-of-way maps to determine environmental and real estate requirements.

242. The proposed pump station will be constructed at a new location approximately two miles east of the intersection of the Yazoo Backwater Levee and Mississippi State Highway 61. The pump station was modified from the previous design in order to remove unnecessary features and to update the design.

243. For the purposes of this cost estimate geotechnical data was not collected. Additionally, a survey was not conducted. Instead the ground surface was modeled based on LiDAR data. At the current stage of the planning process detailed investigations of site conditions were not possible.

244. The new design is based on the previous pump station located at the Steele Bayou pump site and was advanced to approximately a 90% complete state. The previous design was incomplete and would require redesign in order to meet current USACE guidance and code requirements. For the purposes of this cost estimate the previous design was modified, as described below, for use at the Deer Creek site.

PREVIOUS DESIGN

245. The general features of the previous design at the Steele Bayou site included:

a. A pump station intake structure composed of reinforced concrete monoliths and including a trash rack, a trash raking system, an access bridge, and an intake stoplog system

b. A pump station substructure composed of reinforced concrete monoliths and including formed suction intakes, intake and discharge gate systems, a discharge stoplog system, access tunnels, and a floodwall

c. A pump station superstructure composed of a reinforced concrete building and truss roof system with exterior brick facade, including a 40-ton bridge crane

d. A service bay composed of reinforced concrete monolith and a reinforced concrete building and a truss roof system, stairwell access to tunnels, rolling door, and other maintenance items

e. A control building composed of reinforced concrete monolith and reinforced concrete building and truss roof system, stairwell access to tunnels, office and conference room space, control room, storage rooms, restrooms, and elevator;

f. Reinforced concrete wingwalls on both the intake and discharge sides

g. Reinforced concrete floodwalls

h. Vertical lift pumps and diesel-fueled engines, including speed reducers and cooling systems

i. A fuel transfer dock and fuel storage area composed of two 250,000 gallon diesel fuel tanks

j. A highway bridge (Highway 465) that crosses the discharge channel

k. A paint, Oil, and Lubrication (POL) storage building composed of concrete masonry unit walls and concrete roof with membrane roofing

l. A storage building used to house the pumps prior to installation, which would later be repurposed into a storage facility

m. A vehicle garage and associated maintenance and washdown facilities

n. A potable water well (40 gallons per minute) with an associated well building and water treatment facilities

o. An emergency generator and generator building;

p. An architectural plaza area, adjacent to the control building, and an overlook park area

q. Two access roads, one for the control house and another for the maintenance area

Table 4-1. Design Elevations for Previous Design

Description	**Elevation (feet, NGVD 29)**
Project Flood – 2-Year	91.0
Project Flood – 100-Year	100.3
Pump Floor	112.8
Top of Structure (Floodwall)	119.0
Pump On/Off	87.0
Inlet Channel Invert	65.0
Discharge Channel Invert	76.0

246. The previous design included a line of protection across the discharge side of the pump station that consisted of a floodwall at either end of the plant and a floodwall with parapet at the

discharge side of the service bay and substructure monoliths. The protection elevation was 119.0 feet (NGVD 29).

247. Additionally, the previous design included twelve pumps rated at 1,167 cfs for a total plant design capacity of 14,000 cfs. The rated capacity was based on a static (pool-to-pool) head of 3.7 feet. The maximum design static head was 20.0 feet with a capacity of 667 cfs per pump for a total of 8,000 cfs. The pump engines were diesel-fueled engines rated at approximately 2,500 horsepower (hp) each.

248. The pump station monoliths, from the previous design were approximately 89 feet in length and ran perpendicular to the channel. Each monolith was proposed to house three pumps.

249. The intake structure included trash screens and a raking system as well as an access bridge, which allowed vehicles to cross the pump station. The intake structure had a top-of-structure elevation of 107.5 feet (NGVD 29). Additionally, a stoplog system was proposed at the upstream end of the structure and allowed for dewatering.

250. The substructure from the previous design included the formed suction intake for the pumps, a pump bay to house the pumps, discharge piping, and discharge ports. Two access tunnels above the formed suction intake and upstream of the pump bays allowed for access to and inspection of the pumps. The monoliths included slots for intake and discharge gates located upstream of the formed suction intake and downstream of the discharge ports, respectively. The monoliths included a flood wall with parapet on the discharge side, with a protection elevation of 119.0 feet (NGVD 29). The pump floor elevation was 112.8 feet (NGVD 29) and the engines were located on the pump floor in line with the pumps.

251. The pump station superstructure was a reinforced concrete building with brick façade, and was composed of columns and precast concrete panels. The roof was a steel roof deck overlain with rigid insulation and modified bitumen and was supported on trusses. The building included a 40-ton bridge crane with an auxiliary 10-ton hoist that spanned the entire length of the pump station plus service bay.

252. The previous design required a highway bridge located at the intersection of the discharge from the pump station and Mississippi Highway 465. The bridge was designed by USACE but Mississippi Department of Transportation (MDOT) had approval authority of the design. The cost of design and construction of the bridge would be paid for with project funds. The bridge design consisted of two 20-foot lanes on a prestressed concrete girder bridge with 100-foot spans. The total length of the bridge was to be 702 feet.

253. All structures were soil-founded except for the fuel dock, which was elevated on piling, and the highway bridge, which had pile-founded abutments and piers.

UPDATED DESIGN

254. The pump station design has been updated based on new directives and changes since 2007. The following paragraphs describe the major design changes and provide rationale for each change.

255. The location of the pump station was moved from the Steele Bayou pump site, located near the Steele Bayou structure, to the proposed Deer Creek site. This new location will be closer to Highway 61 and to natural gas and electric utilities. Thus, the pump station will no longer require a highway bridge across the discharge channel.

256. The pump engines have been changed from diesel-fueled to natural gas-fueled engines. This change will reduce energy costs and emissions. It will also eliminate the need for diesel fuel infrastructure, including the fuel dock and fuel storage tanks.

257. The service bay and control house structures have been changed from full-depth monoliths to slab-on-grade foundations with grade beams. This change will reduce the overall cost of the structure by reducing the concrete volume and by reducing the total excavation and backfill requirements. The substructure tunnels will be accessed via a reinforced concrete stairwell.

258. The pump station superstructure has been changed from reinforced concrete with brick façade to a prefabricated metal building. This change will reduce the overall cost of the structure.

259. The control house has been reduced to eliminate unnecessary facilities. The conference room, multiple restrooms, and elevator have been removed and the overall size of the facility has been reduced.

260. The potable water well and treatment systems have been removed. It is assumed that potable water will be provided by Valley Park Water District.

261. As previously stated, the highway bridge across the discharge channel will no longer be required. Instead, a precast concrete girder bridge, with precast deck sections, will be constructed across the intake channel along the levee centerline.

262. The storage building and vehicle garage have been removed. It is assumed that on-site pump storage will not be required because the project will be solicited under one contract and pumps will be installed upon delivery.

263. The standby emergency generator building has been removed. The generator will be housed in an enclosure near the service bay.

264. The pump station will be heated by natural gas unit heaters, eliminating the hydronic heating system, including boilers, pumps, heaters, and piping. Engines will be cooled by remote radiators, one each per engine, eliminating the centralized raw water cooling system. The bridge crane will be used to provide vertical movement of equipment to the tunnels, eliminating the need for an elevator. The potable water system (exterior hose bibbs and pressure washer) will be used for exterior building maintenance, which eliminates the “fire hose” type wash down system, including the water storage tank.

265. The architectural plaza area and overlook park area have been removed.

266. Supplemental low flow groundwater wells will be installed in 34 strategic locations throughout the Mississippi Delta as an environmental feature to the project. Future engineering

studies will evaluate the geologic and hydro-geologic conditions of each of the well field sites, and the wells will be pumped to supplement annual low flow conditions. It is estimated that each well site will impact approximately 0.25 to 1.25 acres of land.

267. Access to the site will be over the Yazoo Backwater Levee. Two embankments will connect the pump station to the levee, one on each side of the intake channel. The Yazoo Backwater Levee will be enlarged and paved to facilitate access to the pump station.

ASSUMPTIONS

268. The following assumptions were made in order to produce the required quantities and plans without the detailed site investigation needed to develop precise calculations. These assumptions will be validated during the design phase.

a. The pump station will be constructed under a single contract.

(1). The original design included several contracts and called for procuring the pumps prior to the completion of the pump station structure. The pumps were to be stored on site in a building specifically designed for storage, which will later be repurposed into a maintenance or storage facility. By assuming a single contract, the designers can remove the storage building and assume that the pumps will be delivered to the site after construction of the pump station structure.

b. The new pump station will be designed to the hydraulic criteria of the previous design, and the major structures of the pump station will be largely unchanged from the previous design.

(1). This assumption allows the designers to quickly determine quantities based on the previous design. At this stage of the planning process, detailed site investigations, required to develop detailed calculations, were not possible. The anticipated changes to the new design will include updated pump curves, updated structural elevations based on new hydraulic modeling, and new soils data from borings. These new criteria are not anticipated to significantly affect the cost of the structure.

c. Natural gas supply will be available from the Kinder-Morgan pipeline adjacent to the new site.

(1). This assumption is made because Kinder-Morgan has indicated that they plan to abandon the supply line adjacent to the site. Additionally, they have indicated that they will postpone their decision as of April 2020.

d. The borrow area identified for the previous design will be used for the new design.

(1). A borrow area residing north of and adjacent to the Steele Bayou structure was identified to provide fill material for the previous design. It is assumed that using this borrow material will be the most cost efficient method of procuring fill for the new site location. The material will be hauled along Highway 465 to Highway 61, before being transported along the levee to the pump station.

e. The new pump station will be accessed via the Yazoo Backwater Levee, which will need to be enlarged.

(1). The new location of the pump station is between the Yazoo River and the Yazoo Diversion Canal along the Yazoo Backwater Levee. Enlarging the existing levee and providing surfacing is assumed to produce cost savings versus constructing a new roadway to access the pump station.

f. Electric power will be provided by the Yazoo Valley Electric Power Association (YVEPA), and a new substation will not be required. Water service will be provided by Valley Park Water District, eliminating the need for installation of a USACE owned and operated new water well and water tank. Waste water will be treated on site and disposed of in the intake canal.

(1). Based on preliminary estimates of the required power for the site, YVEPA has indicated that a new substation will not be required and a new distribution line can be installed from existing lines near the pump station. Valley Park has indicated that they have limited capacity for potable water and Valley Park may add an additional well and water tank nearby to provide the required water to the pump station. It is assumed that fire suppression at the new pump station will use stored water.

QUANTITY CALCULATIONS

269. Quantities were generally taken from Microstation models. Models from the previous design were used and modified for the new location at Deer Creek.

270. Earthwork quantities are based on Microstation Inroads triangle volume reports. The ground surface was modeled from LiDAR data and surfaces representing the earthwork features were developed. The dimensions for excavation and fill surfaces are based on updates to the previous design. Estimates were received from the three servicing utility companies for potable water, natural gas, and electrical power connections.

271. Structural quantities are based on three dimensional Microstation models, which were modified from the previous design. New models were developed for the prefabricated metal building, levee bridge, slab on grade foundations, stairwells, and floodwalls. All other structures were taken from the previous design unmodified.

272. Mechanical quantities are based on three dimensional Microstation models, printed drawings of the previous design, and quoted estimates from manufacturers and local distributers.

273. Electrical quantities are based on Microstation models and printed drawings of the previous design. Quantities were taken from printed drawings and miscellaneous tables produced during the previous design effort.

274. Architectural quantities are based on three dimensional modeling using AutoDesk Revit 2020. The architectural features were modified from the previous design to meet current building and DOD/UFC code and energy requirements. The modified design and quantities assume that no high-sustainability elements will be required, but will achieve 30 percent below

current ASHRAE requirements. It is also assumed hurricane-related or impact-related items will not be required.

PROPOSED PLAN DRAWINGS

275. Figure 4-1 shows the locations of the pump station, the supplemental low flow groundwater wells, and the borrow area for the proposed plan. Additionally, detailed engineering drawings for the proposed plan can be found in Attachment A.

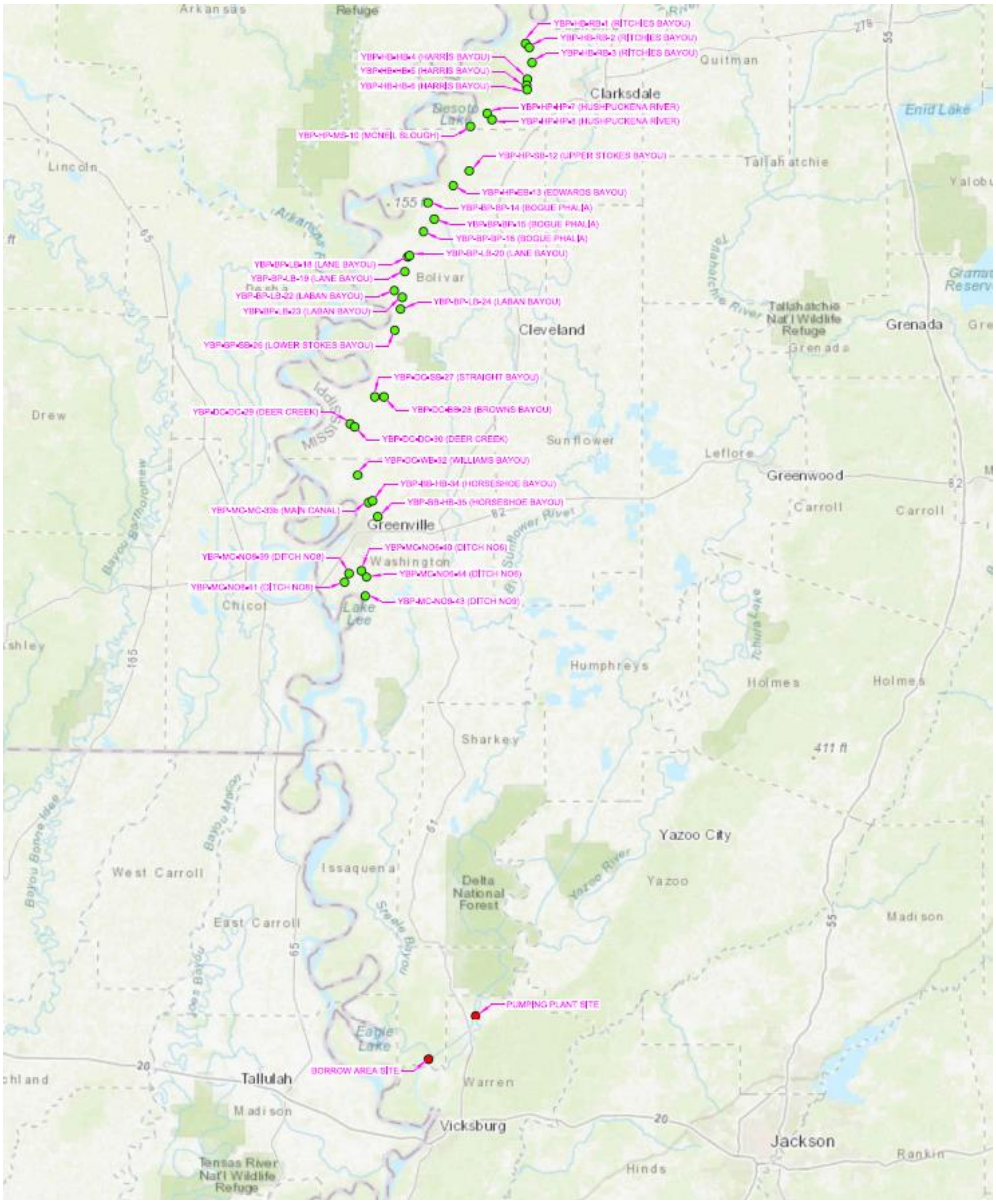

Figure 4-1. The locations for the pump station, supplemental low flow groundwater wells, and borrow area for the proposed plan.

276. Site location maps, aerial photographs, and plan and profile drawings are provided for the 34 supplemental low flow groundwater wells in Attachment B.

SECTION 5 - LITERATURE CITED

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Attachment A

US Army Corps
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Vicksburg District

FC/MR&T

YAZOO RIVER BASIN

WARREN COUNTY, MISSISSIPPI

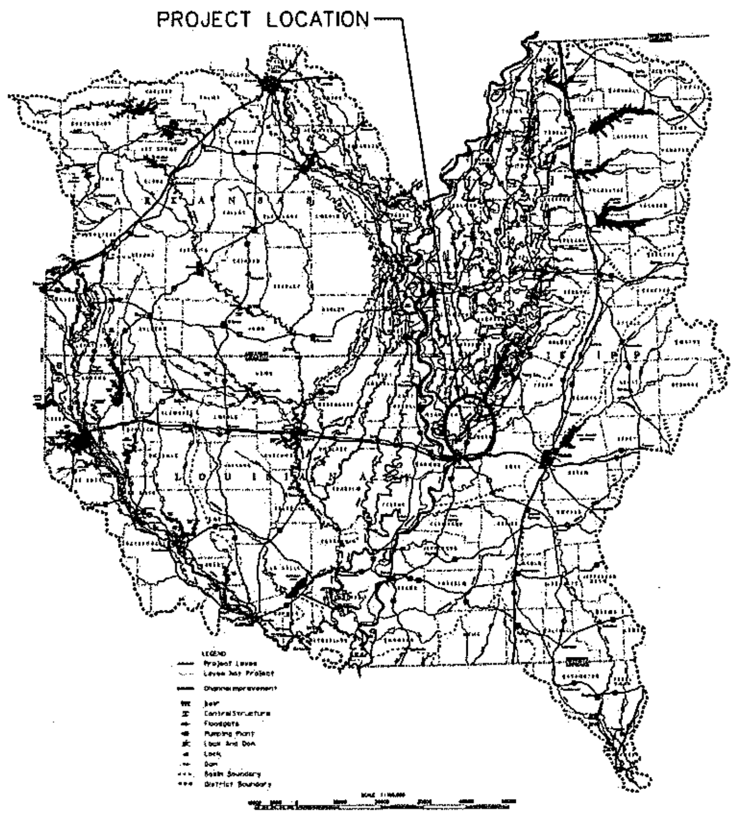
YAZOO BACKWATER AREA

YAZOO BACKWATER PUMPING PLANT

Solicitation: W912EE-XX-X-XXXX
Contract: W912EE-XX-X-XXXX
September 2020

10% NOT FOR CONSTRUCTION

SHEET IDENTIFICATION
G-001



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C-103	STRUCTURE FILL SITE PLAN
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M-502	COMPRESSED AIR SCHEMATIC
M-503	WASTE ENGINE OIL SCHEMATIC
M-504	NATURAL GAS PIPING SCHEMATIC
M-505	POTABLE WATER SYSTEM SCHEMATIC
M-506	UNWATERING SYSTEM SCHEMATIC
M-507	FIRE WATER SYSTEM SCHEMATIC

U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI

E & C DIVISION
DESIGN BRANCH

WARREN COUNTY, MISSISSIPPI
YAZOO BACKWATER AREA
YAZOO BACKWATER PUMPING PLANT

PROJECT LOCATION MAP AND
INDEX TO DRAWINGS

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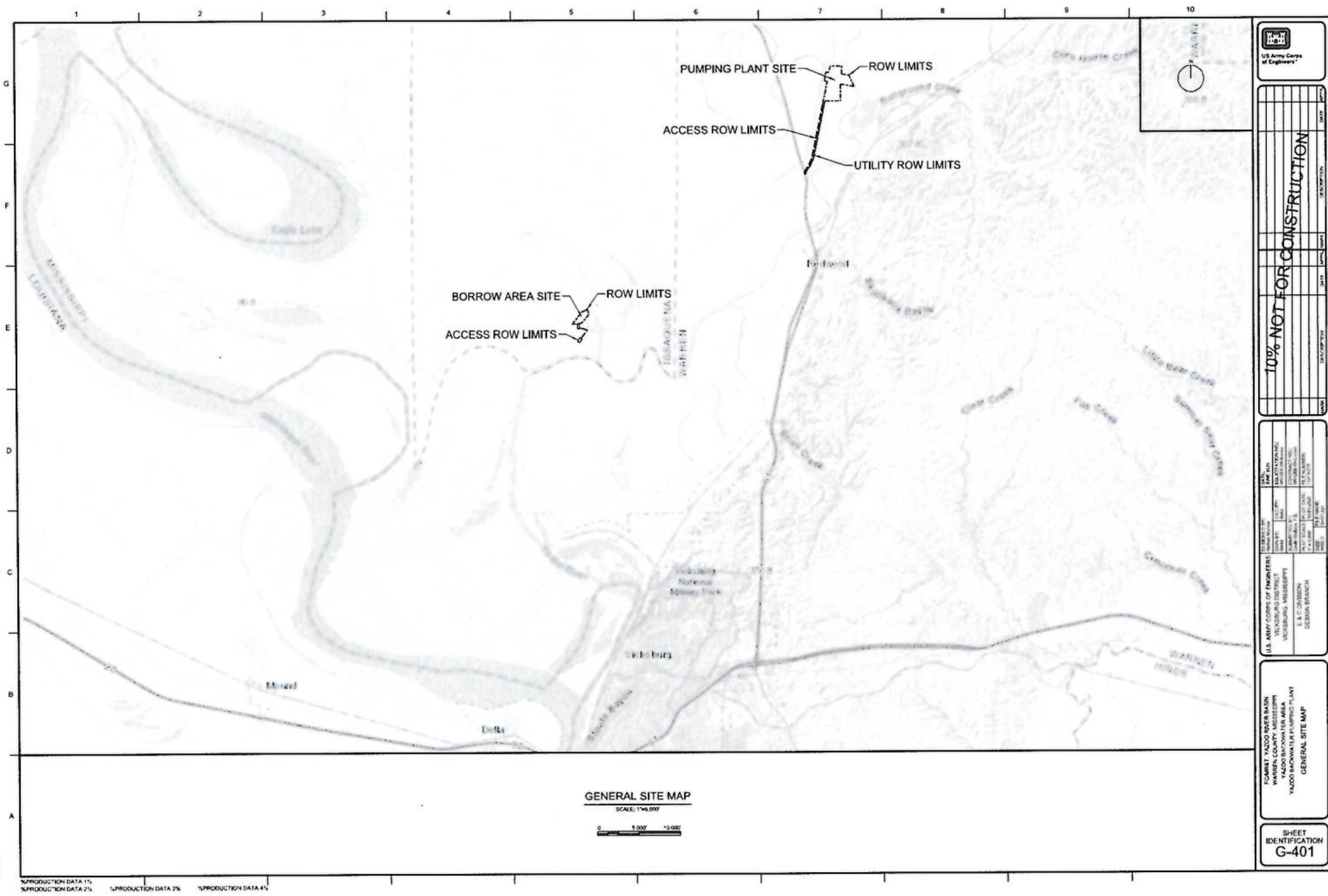
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UTILITY ROW LIMITS
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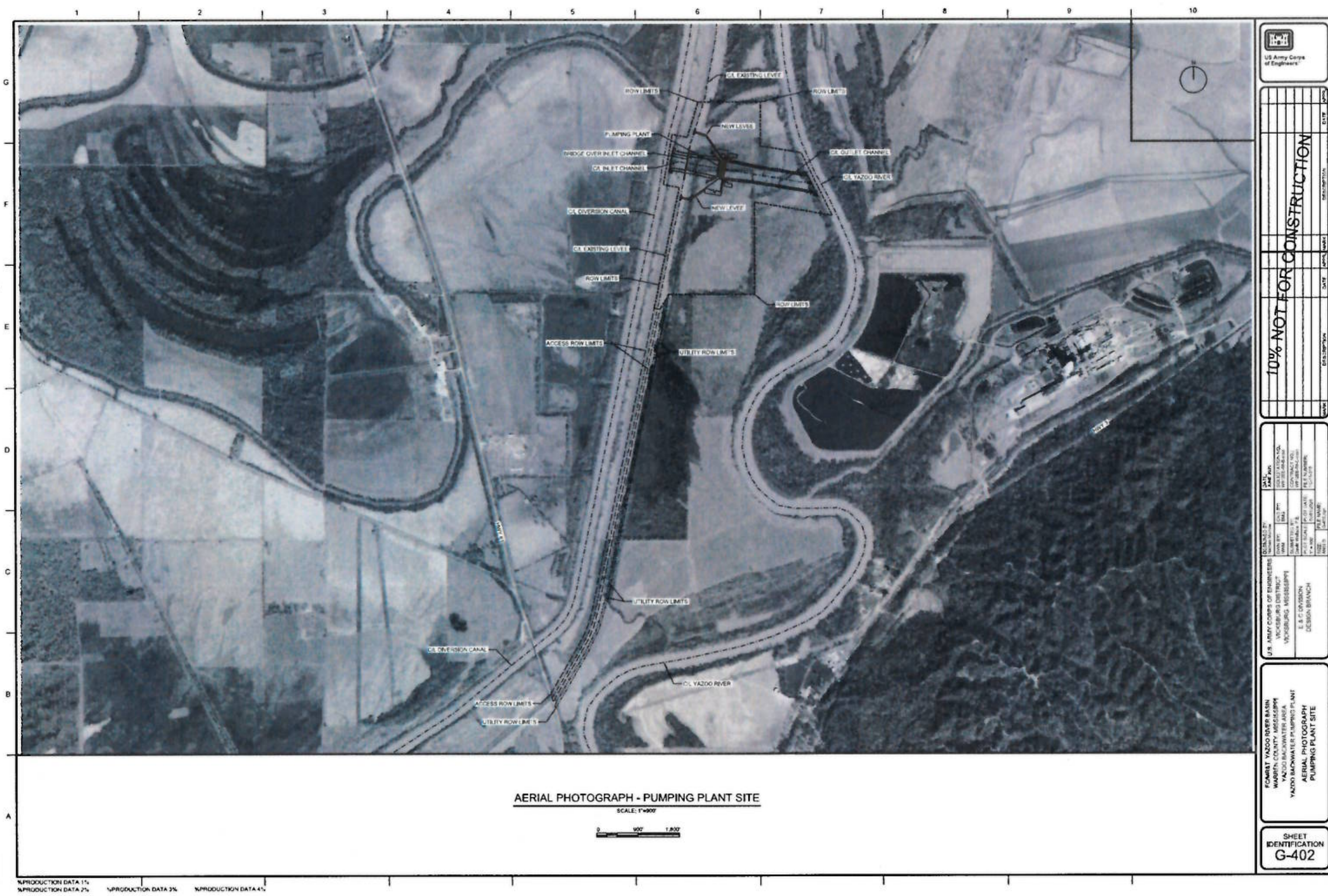
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C/L INLET CHANNEL
NEW LEVEE
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C/L YAZOO RIVER
C/L DIVERSION CANAL
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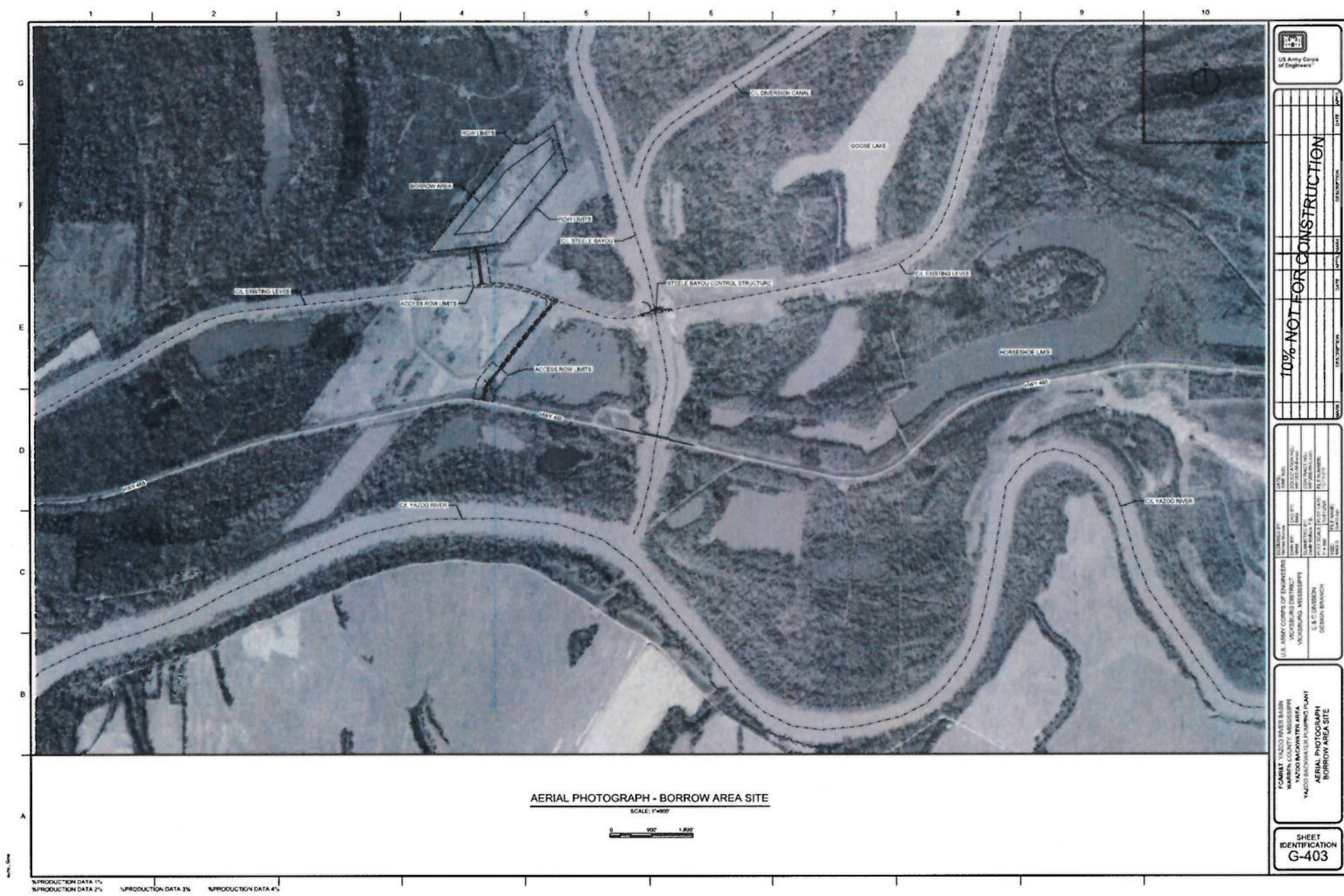
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DESIGN BRANCH
YAZOO RIVER BASIN
WARREN COUNTY, MISSISSIPPI
YAZOO BACKWATER AREA
YAZOO BACKWATER PUMPING PLANT
AERIAL PHOTOGRAPH
BORROW AREA SITE
SHEET IDENTIFICATION
G-403
AERIAL PHOTOGRAPH - BORROW AREA SITE
ROW LIMITS
BORROW AREA
C/L DIVERSION CANAL
GOOSE LAKE
C/L STEELE BAYOU
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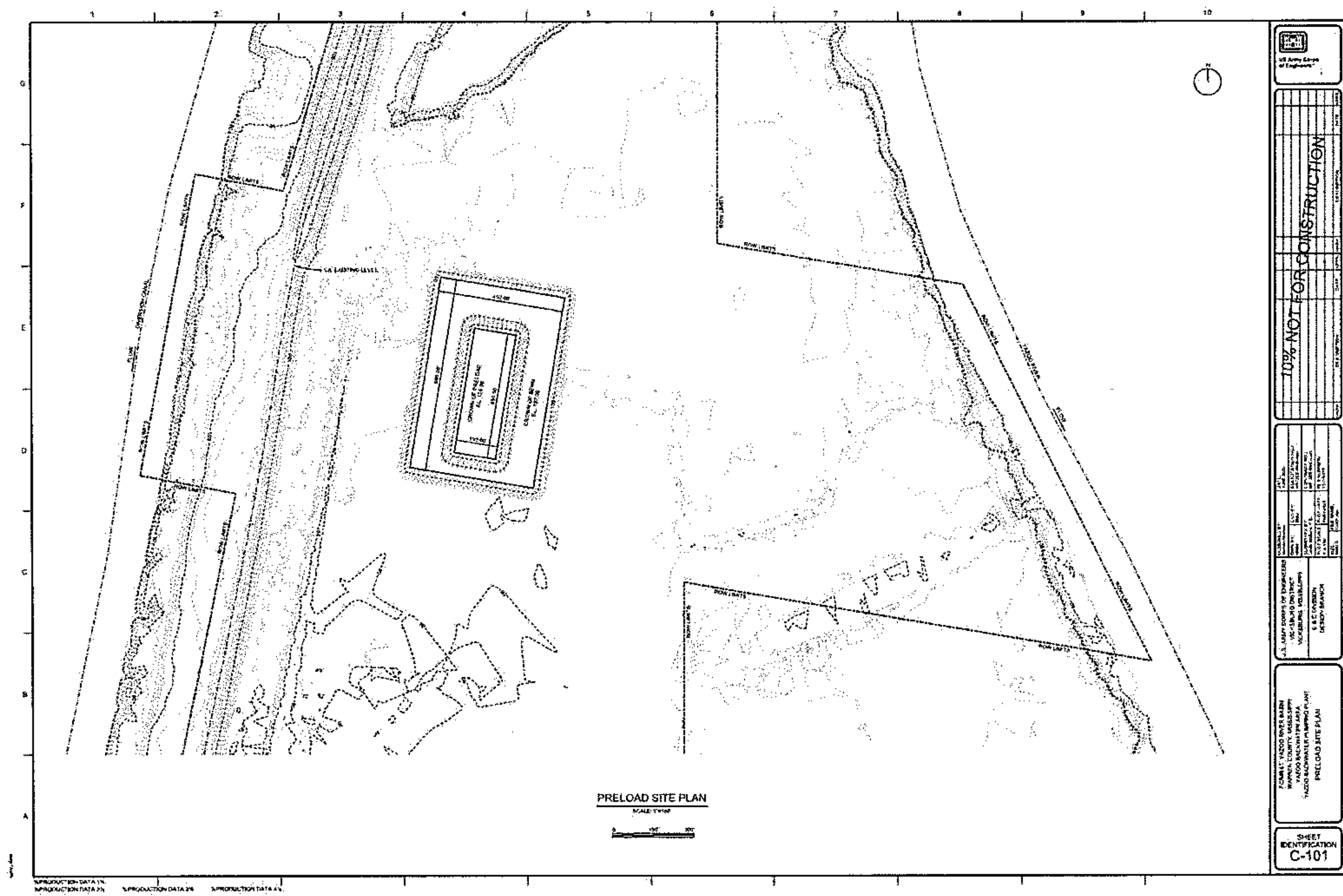
C/L YAZOO RIVER



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VICKSBURG, MISSISSIPPI
G & E DIVISION
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YAZOO BACKWATER PUMPING PLANT
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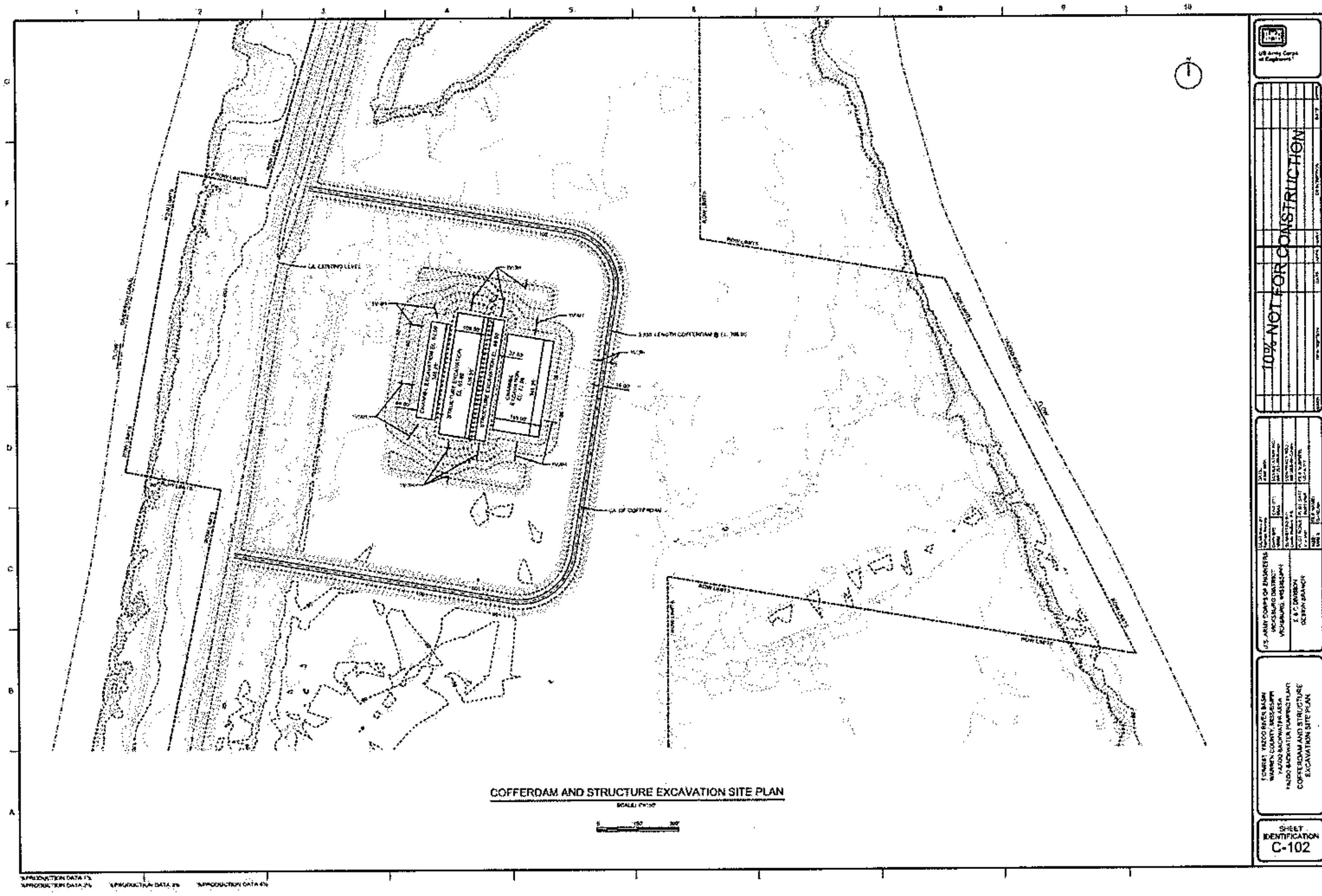
PRELOAD SITE PLAN



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VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
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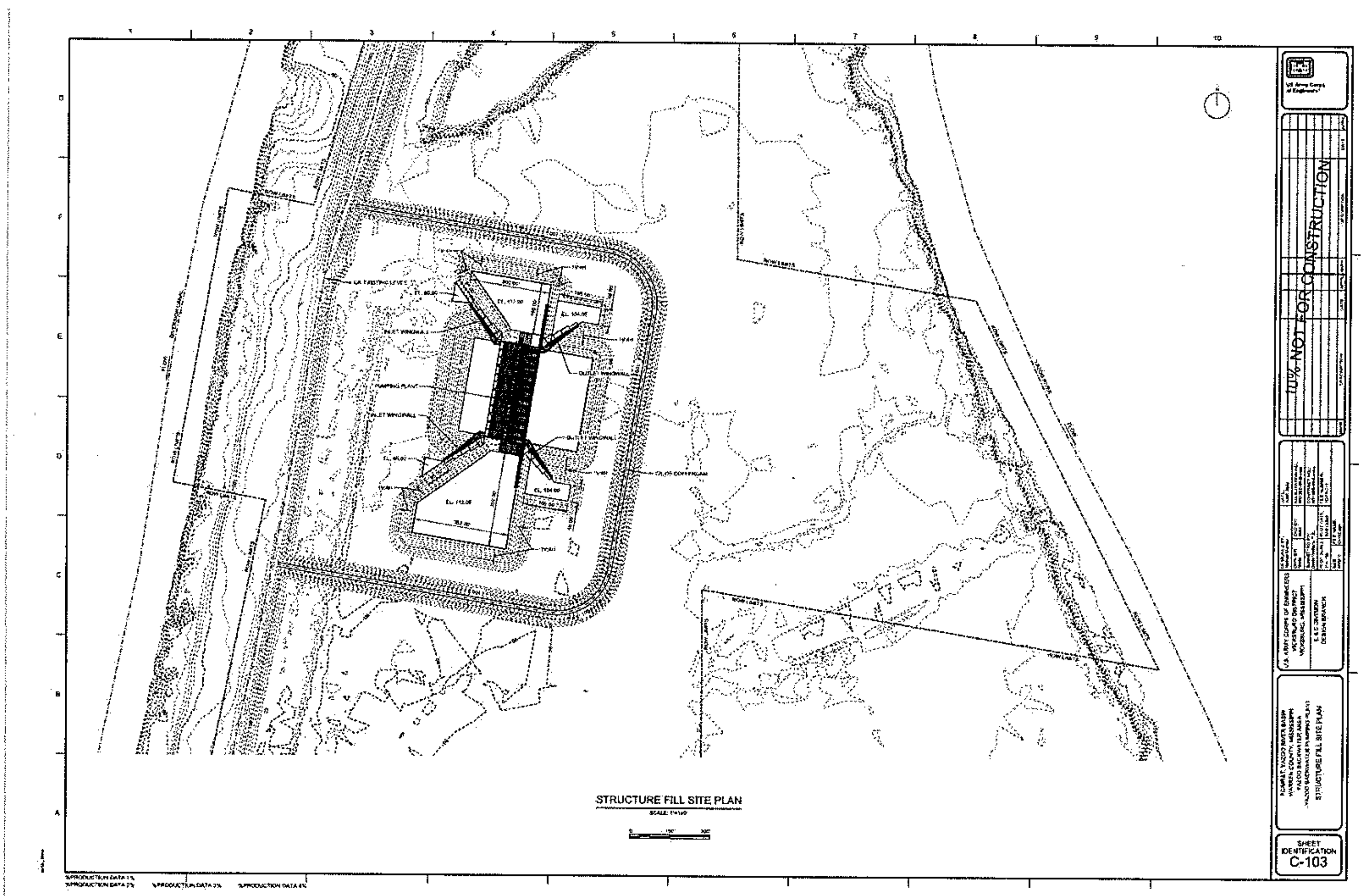

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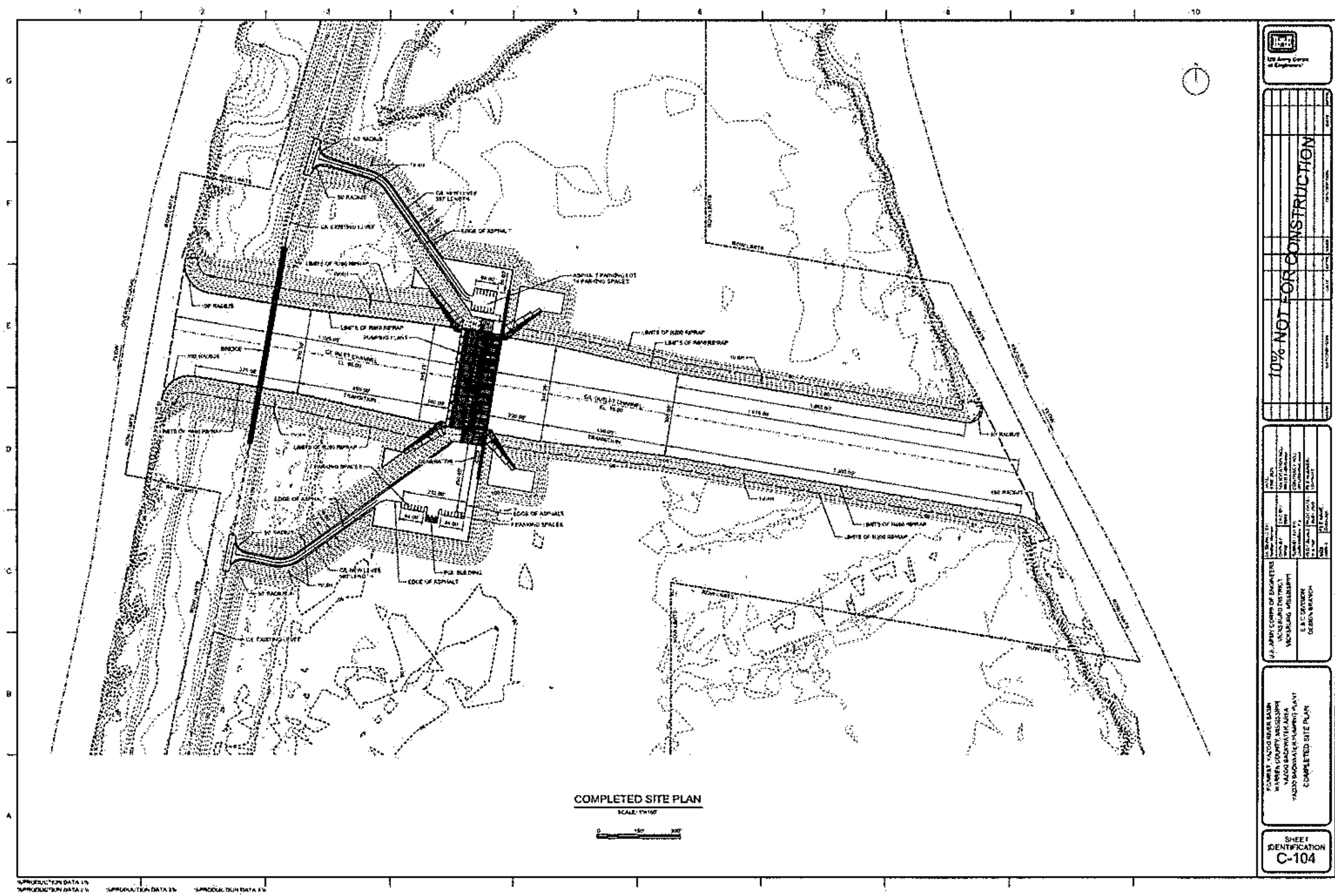

COMPLETED SITE PLAN
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U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
E & C DIVISION
DESIGN BRANCH
YAZOO BACKWATER AREA
COMPLETED SITE PLAN
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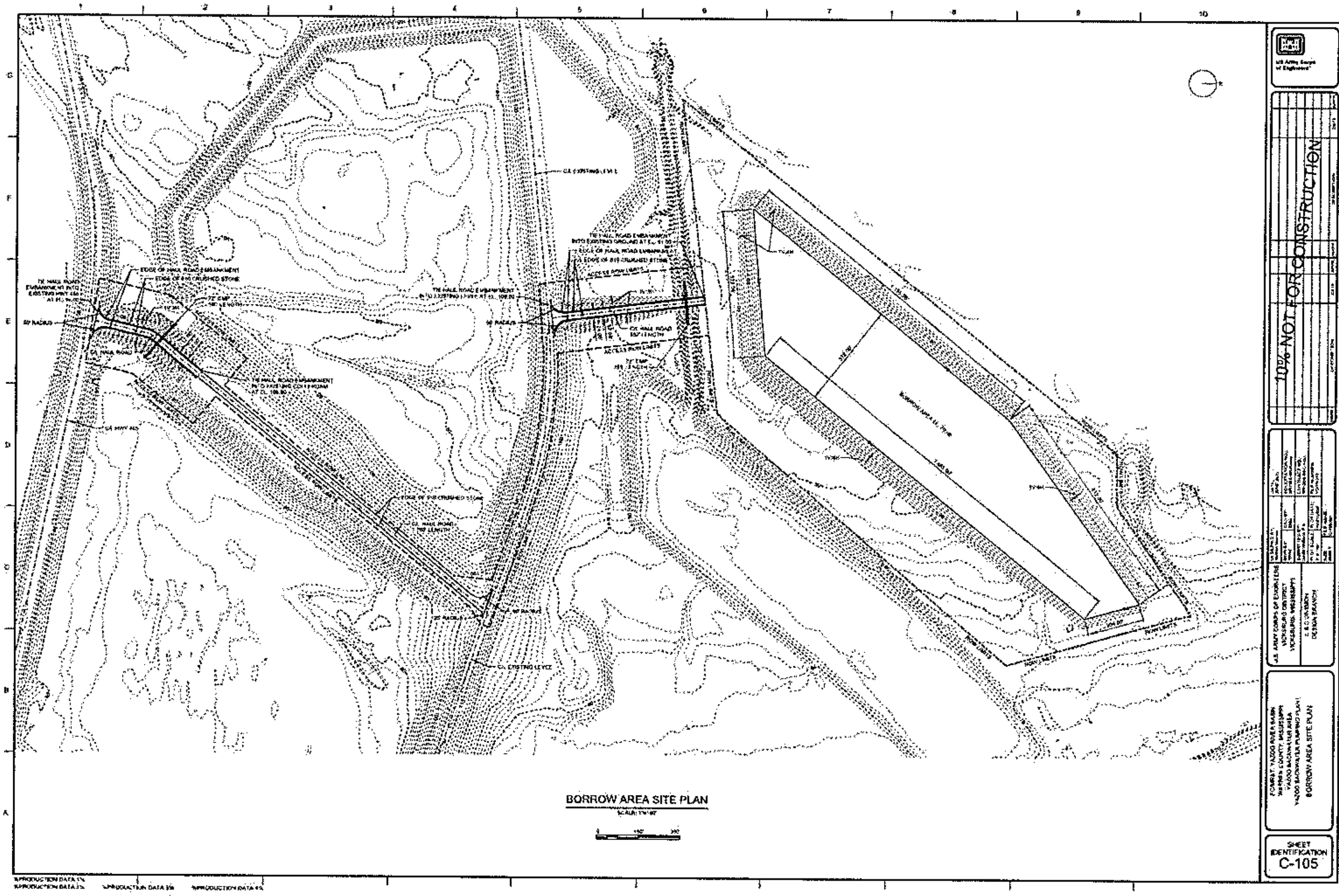
C-104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U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
YAZOO BACKWATER PUMPING PLANT
BORROW AREA SITE PLAN
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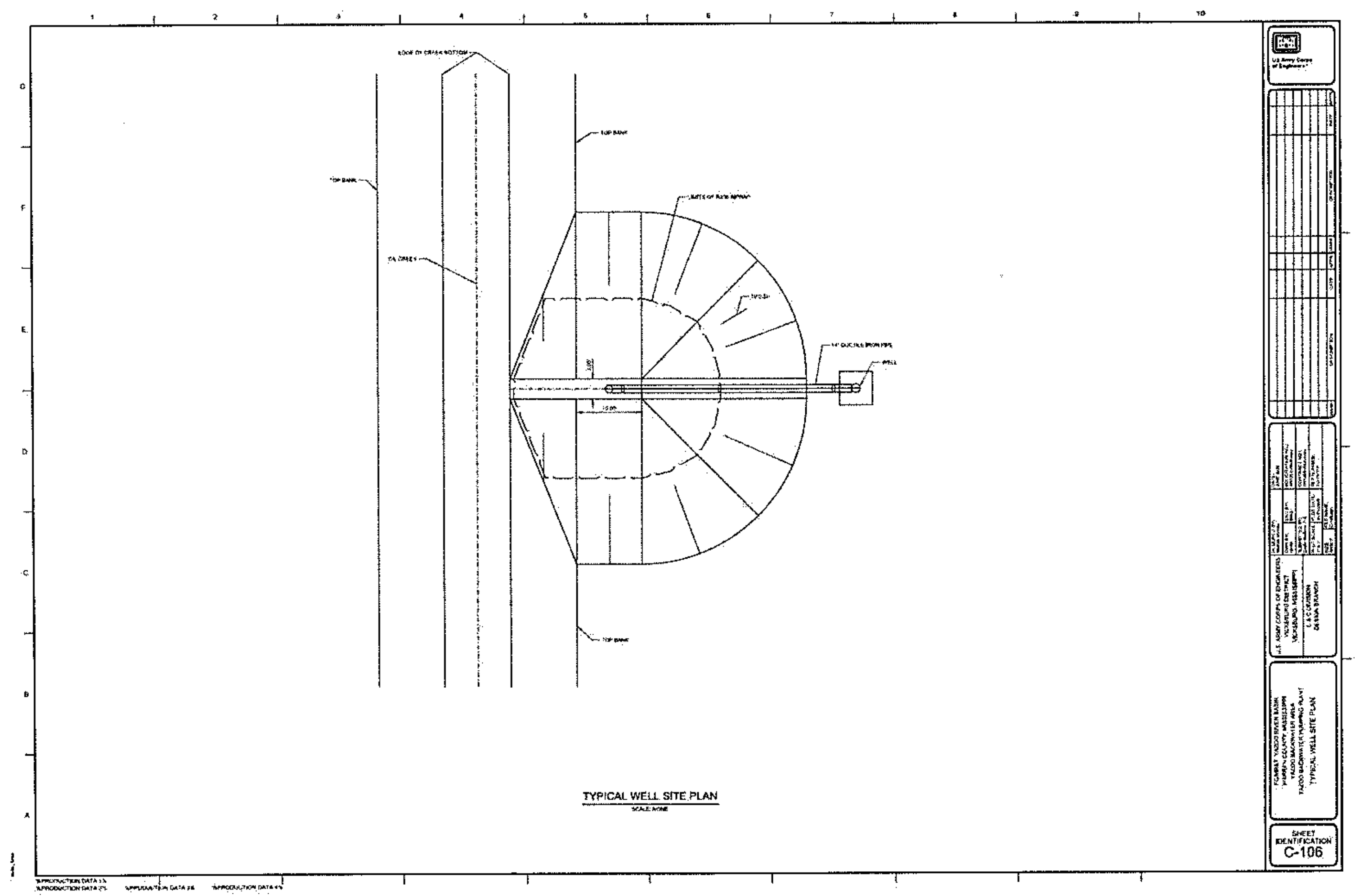
C-105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TOP BANK
TOP BANK
LIMITS OF RIPRAP
CL CREEK
14" DUCTILE IRON PIPE
WELL
TOP BANK
TYPICAL WELL SITE PLAN
SCALE: NONE
US Army Corps of Engineers
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
E & C DIVISION
DESIGN BRANCH
YAZOO BACKWATER AREA
YAZOO BACKWATER PUMPING PLANT
TYPICAL WELL SITE PLAN
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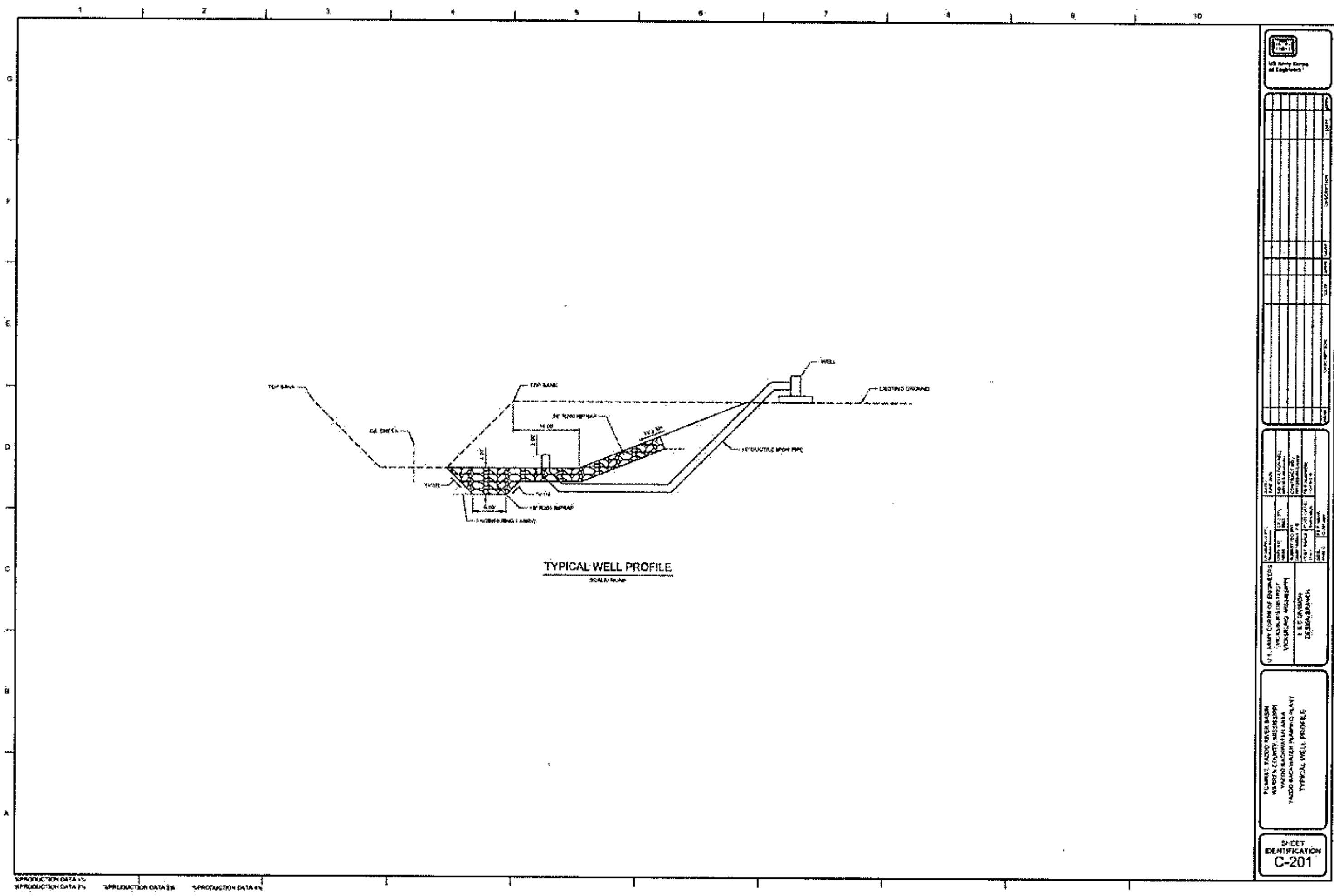
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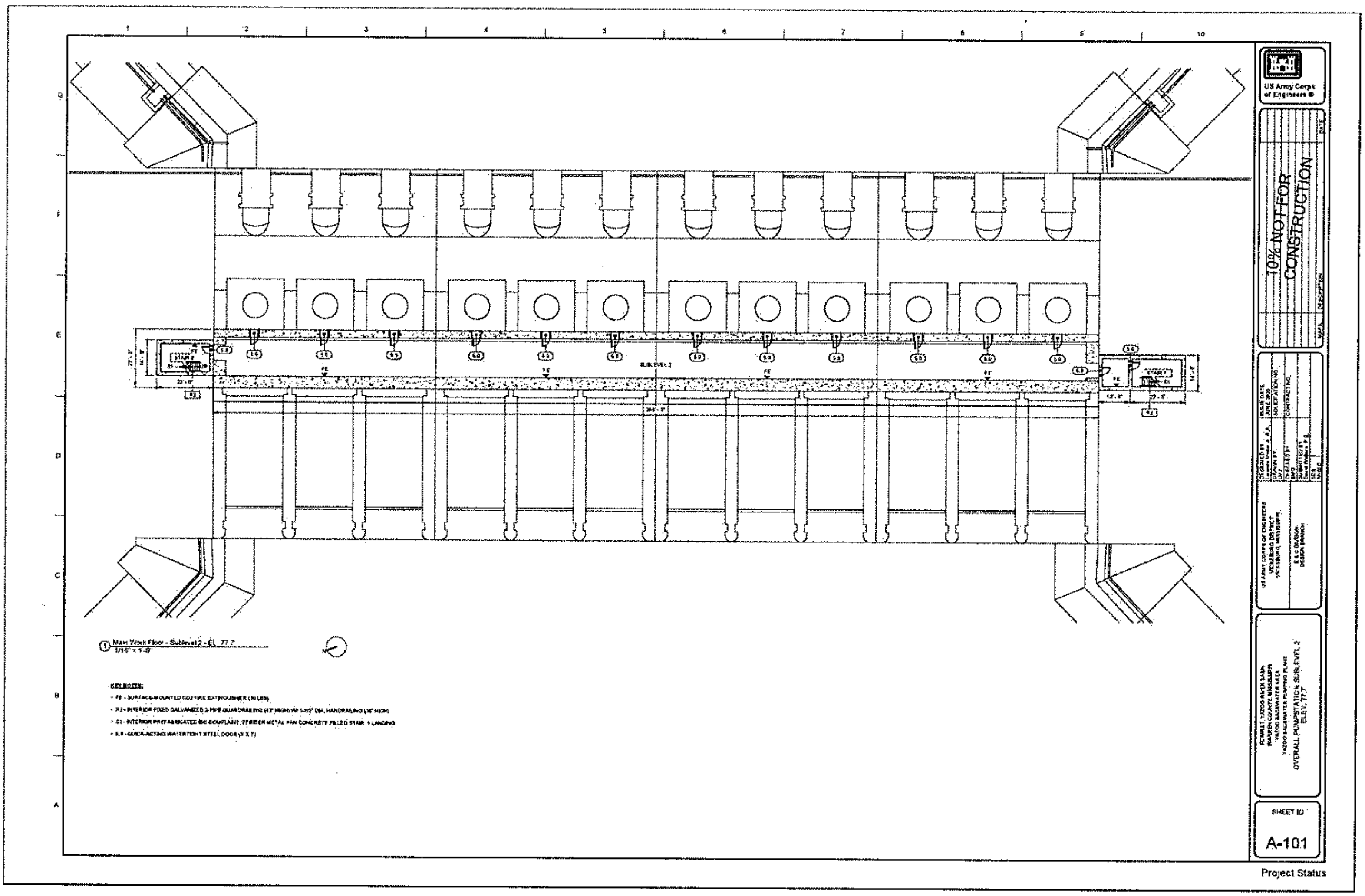

US Army Corps of Engineers ®
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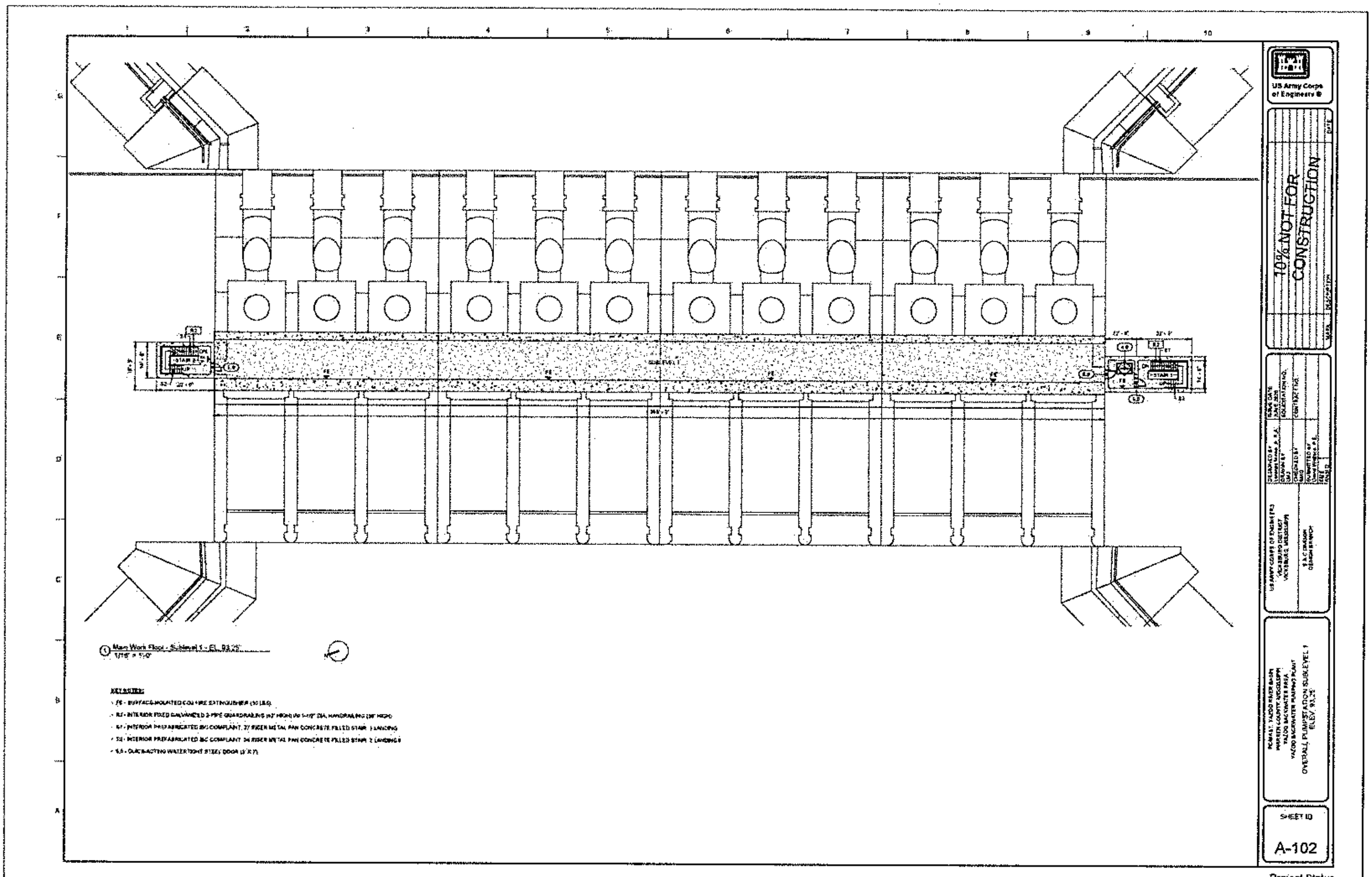
A-101
Project Status



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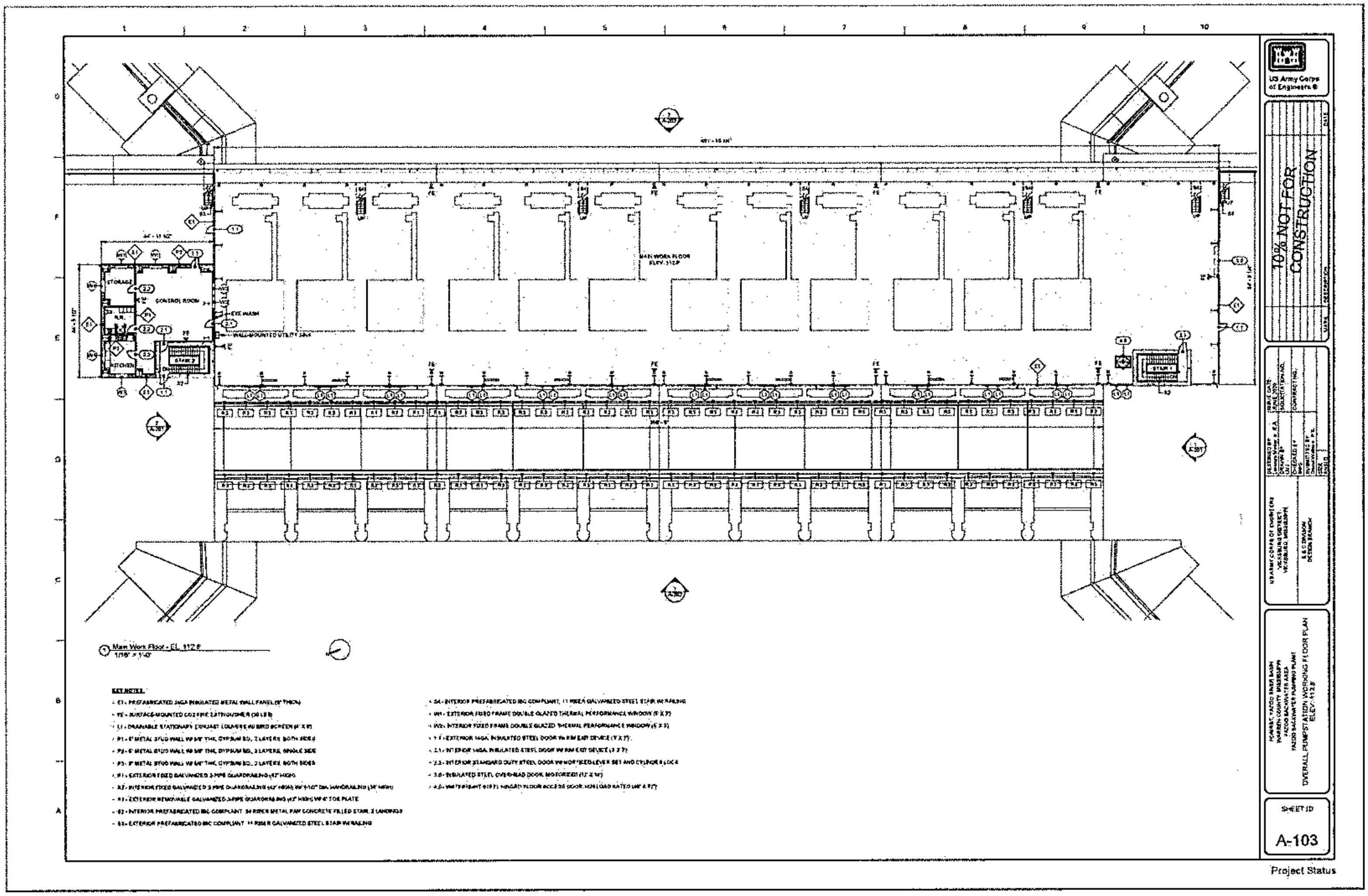

US Army Corps of Engineers
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OVERALL PUMPSTATION WORKING FLOOR PLAN ELEV. 112.8'
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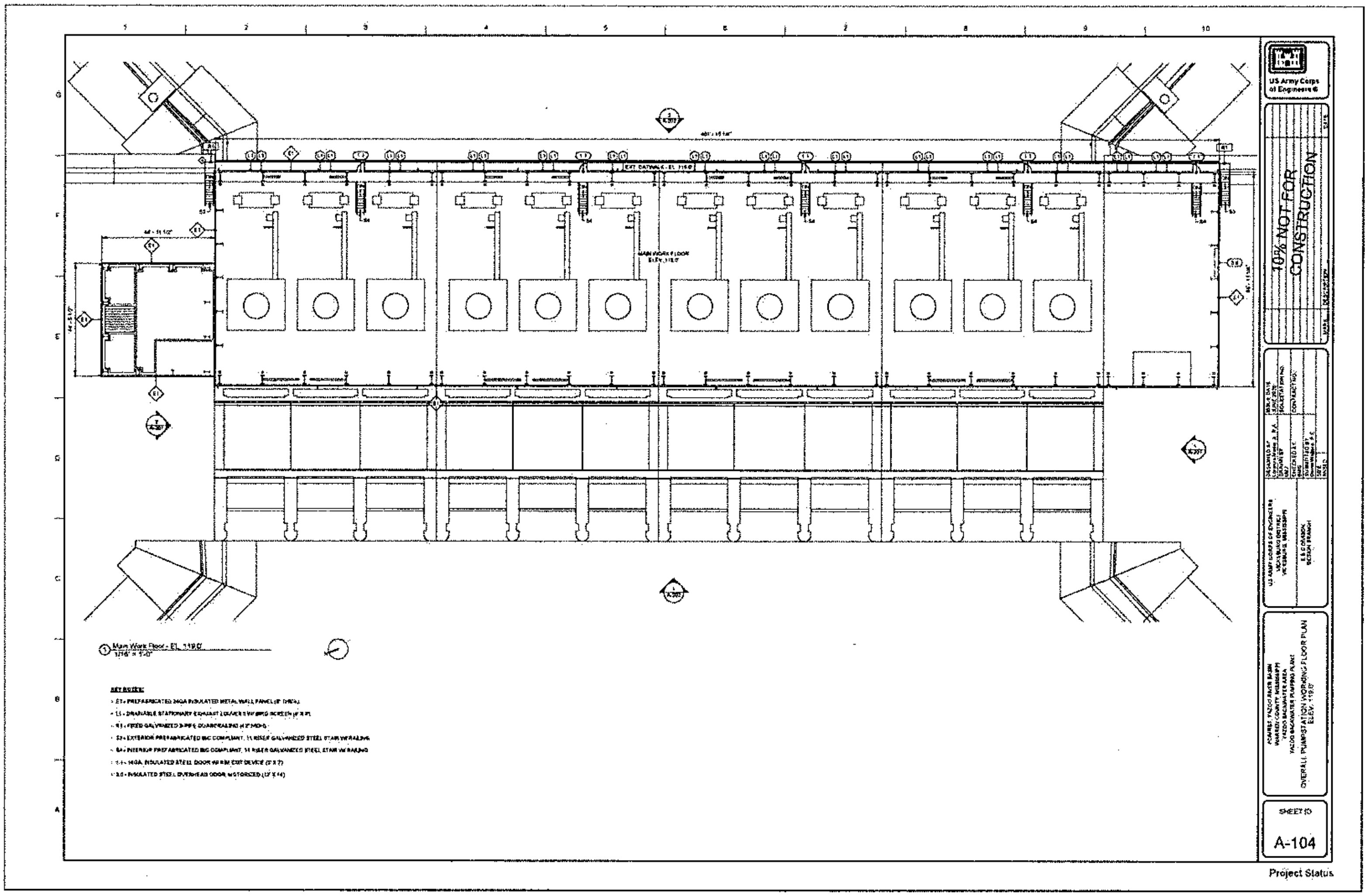
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US ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
YAZOO BACKWATER AREA
YAZOO BACKWATER PUMPING PLANT
OVERALL PUMPSTATION WORKING FLOOR PLAN
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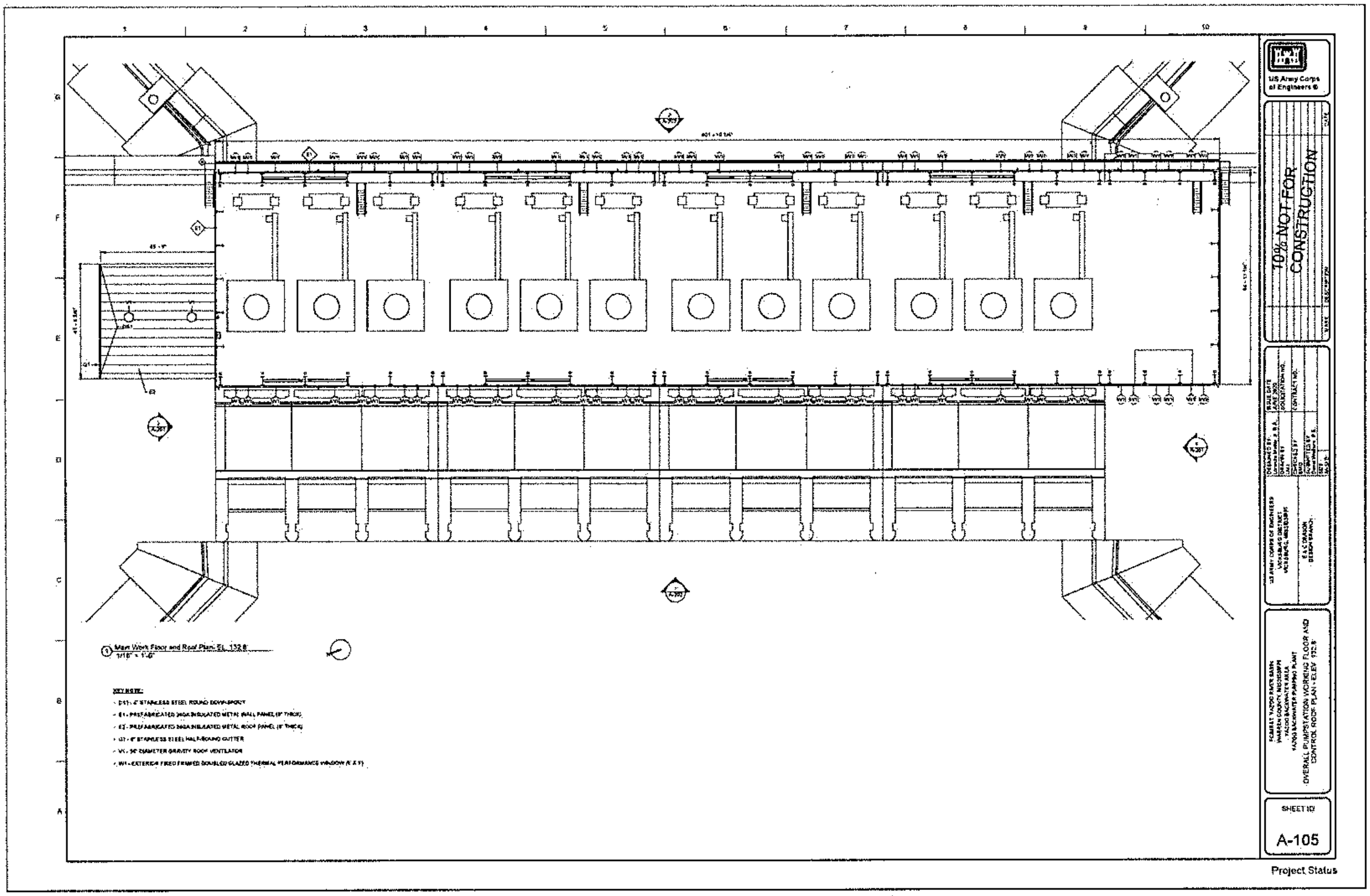
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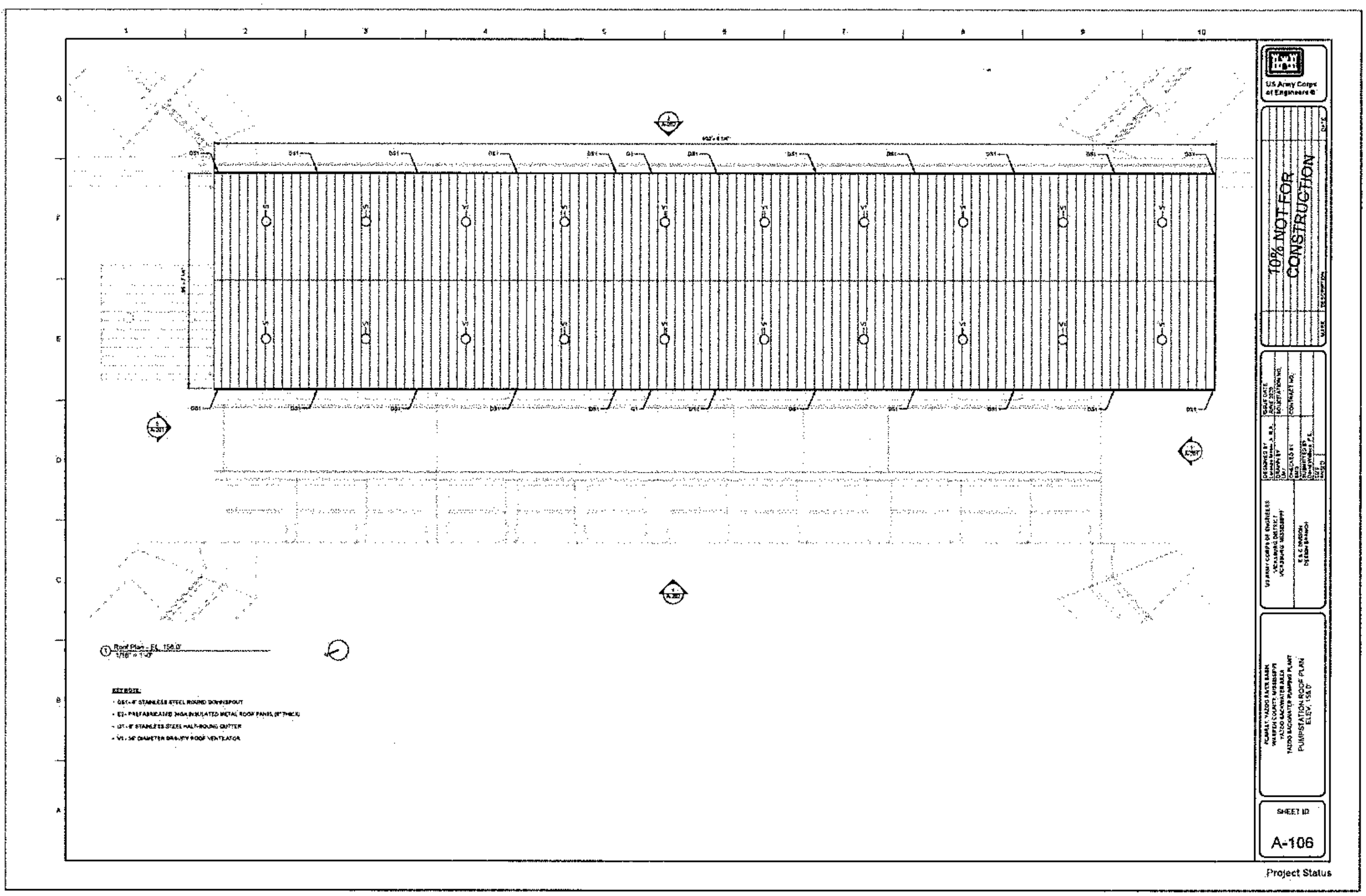
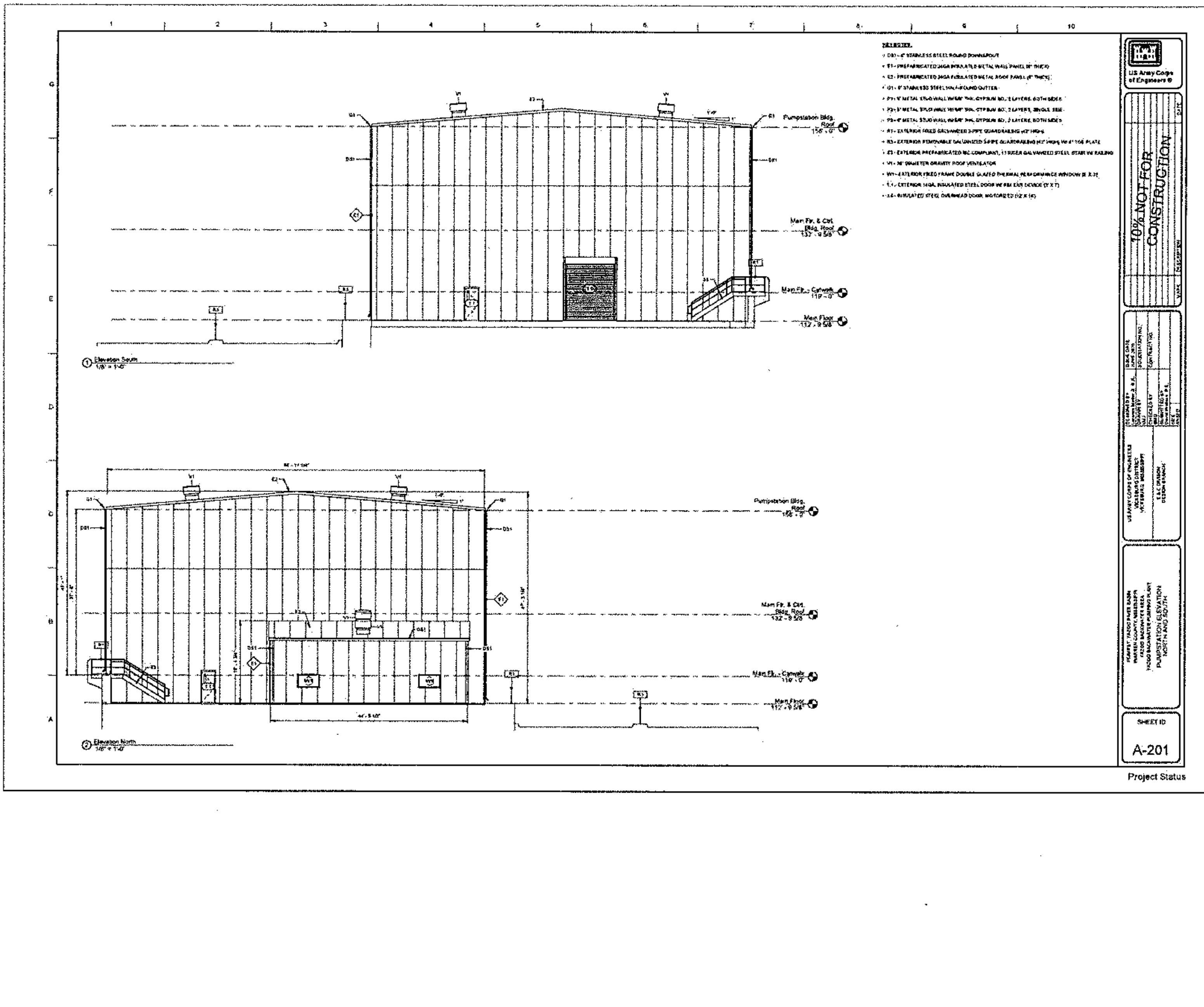
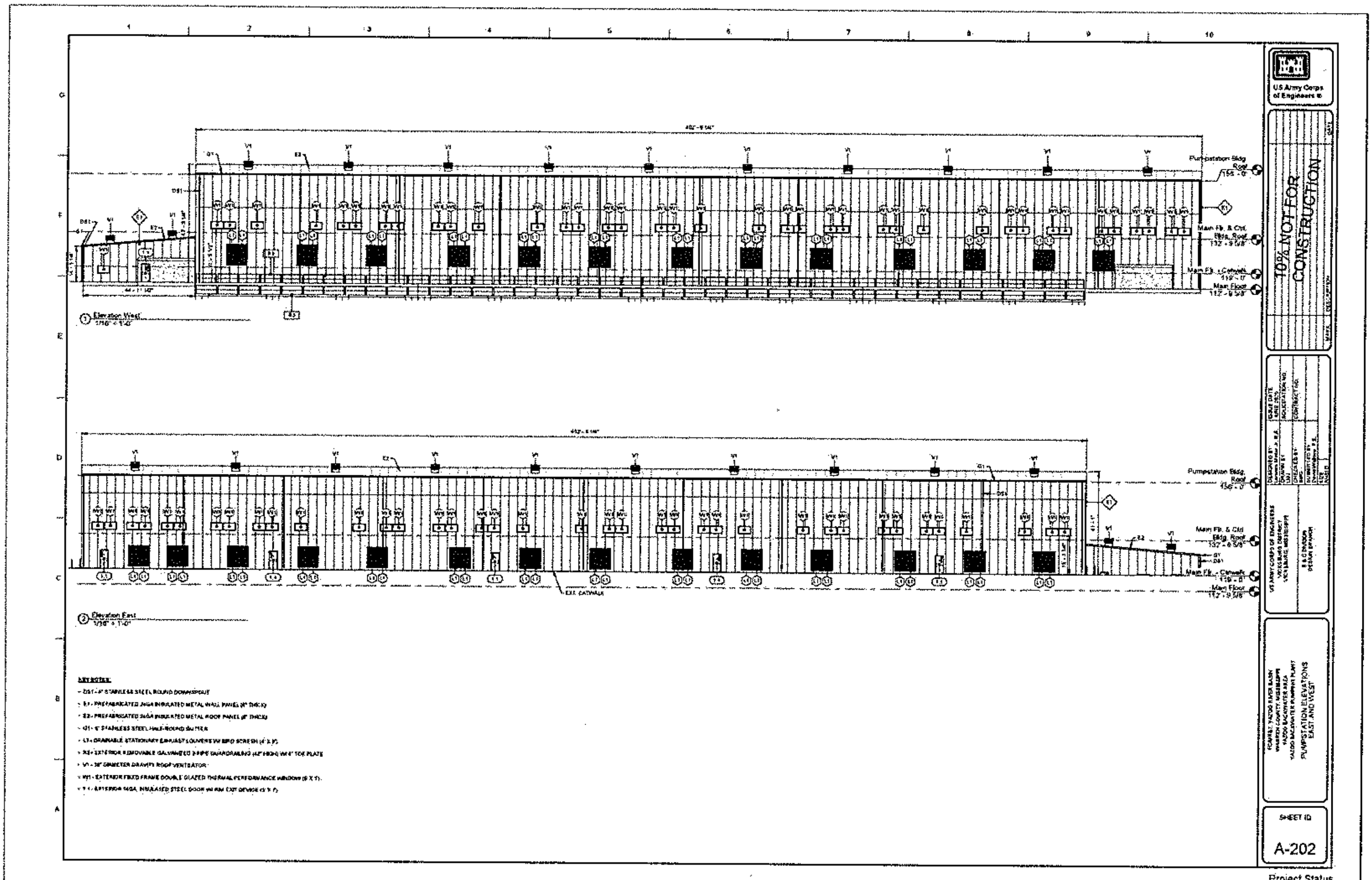
US ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
E & C DIVISION
DESIGN BRANCH
OVERALL PUMPSTATION WORKING FLOOR AND CONTROL ROOF PLAN - ELEV. 132.8'
SHEET ID
A-105
Project Status

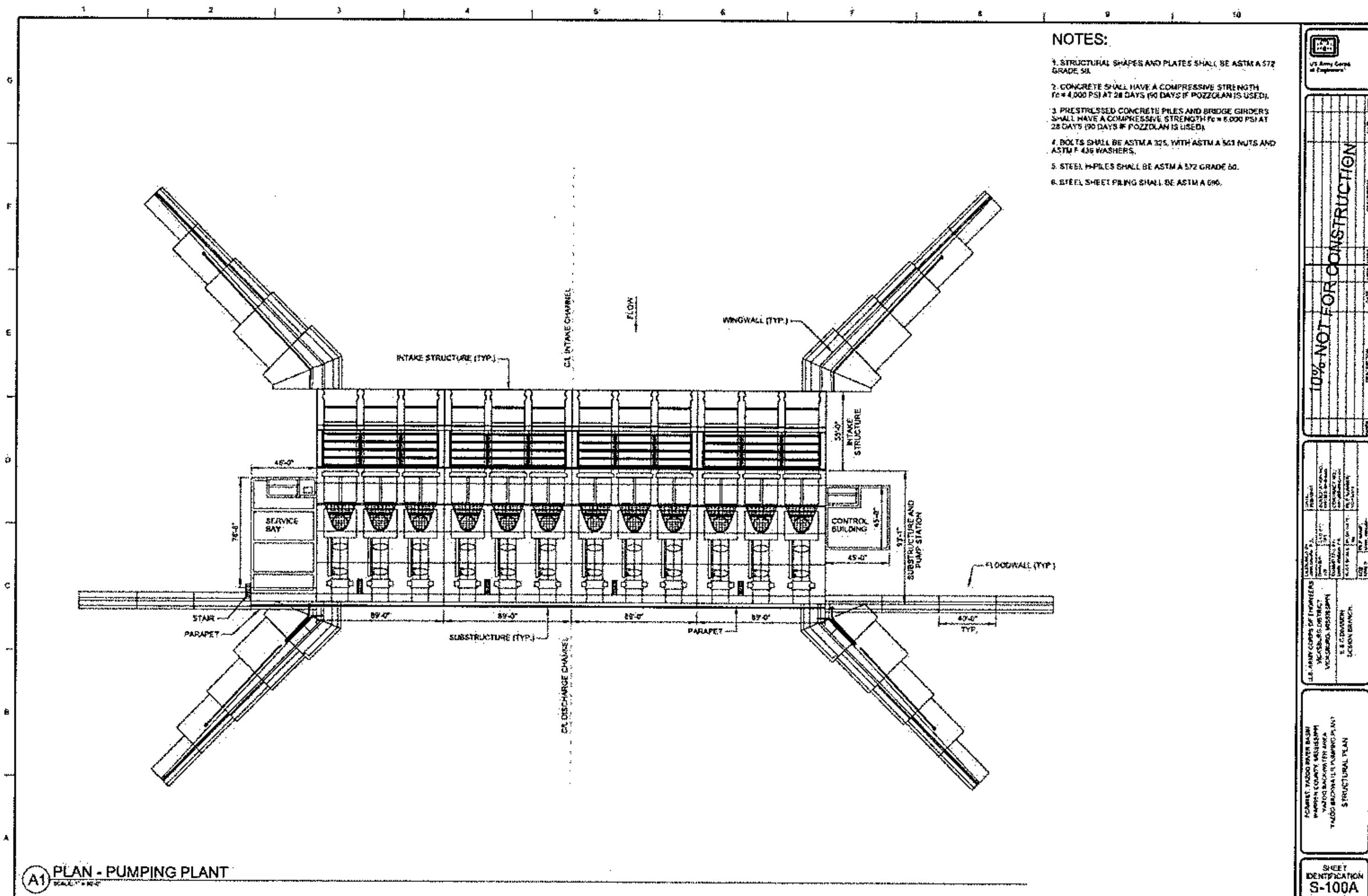


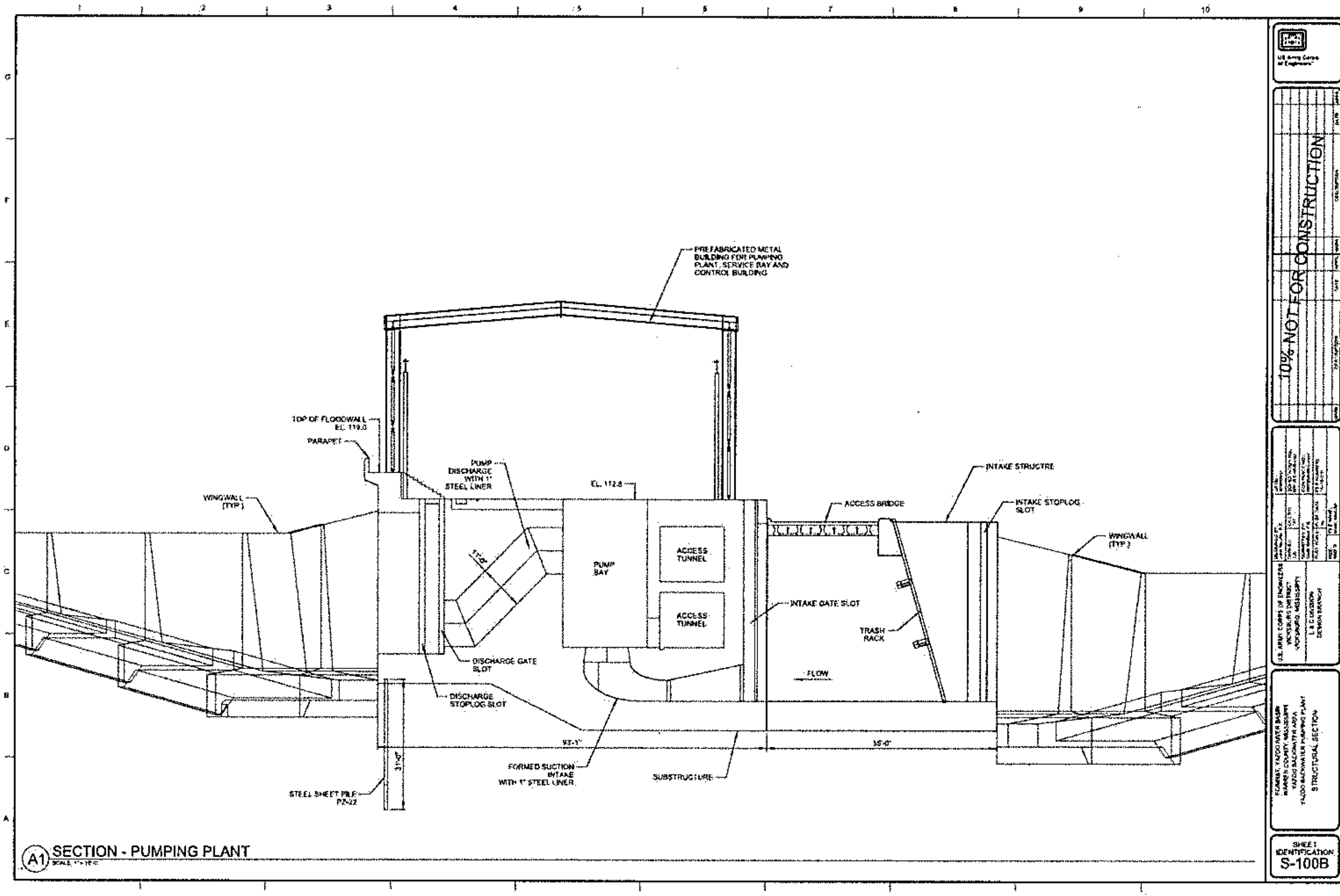
10% NOT FOR CONSTRUCTION
US Army Corps of Engineers
PUMPSTATION ROOF PLAN
SHEET ID
A-106
Project Status


Pumpstation Bldg. Roof 156' - 0"
Main Flr. & Ctrl. Bldg. Roof 132' - 9 5/8"
Main Flr. - Catwalk 119' - 0"
Main Floor 112' - 9 5/8"
Elevation South
Elevation North
US Army Corps of Engineers
10% NOT FOR CONSTRUCTION
US ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
PUMPSTATION ELEVATION
NORTH AND SOUTH
SHEET ID
A-201
Project Status

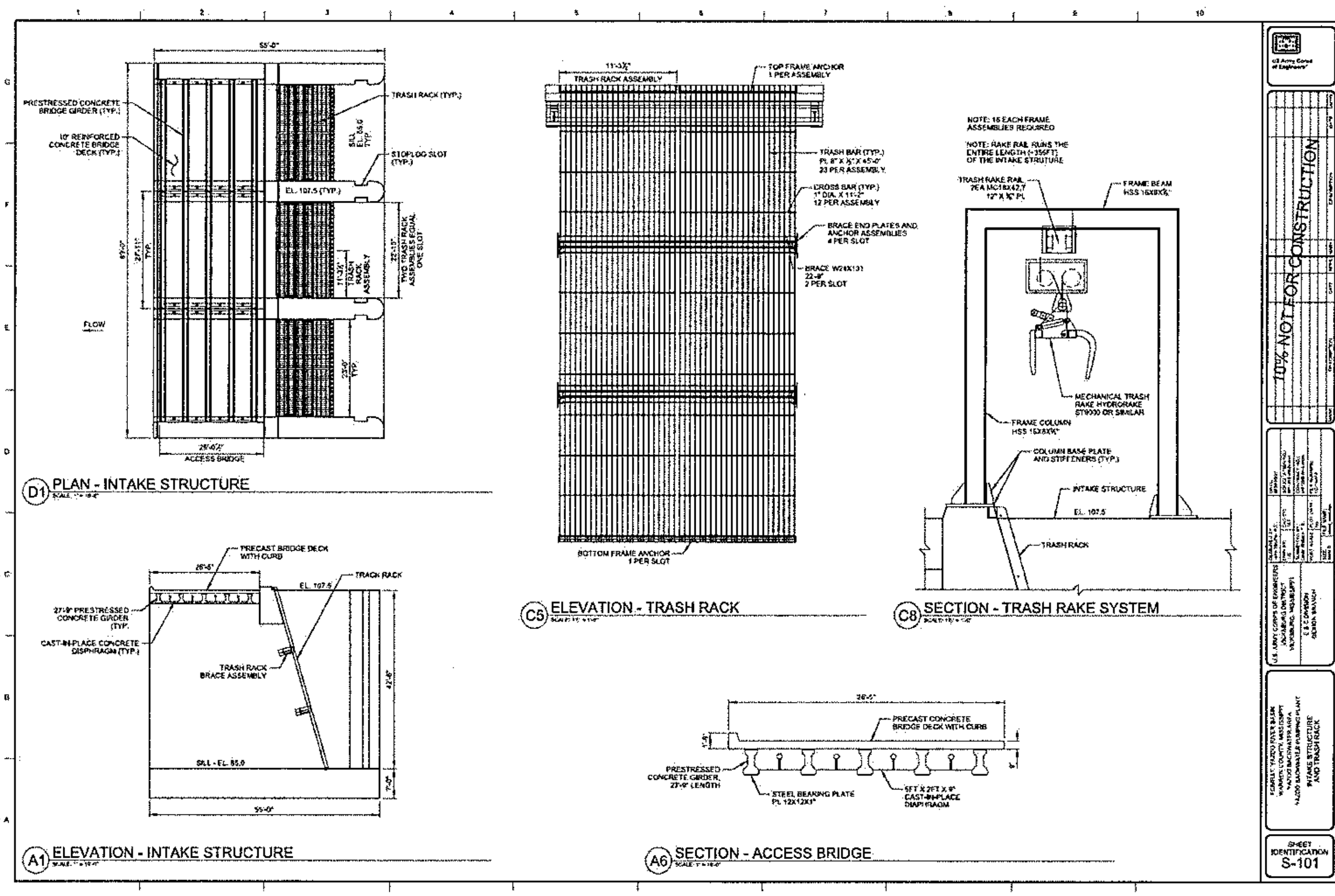


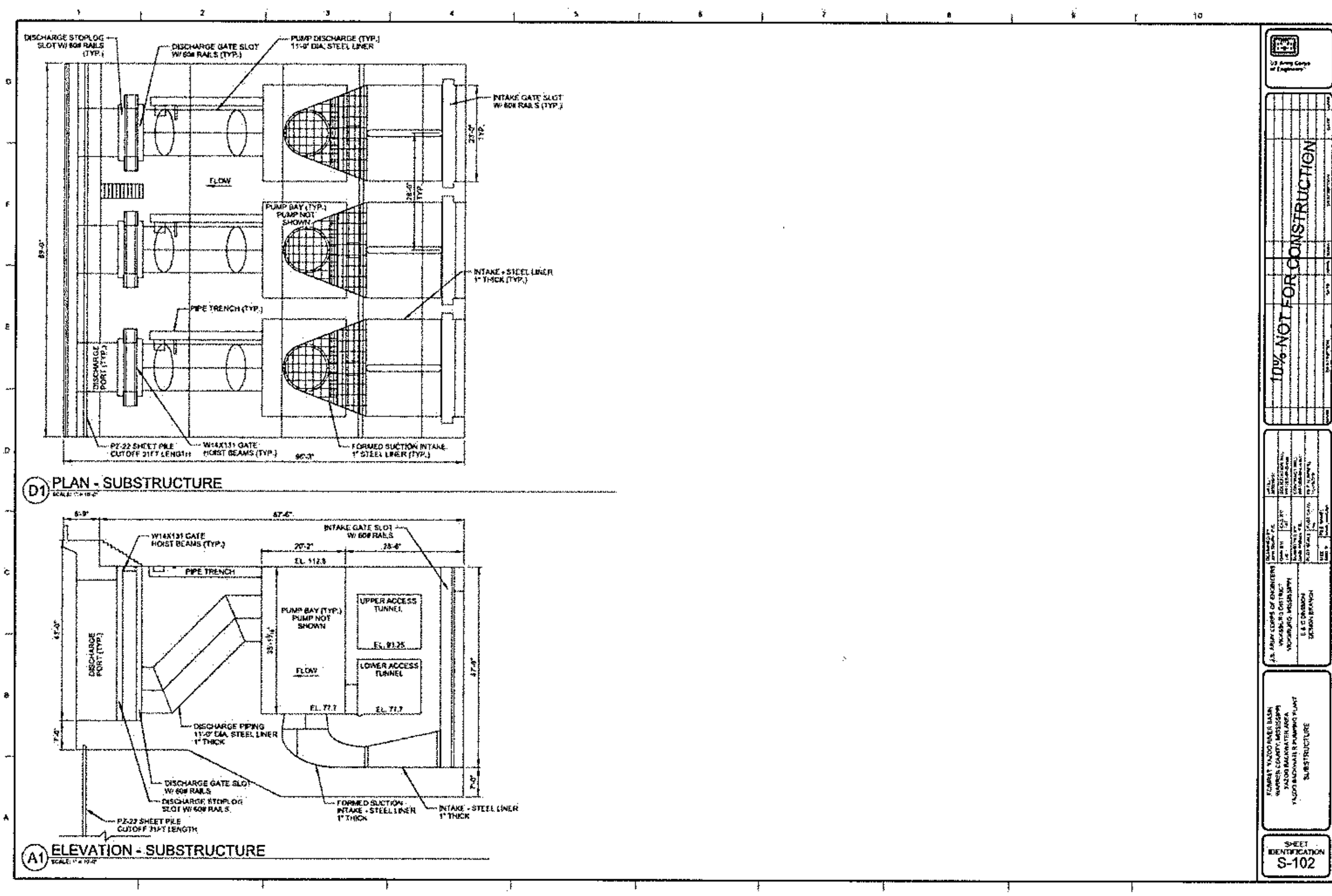
Project Status


NOTES:
1. STRUCTURAL SHAPES AND PLATES SHALL BE ASTM A 572 GRADE 50.
2. CONCRETE SHALL HAVE A COMPRESSIVE STRENGTH f'c = 4,000 PSI AT 28 DAYS (90 DAYS IF POZZOLAN IS USED).
3. PRESTRESSED CONCRETE PILES AND BRIDGE GIRDERS SHALL HAVE A COMPRESSIVE STRENGTH f'c = 5,000 PSI AT 28 DAYS (90 DAYS IF POZZOLAN IS USED).
4. BOLTS SHALL BE ASTM A 325, WITH ASTM A 563 NUTS AND ASTM F 436 WASHERS.
5. STEEL H-PILES SHALL BE ASTM A 572 GRADE 60.
6. STEEL SHEET PILING SHALL BE ASTM A 690.
10% NOT FOR CONSTRUCTION
FLOW
CL INTAKE CHANNEL
CL DISCHARGE CHANNEL
INTAKE STRUCTURE (TYP.)
WINGWALL (TYP.)
INTAKE STRUCTURE
SERVICE BAY
CONTROL BUILDING
SUBSTRUCTURE AND PUMP STATION
FLOODWALL (TYP.)
STAIR
PARAPET
SUBSTRUCTURE (TYP.)
TYP.
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
STRUCTURAL PLAN
SHEET IDENTIFICATION
S-100A
A1 PLAN - PUMPING PLANT

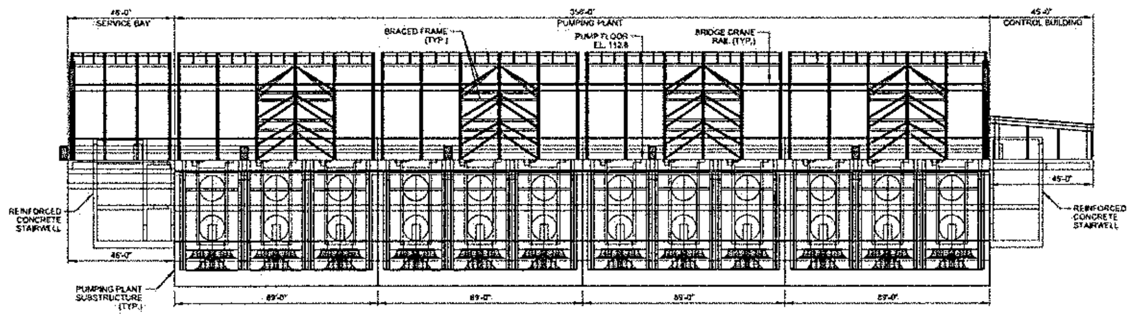


A1 SECTION - PUMPING PLANT

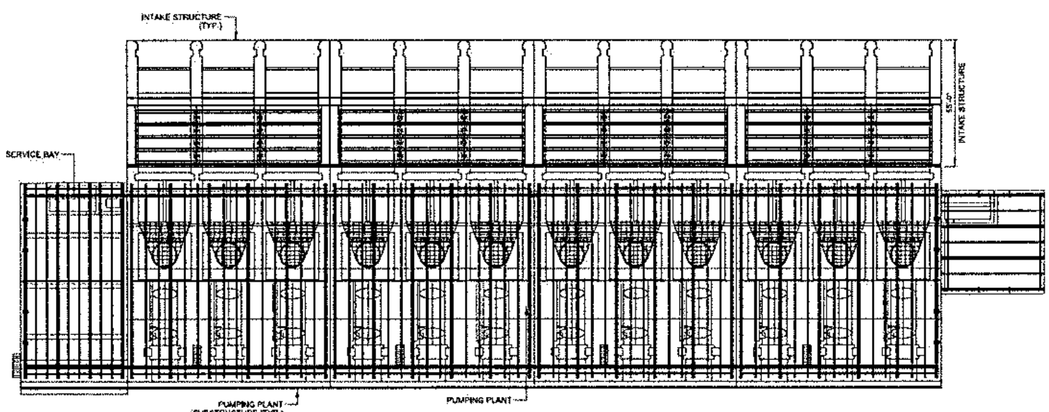

PLAN - INTAKE STRUCTURE
PRESTRESSED CONCRETE BRIDGE GIRDER (TYP.)
10" REINFORCED CONCRETE BRIDGE DECK (TYP.)
TRASH RACK (TYP.)
STOPLOG SLOT (TYP.)
EL. 107.5 (TYP.)
FLOW
ACCESS BRIDGE
ELEVATION - INTAKE STRUCTURE
PRECAST BRIDGE DECK WITH CURB
27'-9" PRESTRESSED CONCRETE GIRDER (TYP.)
CAST-IN-PLACE CONCRETE DIAPHRAGM (TYP.)
TRASH RACK BRACE ASSEMBLY
SILL - EL. 85.0
EL. 107.5
ELEVATION - TRASH RACK
TRASH RACK ASSEMBLY
TOP FRAME ANCHOR 1 PER ASSEMBLY
TRASH BAR (TYP.)
23 PER ASSEMBLY
CROSS BAR (TYP.)
12 PER ASSEMBLY
BRACE END PLATES AND ANCHOR ASSEMBLIES 4 PER SLOT
2 PER SLOT
BOTTOM FRAME ANCHOR 1 PER SLOT
SECTION - TRASH RAKE SYSTEM
NOTE: 16 EACH FRAME ASSEMBLIES REQUIRED
NOTE: RAKE RAIL RUNS THE ENTIRE LENGTH OF THE INTAKE STRUTURE
TRASH RAKE RAIL
FRAME BEAM
MECHANICAL TRASH RAKE HYDRORAKE ST9000 OR SIMILAR
FRAME COLUMN
COLUMN BASE PLATE AND STIFFENERS (TYP.)
INTAKE STRUCTURE
TRASH RACK
SECTION - ACCESS BRIDGE
PRECAST CONCRETE BRIDGE DECK WITH CURB
PRESTRESSED CONCRETE GIRDER, 27'-9" LENGTH
STEEL BEARING PLATE
CAST-IN-PLACE DIAPHRAGM
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS
INTAKE STRUCTURE AND TRASH RACK
SHEET IDENTIFICATION
S-101



DISCHARGE STOPLOG SLOT W/ 60# RAILS (TYP.)
DISCHARGE GATE SLOT W/ 60# RAILS (TYP.)
PUMP DISCHARGE (TYP.) 11'-0" DIA. STEEL LINER
INTAKE GATE SLOT W/ 60# RAILS (TYP.)
FLOW
PUMP BAY (TYP.) PUMP NOT SHOWN
INTAKE - STEEL LINER 1" THICK (TYP.)
PIPE TRENCH (TYP.)
DISCHARGE PORT (TYP.)
PZ-22 SHEET PILE CUTOFF 31FT LENGTH
W14X131 GATE HOIST BEAMS (TYP.)
FORMED SUCTION INTAKE 1" STEEL LINER (TYP.)
D1 PLAN - SUBSTRUCTURE
W14X131 GATE HOIST BEAMS (TYP.)
INTAKE GATE SLOT W/ 60# RAILS
PIPE TRENCH
EL. 112.5
PUMP BAY (TYP.) PUMP NOT SHOWN
UPPER ACCESS TUNNEL
LOWER ACCESS TUNNEL
EL. 77.7
DISCHARGE PIPING 11'-0" DIA. STEEL LINER 1" THICK
DISCHARGE GATE SLOT W/ 60# RAILS
DISCHARGE STOPLOG SLOT W/ 60# RAILS
FORMED SUCTION INTAKE - STEEL LINER 1" THICK
INTAKE - STEEL LINER 1" THICK
PZ-22 SHEET PILE CUTOFF 31FT LENGTH
A1 ELEVATION - SUBSTRUCTURE
US Army Corps of Engineers
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS VICKSBURG DISTRICT VICKSBURG, MISSISSIPPI
E & C DIVISION DESIGN BRANCH
YAZOO RIVER BASIN WARREN COUNTY, MISSISSIPPI YAZOO BACKWATER AREA YAZOO BACKWATER PUMPING PLANT
SUBSTRUCTURE
SHEET IDENTIFICATION
S-102



E1 ELEVATION - PREFABRICATED METAL BUILDING



A1 PLAN - PREFABRICATED METAL BUILDING

US Army Corps of Engineers

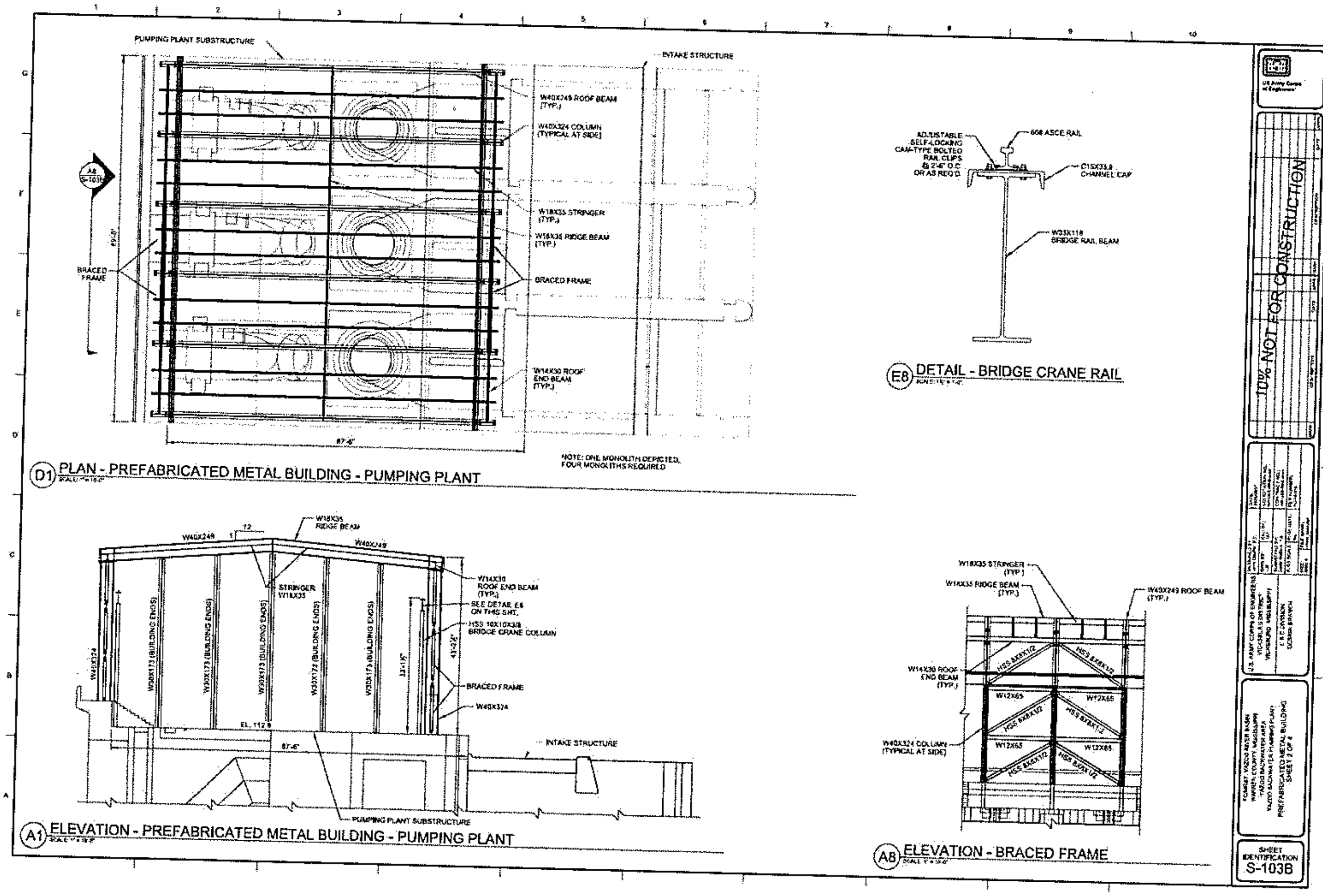
10% NOT FOR CONSTRUCTION

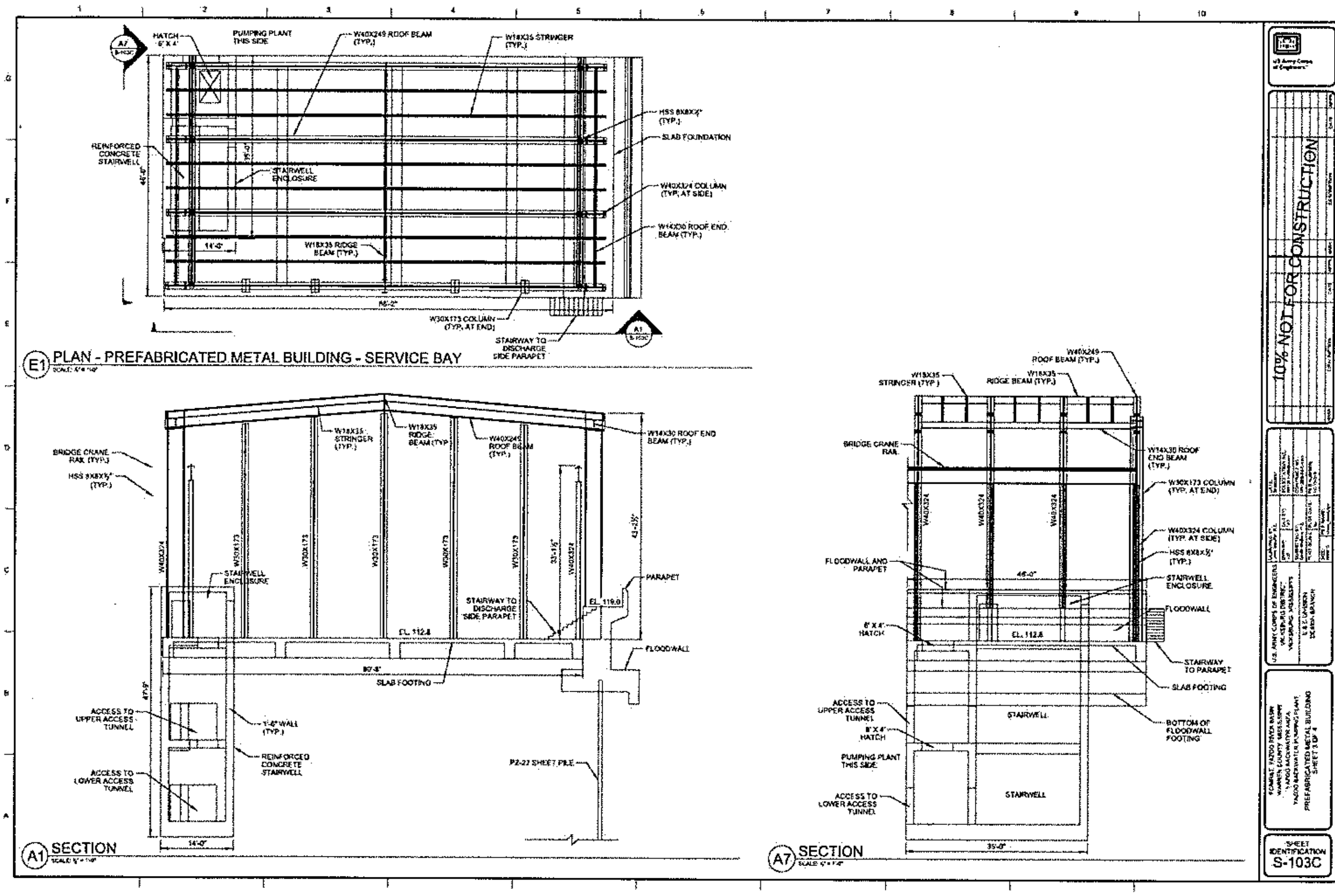
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI

E & C DIVISION
DESIGN BRANCH

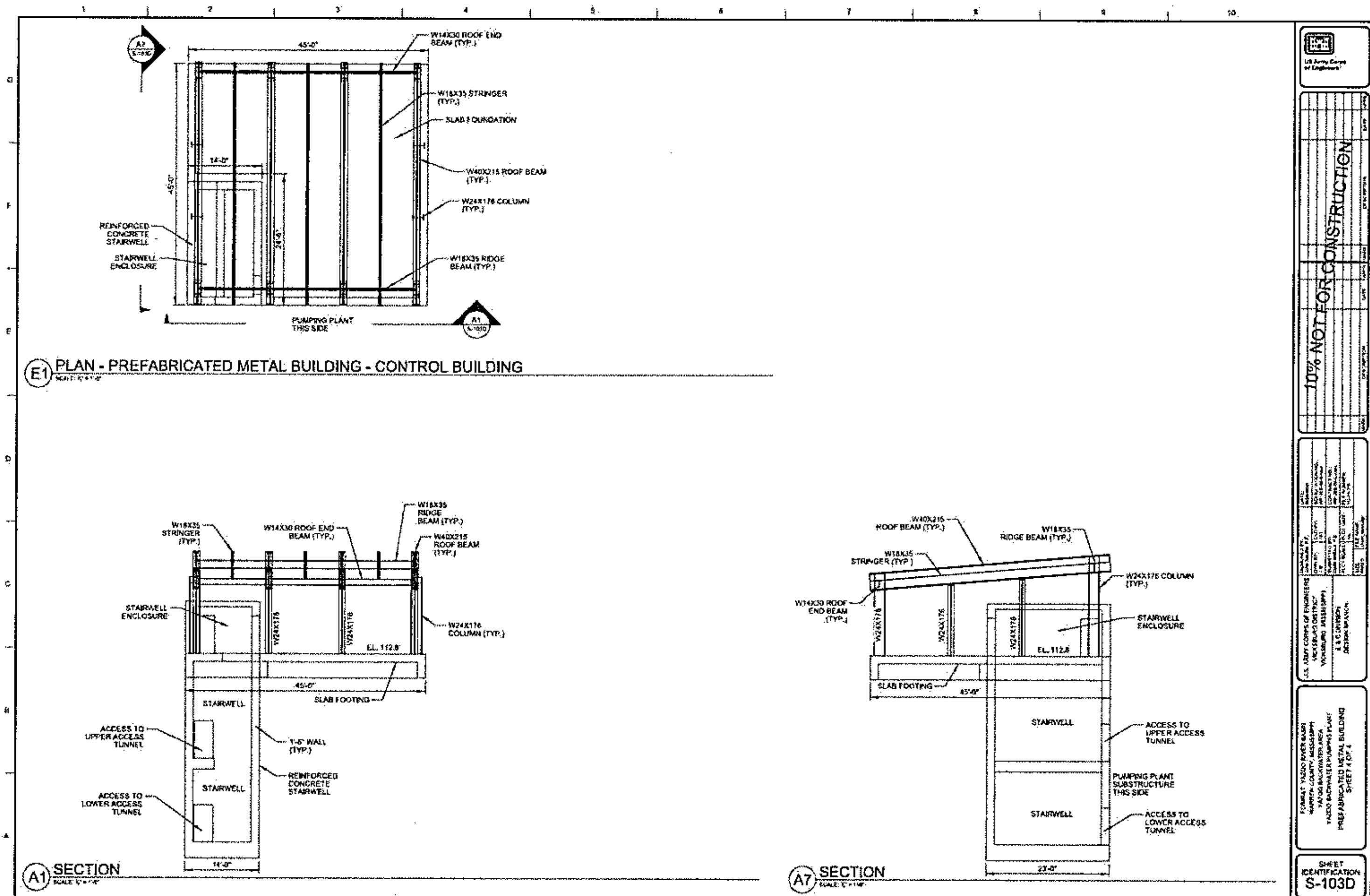
PREFABRICATED METAL BUILDING
SHEET 1 OF 4

SHEET IDENTIFICATION
S-103A

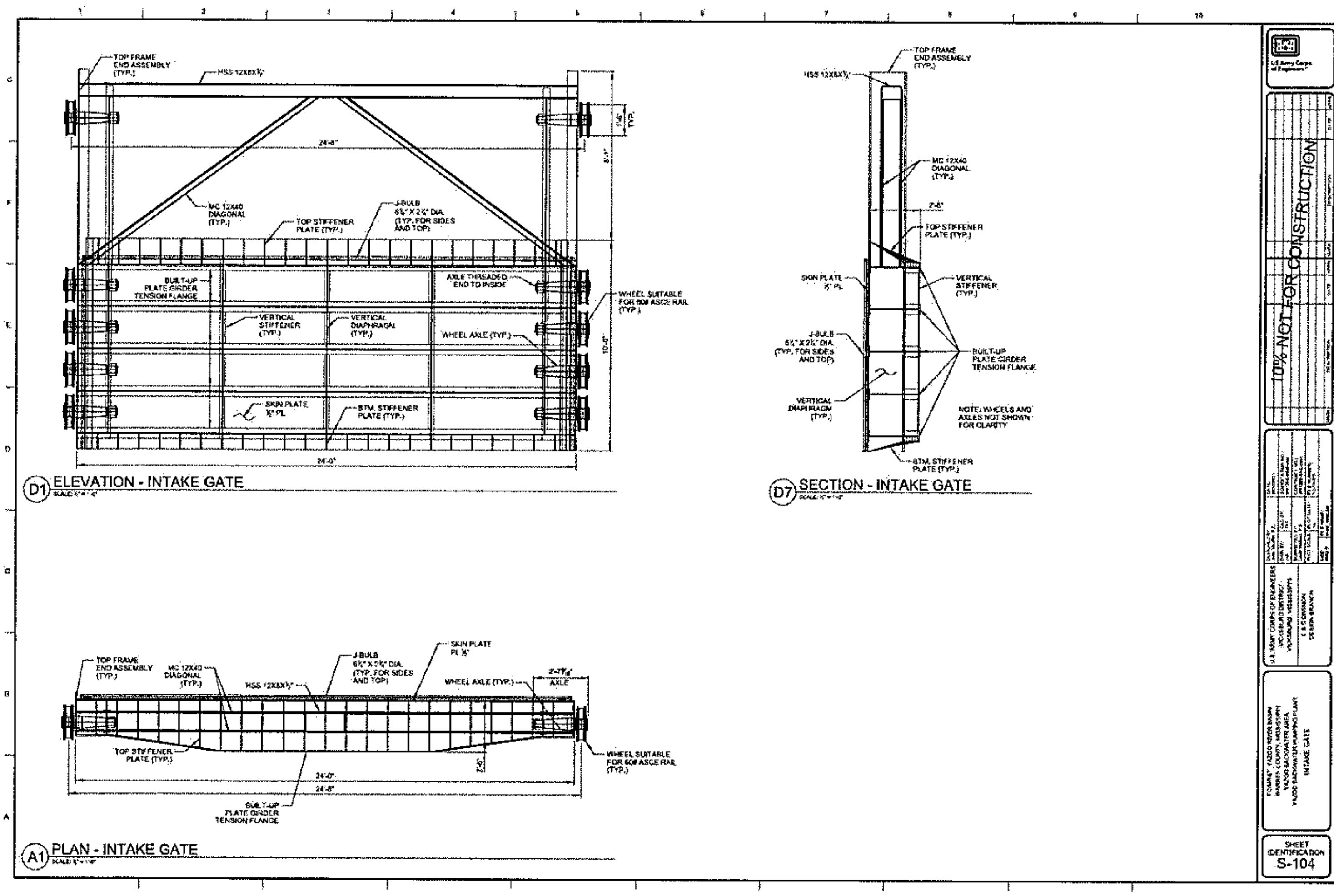

PUMPING PLANT SUBSTRUCTURE
INTAKE STRUCTURE
W40X249 ROOF BEAM (TYP.)
W40X324 COLUMN (TYPICAL AT SIDE)
W18X35 STRINGER (TYP.)
W18X35 RIDGE BEAM (TYP.)
BRACED FRAME
W14X30 ROOF END BEAM (TYP.)
87'-6"
NOTE: ONE MONOLITH DEPICTED, FOUR MONOLITHS REQUIRED
D1 PLAN - PREFABRICATED METAL BUILDING - PUMPING PLANT
ADJUSTABLE SELF-LOCKING CAM-TYPE BOLTED RAIL CLIPS @ 2'-6" O.C. OR AS REQ'D.
60# ASCE RAIL
C15X33.9 CHANNEL CAP
W33X118 BRIDGE RAIL BEAM
E8 DETAIL - BRIDGE CRANE RAIL
W18X35 RIDGE BEAM
W40X249
STRINGER W18X35
W14X30 ROOF END BEAM (TYP.)
SEE DETAIL E8 ON THIS SHT.
HSS 10X10X3/8 BRIDGE CRANE COLUMN
W40X324
W30X173 (BUILDING ENDS)
EL. 112.0
A1 ELEVATION - PREFABRICATED METAL BUILDING - PUMPING PLANT
W12X65
W40X324 COLUMN (TYPICAL AT SIDE)
A8 ELEVATION - BRACED FRAME
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS VICKSBURG DISTRICT VICKSBURG, MISSISSIPPI
YAZOO BACKWATER PUMPING PLANT PREFABRICATED METAL BUILDING
SHEET IDENTIFICATION S-103B



HATCH 6' X 4'
PUMPING PLANT THIS SIDE
W40X249 ROOF BEAM (TYP.)
W18X35 STRINGER (TYP.)
HSS 8X8X½" (TYP.)
SLAB FOUNDATION
REINFORCED CONCRETE STAIRWELL
STAIRWELL ENCLOSURE
W40X324 COLUMN (TYP. AT SIDE)
W14X30 ROOF END BEAM (TYP.)
W18X35 RIDGE BEAM (TYP.)
W30X173 COLUMN (TYP. AT END)
STAIRWAY TO DISCHARGE SIDE PARAPET
E1 PLAN - PREFABRICATED METAL BUILDING - SERVICE BAY
BRIDGE CRANE RAIL (TYP.)
PARAPET
EL. 119.0
EL. 112.8
FLOODWALL
SLAB FOOTING
ACCESS TO UPPER ACCESS TUNNEL
1'-6" WALL (TYP.)
REINFORCED CONCRETE STAIRWELL
ACCESS TO LOWER ACCESS TUNNEL
PZ-27 SHEET PILE
A1 SECTION
FLOODWALL AND PARAPET
8' X 4' HATCH
STAIRWAY TO PARAPET
BOTTOM OF FLOODWALL FOOTING
STAIRWELL
PUMPING PLANT THIS SIDE
A7 SECTION
U.S. Army Corps of Engineers
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS VICKSBURG DISTRICT VICKSBURG, MISSISSIPPI
E & E DIVISION DESIGN BRANCH
YAZOO BACKWATER PUMPING PLANT
PREFABRICATED METAL BUILDING SHEET 3 OF 4
SHEET IDENTIFICATION
S-103C

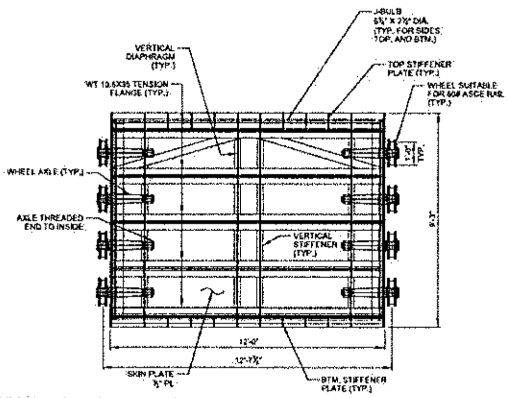


PLAN - PREFABRICATED METAL BUILDING - CONTROL BUILDING
45'-0"
W14X30 ROOF END BEAM (TYP.)
W18X35 STRINGER (TYP.)
SLAB FOUNDATION
W40X215 ROOF BEAM (TYP.)
W24X176 COLUMN (TYP.)
W18X35 RIDGE BEAM (TYP.)
14'-0"
24'-0"
REINFORCED CONCRETE STAIRWELL
STAIRWELL ENCLOSURE
PUMPING PLANT THIS SIDE
SECTION
W18X35 STRINGER (TYP.)
W14X30 ROOF END BEAM (TYP.)
W18X35 RIDGE BEAM (TYP.)
W40X215 ROOF BEAM (TYP.)
STAIRWELL ENCLOSURE
W24X176
W24X176 COLUMN (TYP.)
EL. 112.8'
SLAB FOOTING
STAIRWELL
ACCESS TO UPPER ACCESS TUNNEL
1'-6" WALL (TYP.)
REINFORCED CONCRETE STAIRWELL
ACCESS TO LOWER ACCESS TUNNEL
14'-0"
SECTION
W40X215 ROOF BEAM (TYP.)
W18X35 RIDGE BEAM (TYP.)
W18X35 STRINGER (TYP.)
W24X176 COLUMN (TYP.)
W14X30 ROOF END BEAM (TYP.)
STAIRWELL ENCLOSURE
23'-0"
PUMPING PLANT SUBSTRUCTURE THIS SIDE
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
E & C DIVISION
DESIGN BRANCH
YAZOO BACKWATER AREA
YAZOO BACKWATER PUMPING PLANT
PREFABRICATED METAL BUILDING
SHEET 4 OF 4
SHEET IDENTIFICATION
S-103D

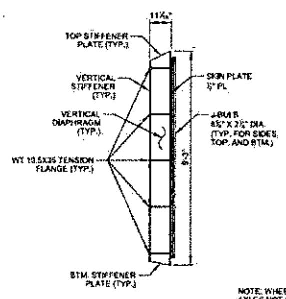


TOP FRAME END ASSEMBLY (TYP.)
HSS 12X8X½"
24'-8"
MC 12X40 DIAGONAL (TYP.)
TOP STIFFENER PLATE (TYP.)
J-BULB 6¾" X 2¼" DIA. (TYP. FOR SIDES AND TOP)
BUILT-UP PLATE GIRDER TENSION FLANGE
AXLE THREADED END TO INSIDE
WHEEL SUITABLE FOR 60# ASCE RAIL (TYP.)
VERTICAL STIFFENER (TYP.)
VERTICAL DIAPHRAGM (TYP.)
WHEEL AXLE (TYP.)
SKIN PLATE ½" PL
BTM. STIFFENER PLATE (TYP.)
24'-0"
D1 ELEVATION - INTAKE GATE
SKIN PLATE ½" PL
NOTE: WHEELS AND AXLES NOT SHOWN FOR CLARITY
D7 SECTION - INTAKE GATE
A1 PLAN - INTAKE GATE
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS
INTAKE GATE
SHEET IDENTIFICATION
S-104

10% NOT FOR CONSTRUCTION

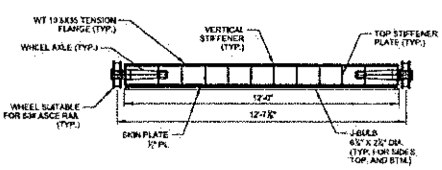


D1 ELEVATION - DISCHARGE GATE



NOTE: WHEELS AND AXLES NOT SHOWN FOR CLARITY

D7 SECTION - DISCHARGE GATE



A1 PLAN - DISCHARGE GATE

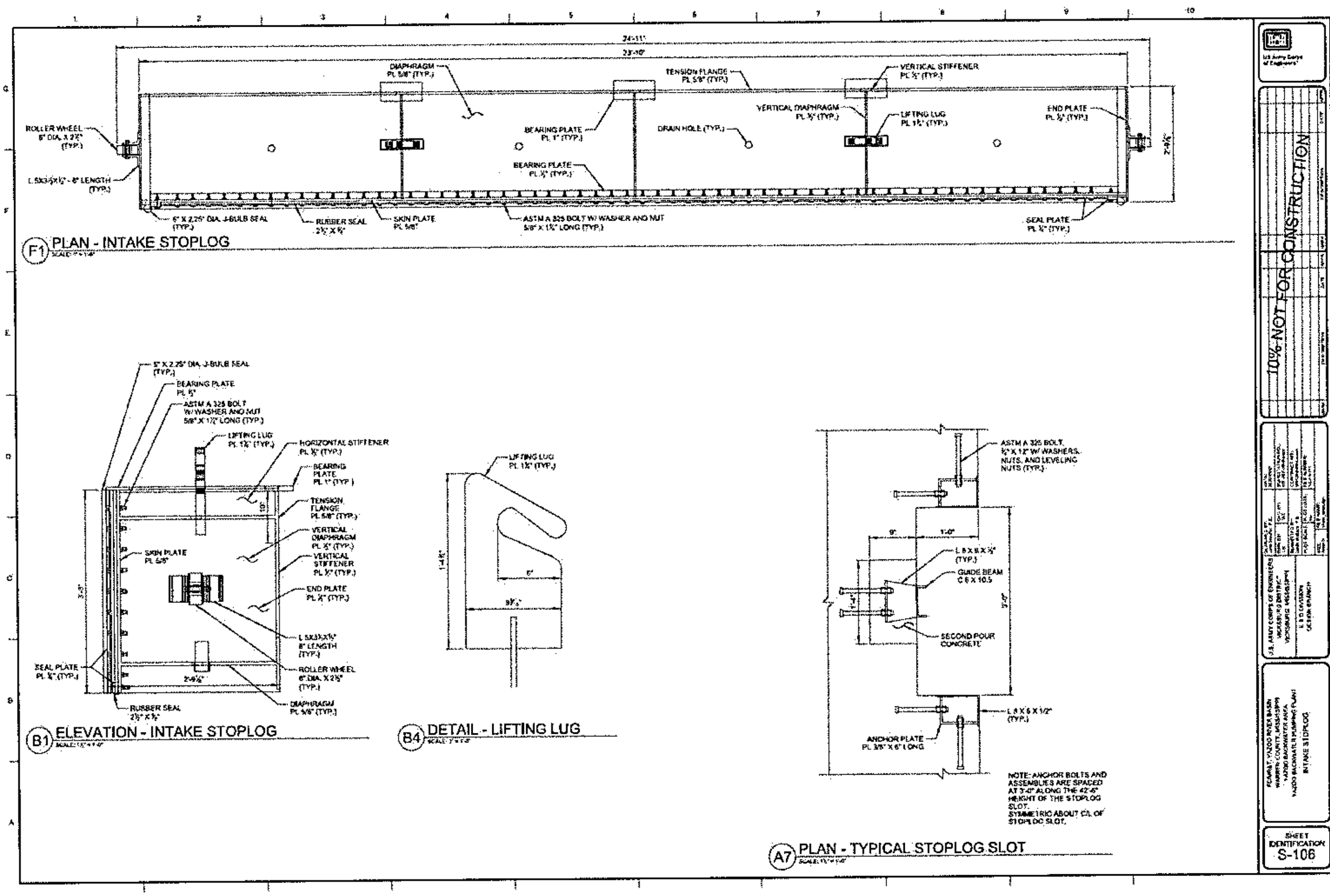
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI

E & C DIVISION
DESIGN BRANCH

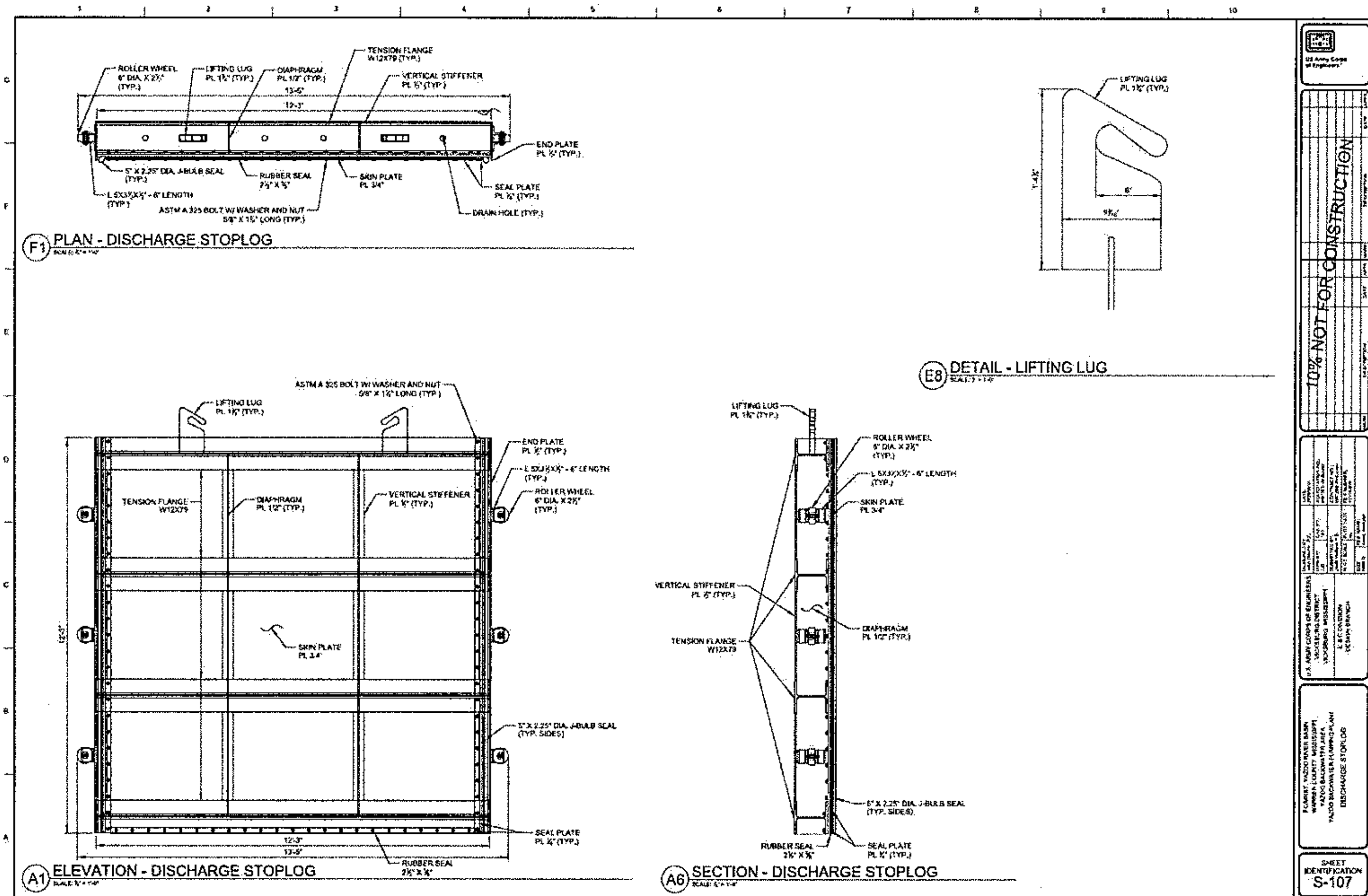
WARREN COUNTY, MISSISSIPPI
YAZOO BACKWATER AREA
YAZOO BACKWATER PUMPING PLANT

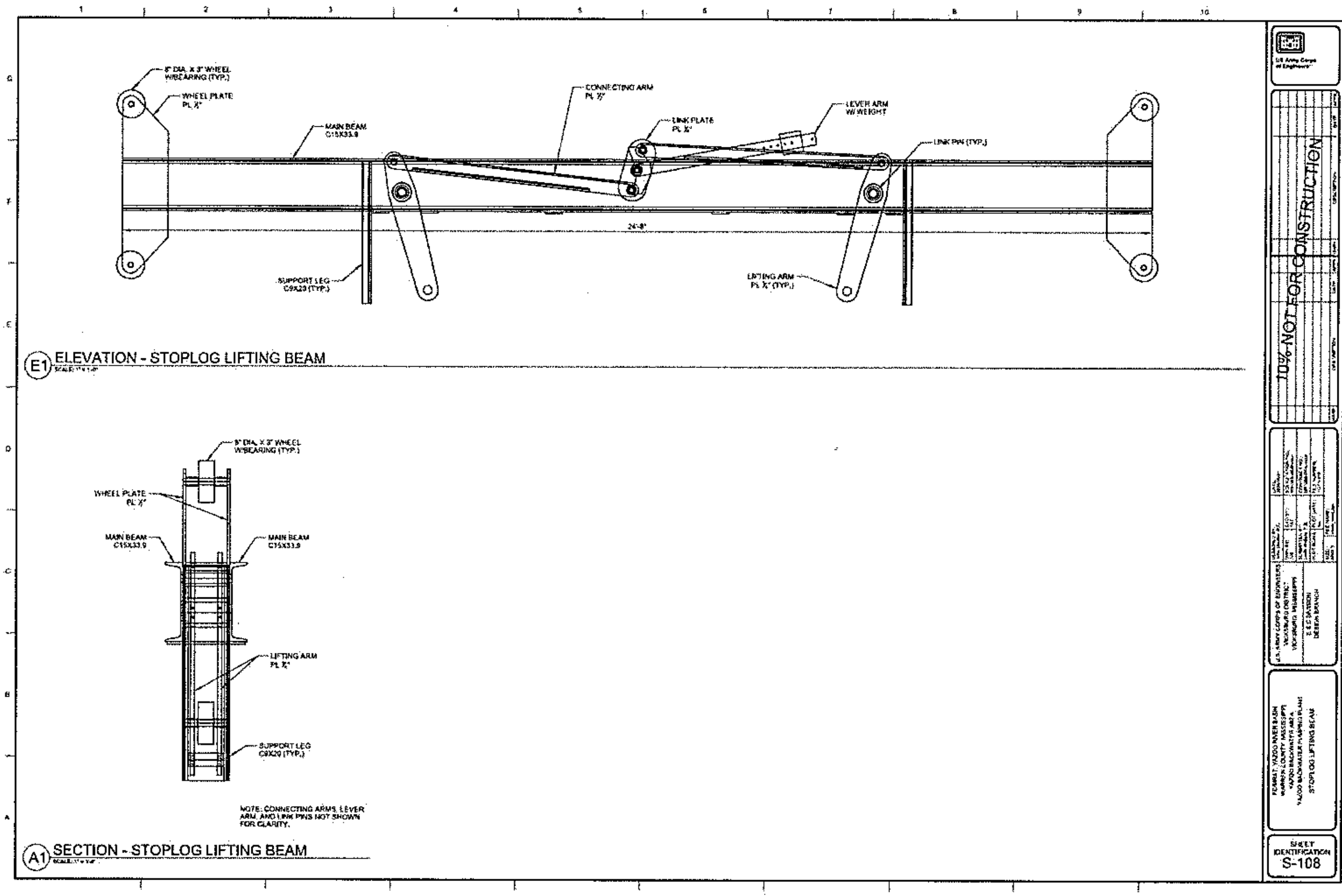
DISCHARGE GATE

SHEET IDENTIFICATION
S-105

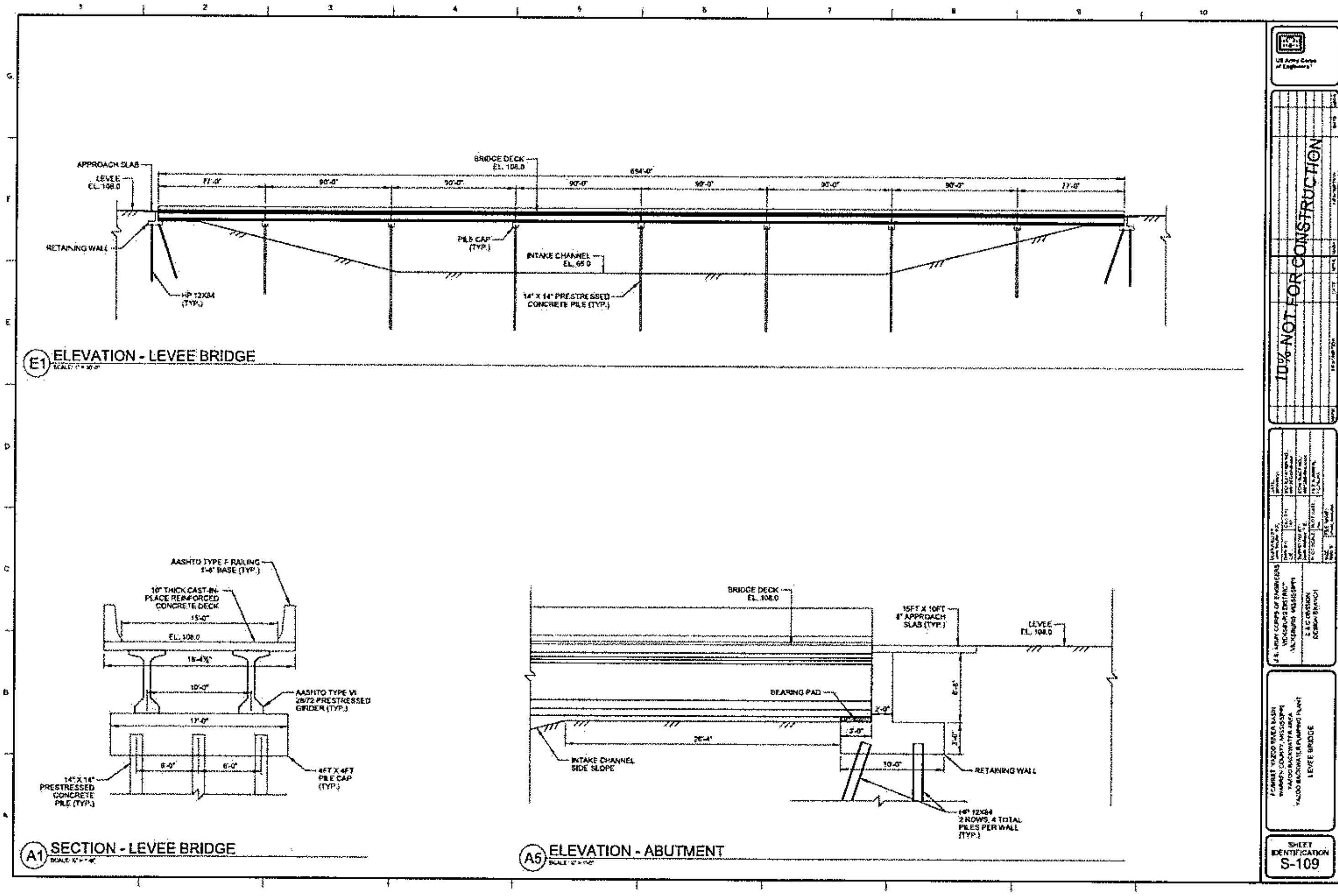


24'-11"
23'-10"
DIAPHRAGM PL 5/8" (TYP.)
TENSION FLANGE PL 5/8" (TYP.)
VERTICAL STIFFENER PL ½" (TYP.)
VERTICAL DIAPHRAGM PL ½" (TYP.)
LIFTING LUG PL 1½" (TYP.)
END PLATE PL ½" (TYP.)
ROLLER WHEEL 6" DIA. X 2½" (TYP.)
BEARING PLATE PL 1" (TYP.)
DRAIN HOLE (TYP.)
BEARING PLATE PL ½" (TYP.)
6" X 2.25" DIA. J-BULB SEAL (TYP.)
RUBBER SEAL 2½" X ½"
SKIN PLATE PL 5/8"
ASTM A 325 BOLT W/ WASHER AND NUT 5/8" X 1½" LONG (TYP.)
SEAL PLATE PL ½" (TYP.)
F1 PLAN - INTAKE STOPLOG
HORIZONTAL STIFFENER PL ½" (TYP.)
TENSION FLANGE PL 5/8" (TYP.)
VERTICAL STIFFENER PL ½" (TYP.)
DIAPHRAGM PL 5/8" (TYP.)
B1 ELEVATION - INTAKE STOPLOG
B4 DETAIL - LIFTING LUG
ASTM A 325 BOLT, ½" X 12" W/ WASHERS, NUTS, AND LEVELING NUTS (TYP.)
GUIDE BEAM C 6 X 10.5
SECOND POUR CONCRETE
L 6 X 6 X 1/2" (TYP.)
ANCHOR PLATE PL 3/8" X 6" LONG
NOTE: ANCHOR BOLTS AND ASSEMBLIES ARE SPACED AT 3'-0" ALONG THE 42'-6" HEIGHT OF THE STOPLOG SLOT. SYMMETRIC ABOUT C/L OF STOPLOG SLOT.
A7 PLAN - TYPICAL STOPLOG SLOT
US Army Corps of Engineers
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS VICKSBURG DISTRICT VICKSBURG, MISSISSIPPI
E & D DIVISION DESIGN BRANCH
INTAKE STOPLOG
SHEET IDENTIFICATION
S-106

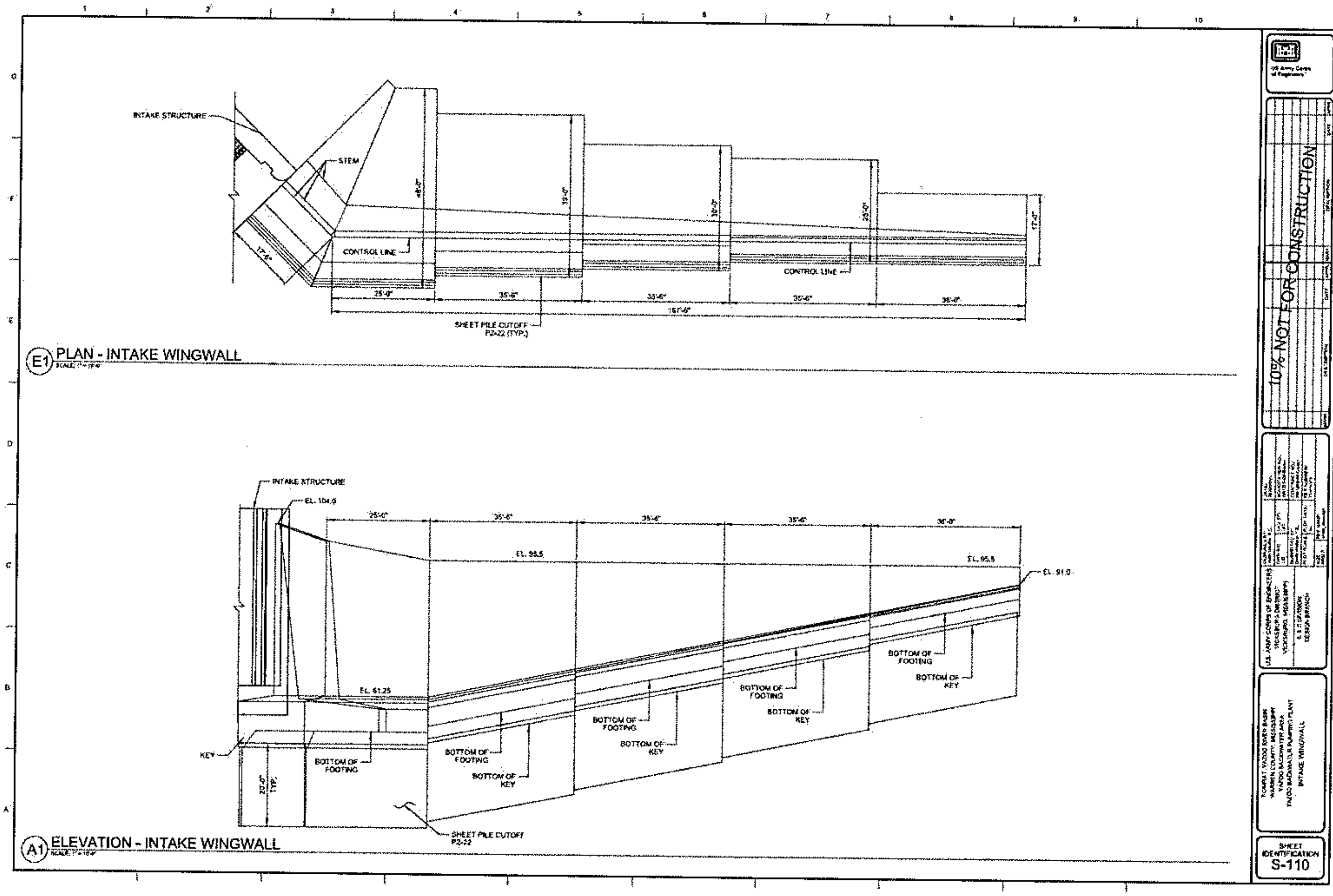

PLAN - DISCHARGE STOPLOG
ELEVATION - DISCHARGE STOPLOG
SECTION - DISCHARGE STOPLOG
DETAIL - LIFTING LUG
ROLLER WHEEL
LIFTING LUG
DIAPHRAGM
TENSION FLANGE
VERTICAL STIFFENER
END PLATE
RUBBER SEAL
SKIN PLATE
SEAL PLATE
DRAIN HOLE (TYP.)
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
DISCHARGE STOPLOG
SHEET IDENTIFICATION
S-107

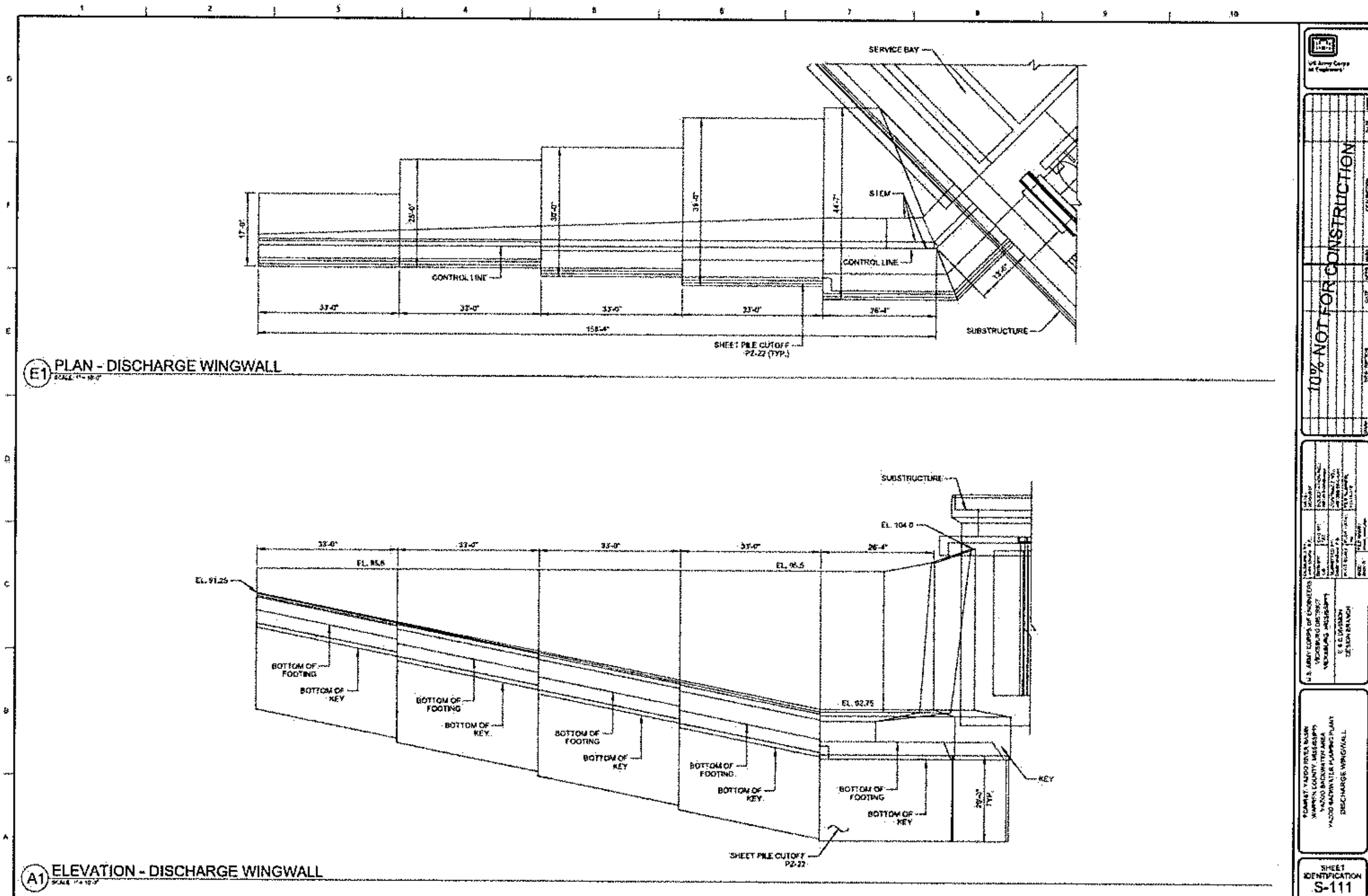


8" DIA. X 3" WHEEL W/BEARING (TYP.)
WHEEL PLATE PL ¾"
MAIN BEAM C15X33.9
CONNECTING ARM PL ¾"
LINK PLATE PL ¾"
LEVER ARM W/ WEIGHT
LINK PIN (TYP.)
24'-8"
SUPPORT LEG C9X20 (TYP.)
LIFTING ARM PL ¾" (TYP.)
E1 ELEVATION - STOPLOG LIFTING BEAM
8" DIA. X 3" WHEEL W/BEARING (TYP.)
WHEEL PLATE PL ¾"
MAIN BEAM C15X33.9
MAIN BEAM C15X33.9
LIFTING ARM PL ¾"
SUPPORT LEG C9X20 (TYP.)
NOTE: CONNECTING ARMS, LEVER ARM, AND LINK PINS NOT SHOWN FOR CLARITY.
A1 SECTION - STOPLOG LIFTING BEAM
US Army Corps of Engineers
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
E & C DIVISION
DESIGN BRANCH
STOPLOG LIFTING BEAM
SHEET IDENTIFICATION
S-108

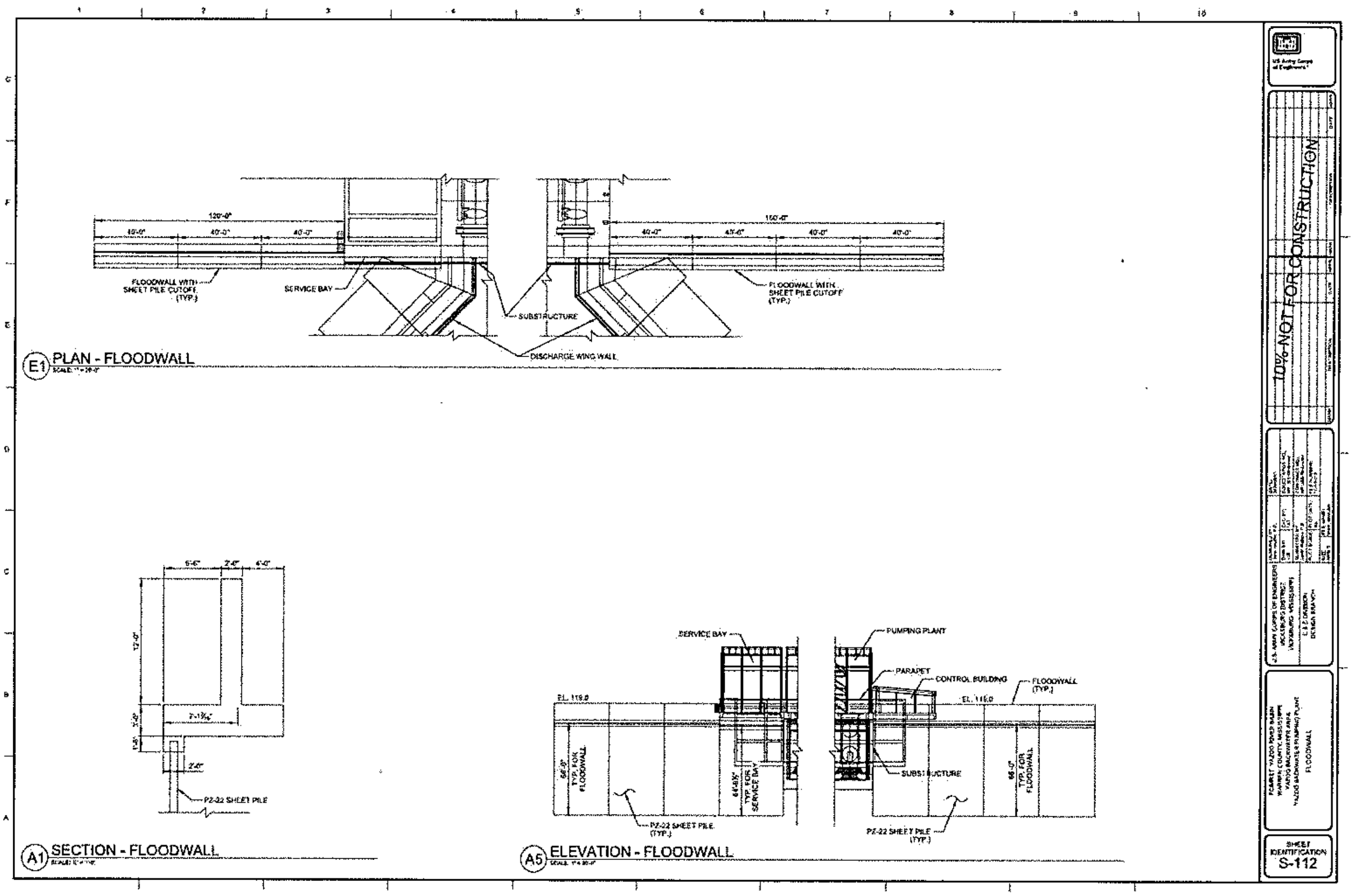


APPROACH SLAB
LEVEE EL. 108.0
BRIDGE DECK EL. 108.0
694'-0"
77'-0"
90'-0"
RETAINING WALL
PILE CAP (TYP.)
INTAKE CHANNEL EL. 65.0
HP 12X84 (TYP.)
14" X 14" PRESTRESSED CONCRETE PILE (TYP.)
E1 ELEVATION - LEVEE BRIDGE
AASHTO TYPE F RAILING 1'-6" BASE (TYP.)
10" THICK CAST-IN-PLACE REINFORCED CONCRETE DECK
15'-0"
EL. 108.0
18'-4½"
10'-0"
AASHTO TYPE VI 28/72 PRESTRESSED GIRDER (TYP.)
17'-0"
6'-0"
14" X 14" PRESTRESSED CONCRETE PILE (TYP.)
4FT X 4FT PILE CAP (TYP.)
A1 SECTION - LEVEE BRIDGE
BRIDGE DECK EL. 108.0
15FT X 10FT 8" APPROACH SLAB (TYP.)
LEVEE EL. 108.0
BEARING PAD
2'-0"
3'-0"
26'-4"
10'-0"
INTAKE CHANNEL SIDE SLOPE
RETAINING WALL
HP 12X84 2 ROWS, 4 TOTAL PILES PER WALL (TYP.)
A5 ELEVATION - ABUTMENT
10% NOT FOR CONSTRUCTION
LEVEE BRIDGE
SHEET IDENTIFICATION
S-109

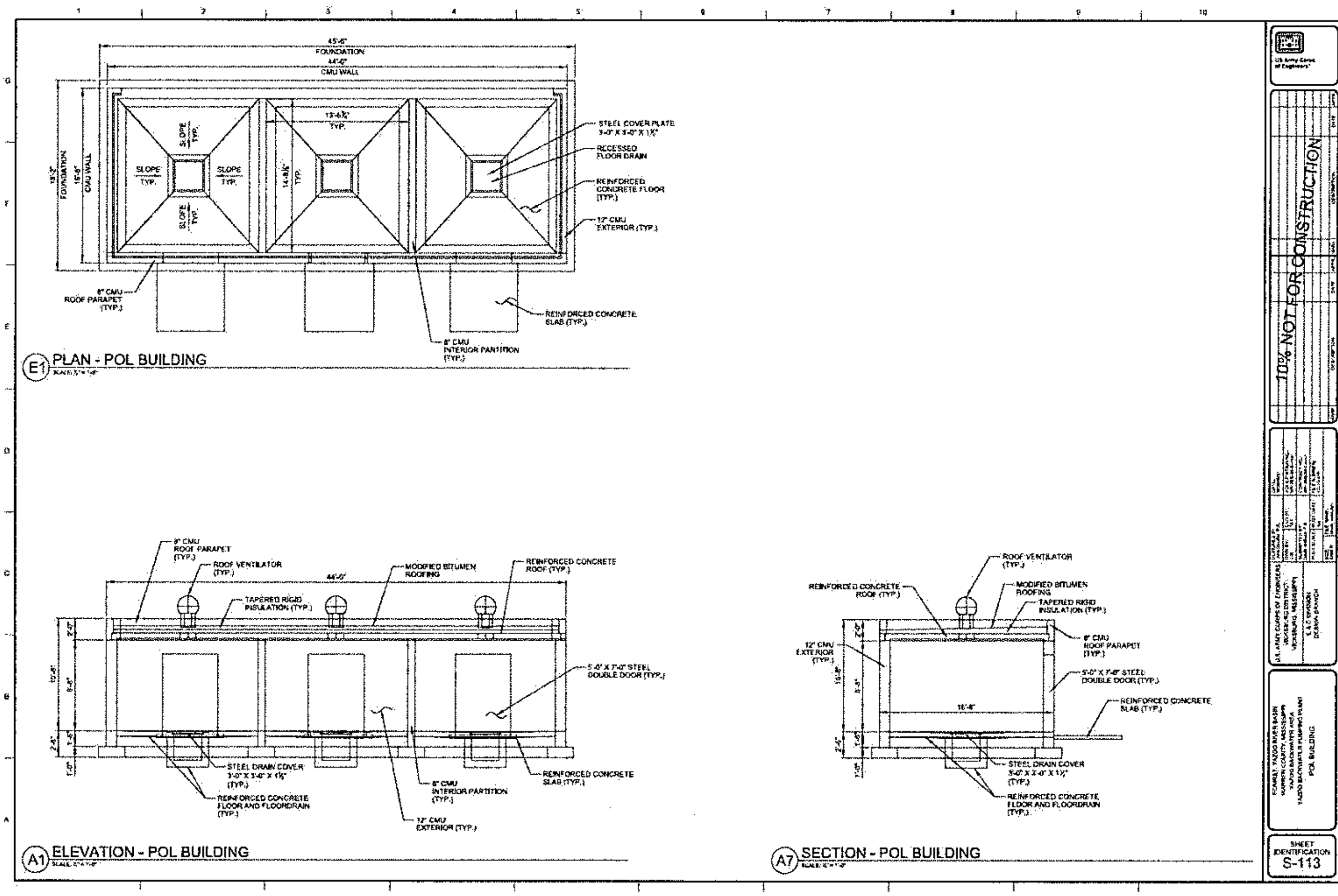

INTAKE STRUCTURE
STEM
48'-0"
39'-0"
30'-0"
25'-0"
17'-0"
17'-6"
CONTROL LINE
CONTROL LINE
25'-0"
35'-6"
35'-6"
35'-6"
36'-0"
167'-6"
SHEET PILE CUTOFF
PZ-22 (TYP.)
E1 PLAN - INTAKE WINGWALL
INTAKE STRUCTURE
EL. 104.0
25'-0"
35'-6"
35'-6"
35'-6"
36'-0"
EL. 95.5
EL. 95.5
EL. 91.0
EL. 61.25
KEY
BOTTOM OF FOOTING
BOTTOM OF FOOTING
BOTTOM OF KEY
BOTTOM OF FOOTING
BOTTOM OF KEY
BOTTOM OF FOOTING
BOTTOM OF KEY
BOTTOM OF FOOTING
BOTTOM OF KEY
22'-0" TYP.
SHEET PILE CUTOFF
PZ-22
A1 ELEVATION - INTAKE WINGWALL
US Army Corps of Engineers
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
E & C DIVISION
DESIGN BRANCH
INTAKE WINGWALL
SHEET IDENTIFICATION
S-110

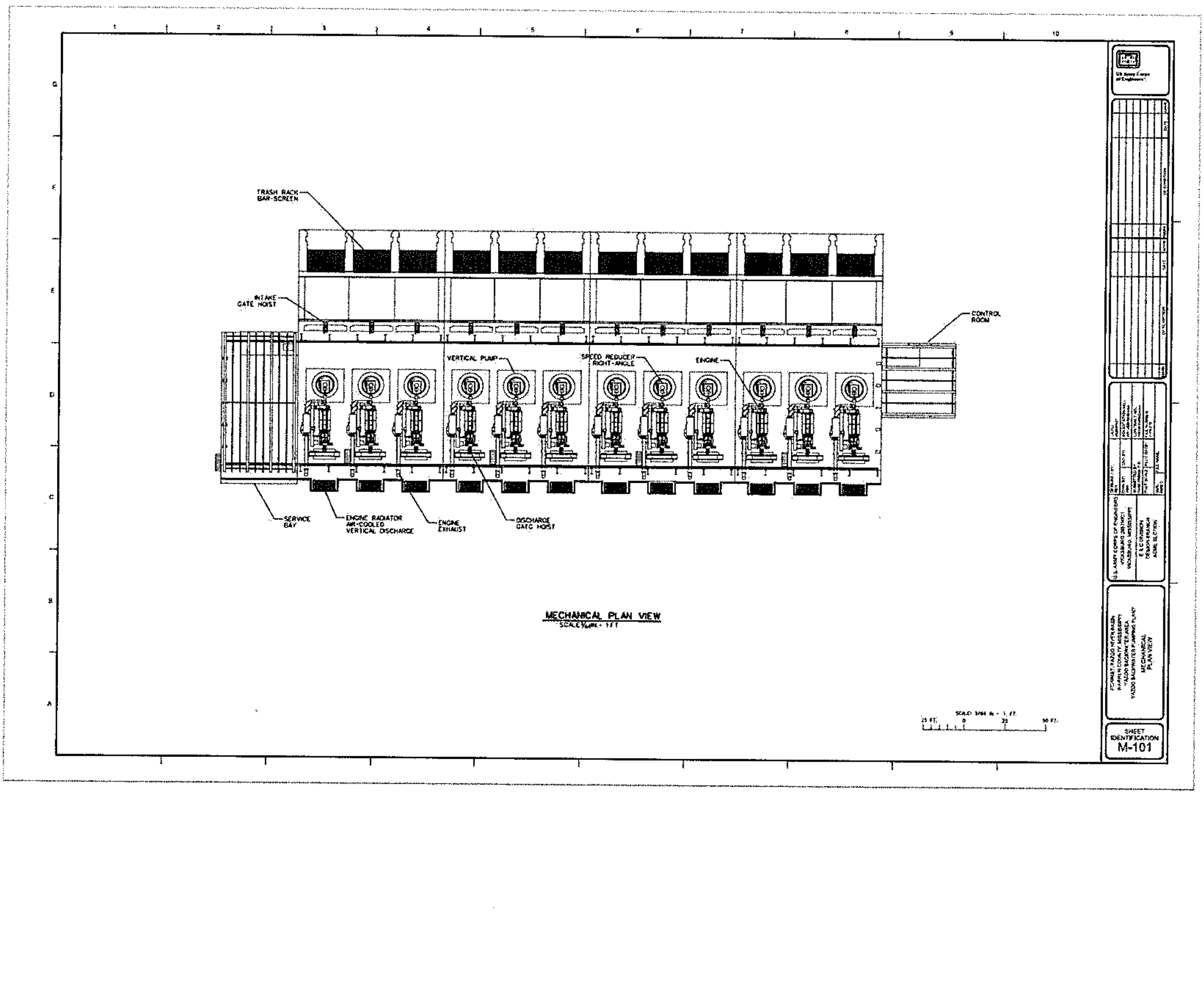


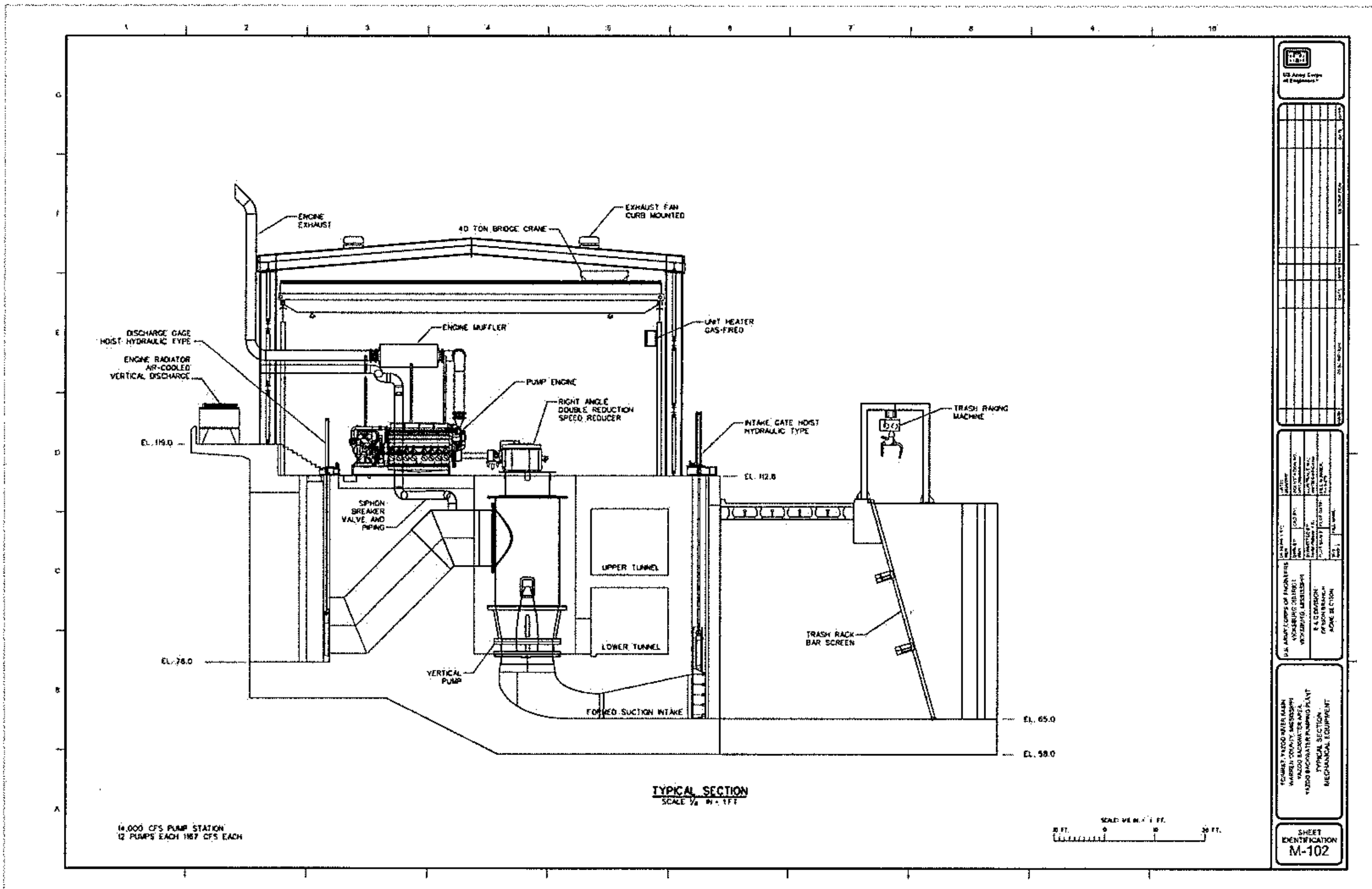
SERVICE BAY
STEM
CONTROL LINE
SUBSTRUCTURE
SHEET PILE CUTOFF PZ-22 (TYP.)
17'-0"
25'-0"
33'-0"
39'-0"
44'-7"
33'-0"
33'-0"
33'-0"
33'-0"
26'-4"
158'-4"
PLAN - DISCHARGE WINGWALL
EL. 104.0
EL. 91.25
EL. 95.5
EL. 92.75
BOTTOM OF FOOTING
BOTTOM OF KEY
KEY
SHEET PILE CUTOFF PZ-22
ELEVATION - DISCHARGE WINGWALL
10% NOT FOR CONSTRUCTION
SHEET IDENTIFICATION
S-111



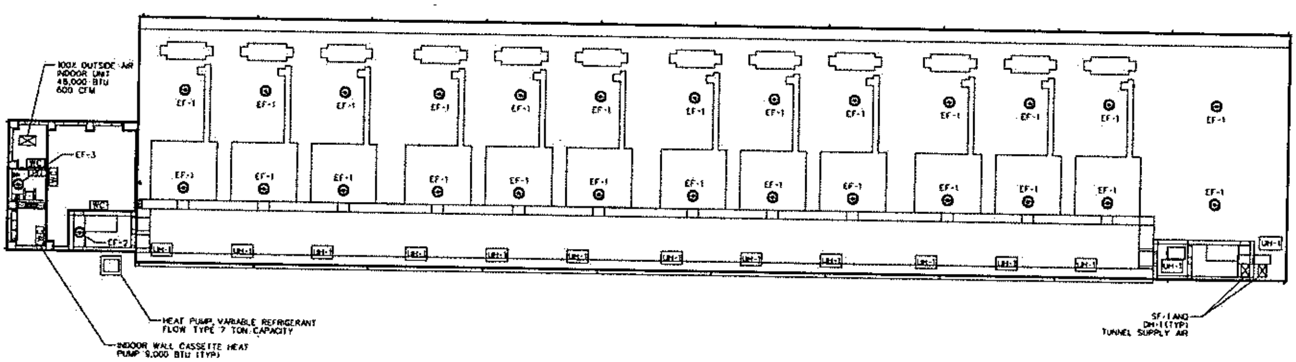
120'-0"
40'-0"
150'-0"
FLOODWALL WITH SHEET PILE CUTOFF (TYP.)
SERVICE BAY
SUBSTRUCTURE
DISCHARGE WING WALL
E1 PLAN - FLOODWALL
5'-6"
2'-0"
4'-0"
12'-0"
PZ-22 SHEET PILE
A1 SECTION - FLOODWALL
SERVICE BAY
PUMPING PLANT
PARAPET
CONTROL BUILDING
FLOODWALL (TYP.)
EL. 115.0
TYP. FOR FLOODWALL
TYP. FOR SERVICE BAY
PZ-22 SHEET PILE (TYP.)
A5 ELEVATION - FLOODWALL
US Army Corps of Engineers
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
E & C DIVISION
DESIGN BRANCH
WARREN COUNTY, MISSISSIPPI
YAZOO BACKWATER AREA
YAZOO BACKWATER PUMPING PLANT
FLOODWALL
SHEET IDENTIFICATION
S-112


PLAN - POL BUILDING
45'-6" FOUNDATION
44'-0" CMU WALL
SLOPE TYP.
13'-6¾" TYP.
STEEL COVER PLATE
RECESSED FLOOR DRAIN
REINFORCED CONCRETE FLOOR (TYP.)
12" CMU EXTERIOR (TYP.)
8" CMU ROOF PARAPET (TYP.)
REINFORCED CONCRETE SLAB (TYP.)
8" CMU INTERIOR PARTITION (TYP.)
ELEVATION - POL BUILDING
ROOF VENTILATOR (TYP.)
TAPERED RIGID INSULATION (TYP.)
MODIFIED BITUMEN ROOFING
REINFORCED CONCRETE ROOF (TYP.)
5'-0" X 7'-0" STEEL DOUBLE DOOR (TYP.)
STEEL DRAIN COVER
REINFORCED CONCRETE FLOOR AND FLOORDRAIN (TYP.)
12" CMU EXTERIOR (TYP.)
SECTION - POL BUILDING
10% NOT FOR CONSTRUCTION
POL BUILDING
SHEET IDENTIFICATION
S-113


TRASH RACK
BAR-SCREEN
INTAKE
GATE HOIST
CONTROL
ROOM
VERTICAL PUMP
SPEED REDUCER
RIGHT-ANGLE
ENGINE
SERVICE
BAY
ENGINE RADIATOR
AIR-COOLED
VERTICAL DISCHARGE
ENGINE
EXHAUST
DISCHARGE
GATE HOIST
MECHANICAL PLAN VIEW
US Army Corps
of Engineers
SHEET
IDENTIFICATION
M-101



ENGINE EXHAUST
EXHAUST FAN CURB MOUNTED
40 TON BRIDGE CRANE
DISCHARGE GAGE HOIST HYDRAULIC TYPE
ENGINE MUFFLER
UNIT HEATER GAS-FIRED
ENGINE RADIATOR AIR-COOLED VERTICAL DISCHARGE
PUMP ENGINE
RIGHT ANGLE DOUBLE REDUCTION SPEED REDUCER
INTAKE GATE HOIST HYDRAULIC TYPE
TRASH RAKING MACHINE
EL. 119.0
EL. 112.8
SIPHON BREAKER VALVE AND PIPING
UPPER TUNNEL
LOWER TUNNEL
TRASH RACK BAR SCREEN
EL. 76.0
VERTICAL PUMP
FORMED SUCTION INTAKE
EL. 65.0
EL. 58.0
TYPICAL SECTION
14,000 CFS PUMP STATION
12 PUMPS EACH 1167 CFS EACH
US Army Corps of Engineers
TYPICAL SECTION - MECHANICAL EQUIPMENT
SHEET IDENTIFICATION
M-102



HVAC PLAN VIEW

SCALE: 1/4" = 1 FT.

FAN SCHEDULE			
TAG	CFM	SP	TYPE
EF-1	13,500	0.25"	CENTRIFUGAL, ROOF TOP, CURB MOUNTED
EF-2	10,000	0.25"	CENTRIFUGAL, ROOF TOP, CURB MOUNTED
EF-3	200	0.125"	CENTRIFUGAL, ROOF TOP, CURB MOUNTED
SF-1	12,000	1.5"	CENTRIFUGAL, INLINE, DUCT MOUNTED

HEATER SCHEDULE			
TAG	MBH	SP	TYPE
UH-1	306	NAT GAS	UNIT HEATER, FAN POWERED, WALL MOUNTED
UH-2	550	NAT GAS	DUCT HEATER

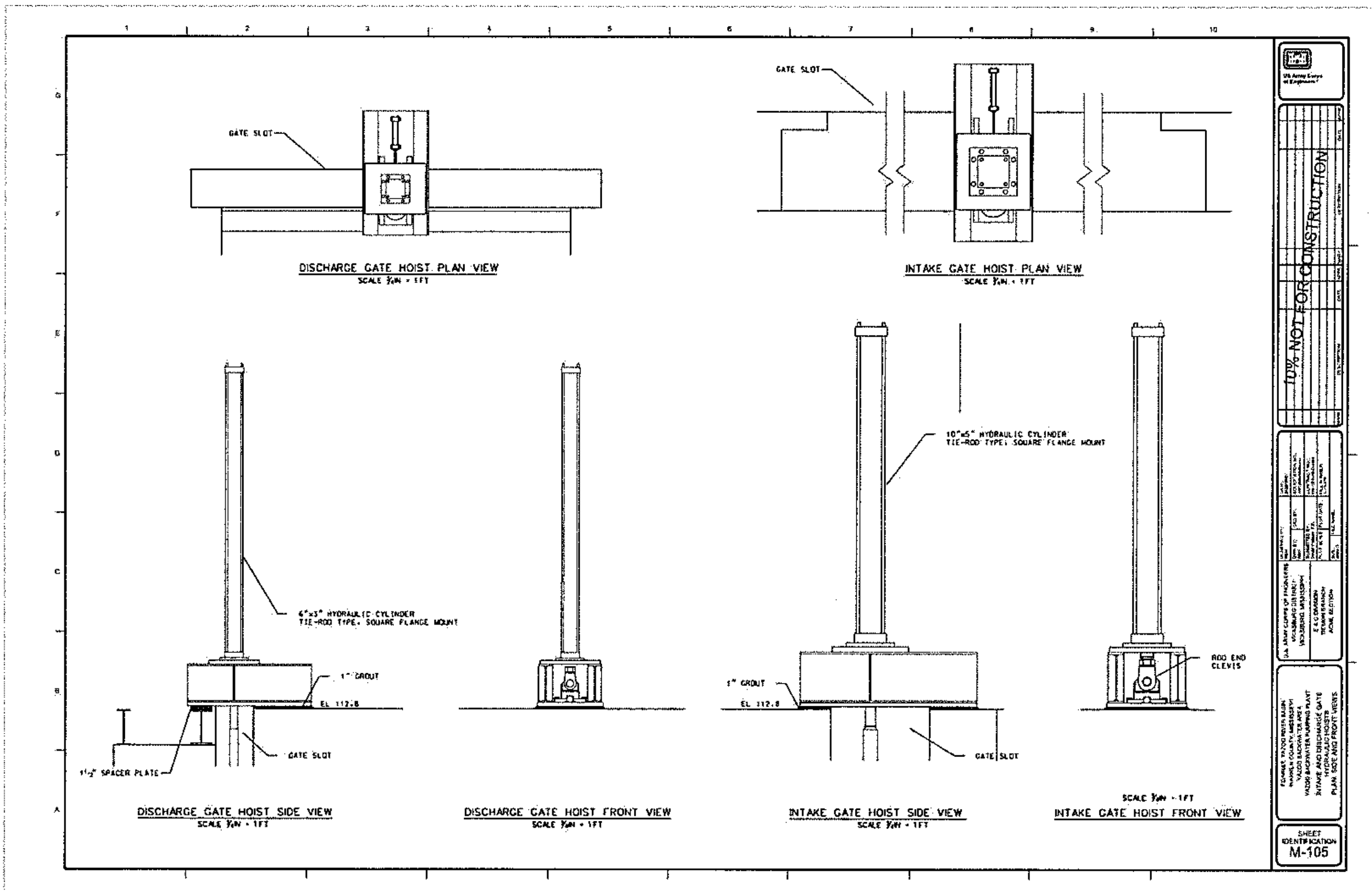
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI

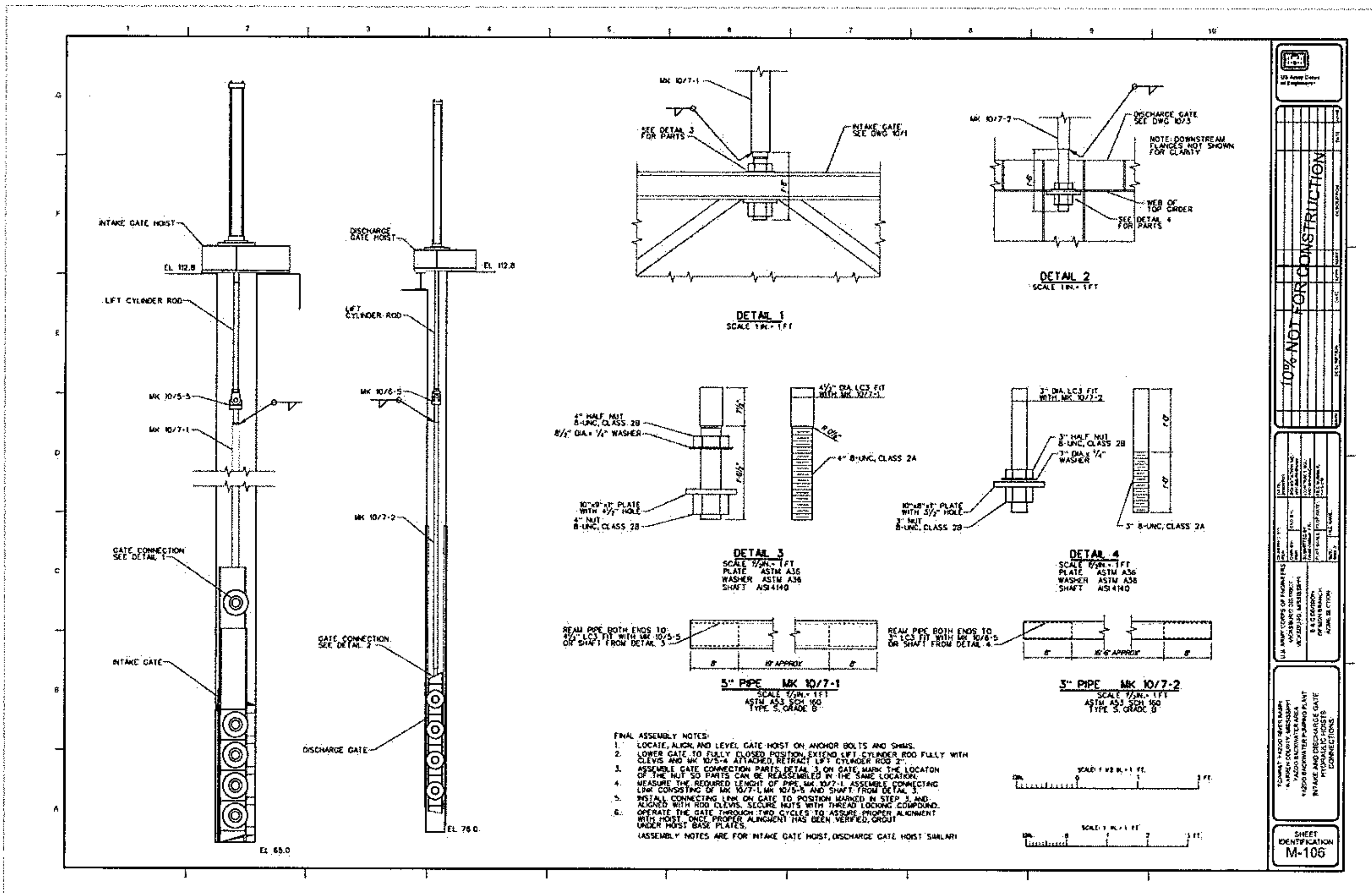
E & C DIVISION
DESIGN BRANCH
ACML SECTION

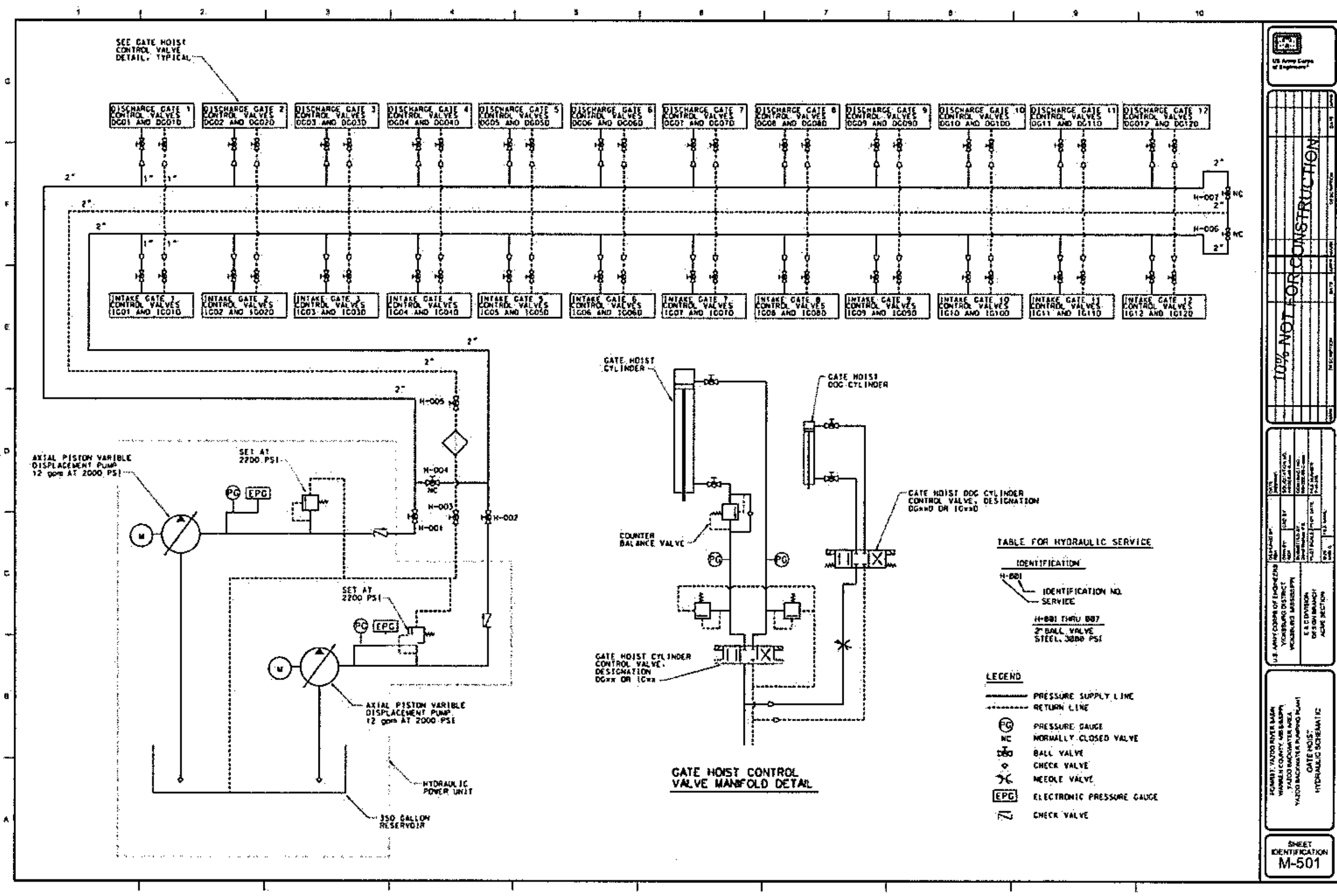
FORREST YAZOO RIVER BASIN
WARREN COUNTY, MISSISSIPPI
YAZOO BACKWATER AREA
YAZOO BACKWATER PUMPING PLANT

HVAC
PLAN VIEW

SHEET IDENTIFICATION
M-103


GATE SLOT
DISCHARGE GATE HOIST PLAN VIEW
SCALE 3/8IN = 1FT
GATE SLOT
INTAKE GATE HOIST PLAN VIEW
SCALE 3/8IN = 1FT
6"x3" HYDRAULIC CYLINDER
TIE-ROD TYPE, SQUARE FLANGE MOUNT
1" GROUT
EL 112.8
GATE SLOT
1 1/2" SPACER PLATE
10"x5" HYDRAULIC CYLINDER
TIE-ROD TYPE, SQUARE FLANGE MOUNT
1" GROUT
EL 112.8
GATE SLOT
ROD END
CLEVIS
DISCHARGE GATE HOIST SIDE VIEW
SCALE 3/8IN = 1FT
DISCHARGE GATE HOIST FRONT VIEW
SCALE 3/8IN = 1FT
INTAKE GATE HOIST SIDE VIEW
SCALE 3/8IN = 1FT
SCALE 3/8IN = 1FT
INTAKE GATE HOIST FRONT VIEW
US Army Corps of Engineers
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
E & C DIVISION
DESIGN BRANCH
ACWE SECTION
YAZOO BACKWATER PUMPING PLANT
INTAKE AND DISCHARGE GATE
HYDRAULIC HOISTS
PLAN, SIDE AND FRONT VIEWS
SHEET IDENTIFICATION
M-105


INTAKE GATE HOIST
EL 112.8
LIFT CYLINDER ROD
MK 10/5-5
MK 10/7-1
GATE CONNECTION SEE DETAIL 1
INTAKE GATE
EL 65.0
DISCHARGE GATE HOIST
LIFT CYLINDER ROD
MK 10/6-5
MK 10/7-2
GATE CONNECTION SEE DETAIL 2
DISCHARGE GATE
EL 76.0
SEE DETAIL 3 FOR PARTS
INTAKE GATE SEE DWG 10/1
DETAIL 1
SCALE 1IN.= 1FT
DISCHARGE GATE SEE DWG 10/3
NOTE: DOWNSTREAM FLANGES NOT SHOWN FOR CLARITY
WEB OF TOP GIRDER
SEE DETAIL 4 FOR PARTS
DETAIL 2
SCALE 1IN.= 1FT
4" HALF NUT 8-UNC, CLASS 2B
8 1/2" DIA.x 1/4" WASHER
10"x9"x1" PLATE WITH 4 1/2" HOLE
4" NUT 8-UNC, CLASS 2B
4 1/2" DIA. LC3 FIT WITH MK 10/7-1
4" 8-UNC, CLASS 2A
DETAIL 3
SCALE 1/2IN.= 1FT
PLATE ASTM A36
WASHER ASTM A36
SHAFT AISI 4140
3" DIA. LC3 FIT WITH MK 10/7-2
3" HALF NUT 8-UNC, CLASS 2B
7" DIA.x 1/4" WASHER
10"x8"x1" PLATE WITH 3 1/2" HOLE
3" NUT 8-UNC, CLASS 2B
3" 8-UNC, CLASS 2A
DETAIL 4
SCALE 1/2IN.= 1FT
PLATE ASTM A36
WASHER ASTM A36
SHAFT AISI 4140
REAM PIPE BOTH ENDS TO 4 1/2" LC3 FIT WITH MK 10/5-5 OR SHAFT FROM DETAIL 3
8" 19' APPROX 8"
5" PIPE MK 10/7-1
SCALE 1/2IN.= 1FT
ASTM A53 SCH 160
TYPE S, GRADE B
REAM PIPE BOTH ENDS TO 3" LC3 FIT WITH MK 10/6-5 OR SHAFT FROM DETAIL 4
8" 16'-6" APPROX 8"
3" PIPE MK 10/7-2
SCALE 1/2IN.= 1FT
ASTM A53 SCH 160
TYPE S, GRADE B
FINAL ASSEMBLY NOTES:
1. LOCATE, ALIGN, AND LEVEL GATE HOIST ON ANCHOR BOLTS AND SHIMS.
2. LOWER GATE TO FULLY CLOSED POSITION, EXTEND LIFT CYLINDER ROD FULLY WITH CLEVIS AND MK 10/5-4 ATTACHED, RETRACT LIFT CYLINDER ROD 2".
3. ASSEMBLE GATE CONNECTION PARTS, DETAIL 3, ON GATE, MARK THE LOCATON OF THE NUT SO PARTS CAN BE REASSEMBLED IN THE SAME LOCATION.
4. MEASURE THE REQUIRED LENGHT OF PIPE, MK 10/7-1. ASSEMBLE CONNECTING LINK CONSISTING OF MK 10/7-1, MK 10/5-5 AND SHAFT FROM DETAIL 3.
5. INSTALL CONNECTING LINK ON GATE TO POSITION MARKED IN STEP 3, AND ALIGNED WITH ROD CLEVIS. SECURE NUTS WITH THREAD LOCKING COMPOUND.
6. OPERATE THE GATE THROUGH TWO CYCLES TO ASSURE PROPER ALIGNMENT WITH HOIST. ONCE PROPER ALINGMENT HAS BEEN VERIFIED, GROUT UNDER HOIST BASE PLATES.
(ASSEMBLY NOTES ARE FOR INTAKE GATE HOIST, DISCHARGE GATE HOIST SIMILAR)
US Army Corps of Engineers
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
YAZOO BACKWATER AREA
YAZOO BACKWATER PUMPING PLANT
INTAKE AND DISCHARGE GATE
HYDRAULIC HOISTS
CONNECTIONS
SHEET IDENTIFICATION
M-106


SEE GATE HOIST CONTROL VALVE DETAIL, TYPICAL
DISCHARGE GATE 1 CONTROL VALVES DG01 AND DG01D
DISCHARGE GATE 2 CONTROL VALVES DG02 AND DG02D
DISCHARGE GATE 3 CONTROL VALVES DG03 AND DG03D
DISCHARGE GATE 4 CONTROL VALVES DG04 AND DG04D
DISCHARGE GATE 5 CONTROL VALVES DG05 AND DG05D
DISCHARGE GATE 6 CONTROL VALVES DG06 AND DG06D
DISCHARGE GATE 7 CONTROL VALVES DG07 AND DG07D
DISCHARGE GATE 8 CONTROL VALVES DG08 AND DG08D
DISCHARGE GATE 9 CONTROL VALVES DG09 AND DG09D
DISCHARGE GATE 10 CONTROL VALVES DG10 AND DG10D
DISCHARGE GATE 11 CONTROL VALVES DG11 AND DG11D
DISCHARGE GATE 12 CONTROL VALVES DG12 AND DG12D
INTAKE GATE 1 CONTROL VALVES IG01 AND IG01D
INTAKE GATE 2 CONTROL VALVES IG02 AND IG02D
INTAKE GATE 3 CONTROL VALVES IG03 AND IG03D
INTAKE GATE 4 CONTROL VALVES IG04 AND IG04D
INTAKE GATE 5 CONTROL VALVES IG05 AND IG05D
INTAKE GATE 6 CONTROL VALVES IG06 AND IG06D
INTAKE GATE 7 CONTROL VALVES IG07 AND IG07D
INTAKE GATE 8 CONTROL VALVES IG08 AND IG08D
INTAKE GATE 9 CONTROL VALVES IG09 AND IG09D
INTAKE GATE 10 CONTROL VALVES IG10 AND IG10D
INTAKE GATE 11 CONTROL VALVES IG11 AND IG11D
INTAKE GATE 12 CONTROL VALVES IG12 AND IG12D
H-007
H-006
H-005
H-004
H-003
H-002
H-001
NC
AXIAL PISTON VARIBLE DISPLACEMENT PUMP 12 gpm AT 2000 PSI
SET AT 2200 PSI
PG
EPG
HYDRAULIC POWER UNIT
350 GALLON RESERVOIR
GATE HOIST CYLINDER
GATE HOIST DOG CYLINDER
COUNTER BALANCE VALVE
GATE HOIST DOG CYLINDER CONTROL VALVE, DESIGNATION DGxxD OR IGxxD
GATE HOIST CYLINDER CONTROL VALVE, DESIGNATION DGxx OR IGxx
GATE HOIST CONTROL VALVE MANIFOLD DETAIL
TABLE FOR HYDRAULIC SERVICE
IDENTIFICATION
H-001
IDENTIFICATION NO.
SERVICE
H-001 THRU 007
2" BALL VALVE STEEL, 3000 PSI
LEGEND
PRESSURE SUPPLY LINE
RETURN LINE
PRESSURE GAUGE
NC NORMALLY CLOSED VALVE
BALL VALVE
CHECK VALVE
NEEDLE VALVE
EPG ELECTRONIC PRESSURE GAUGE
CHECK VALVE
US Army Corps of Engineers
10% NOT FOR CONSTRUCTION
GATE HOIST HYDRAULIC SCHEMATIC
SHEET IDENTIFICATION
M-501

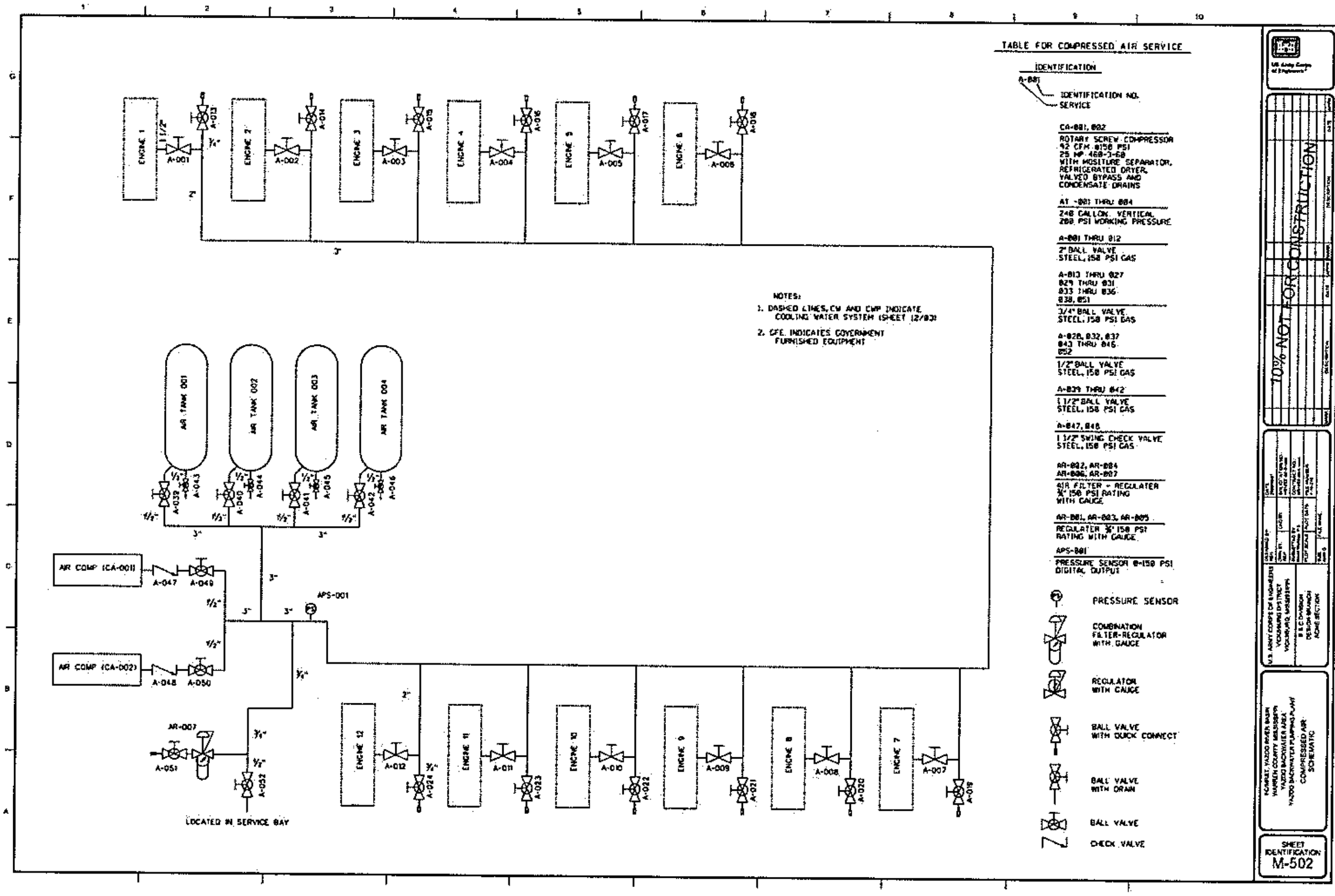
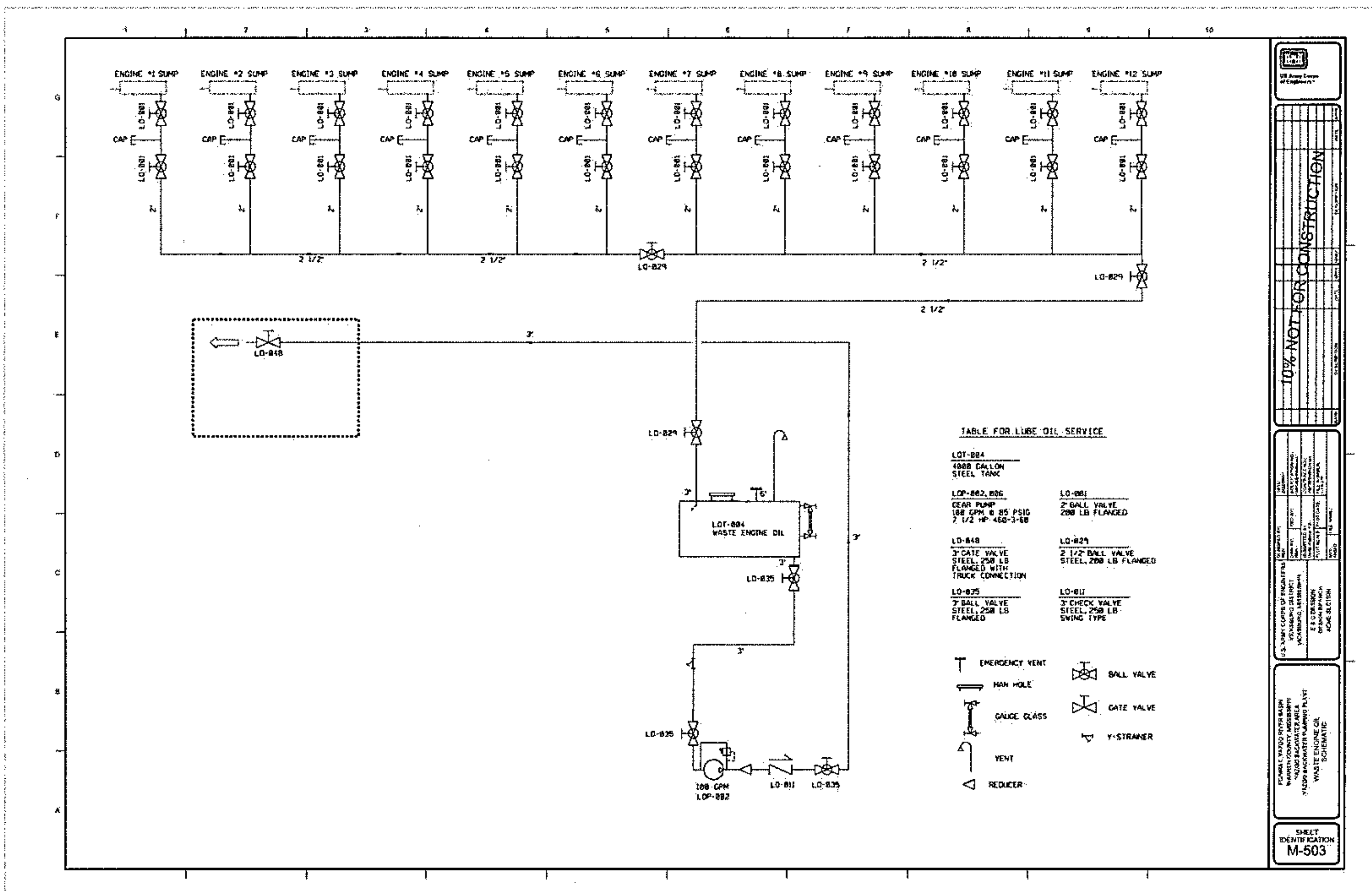
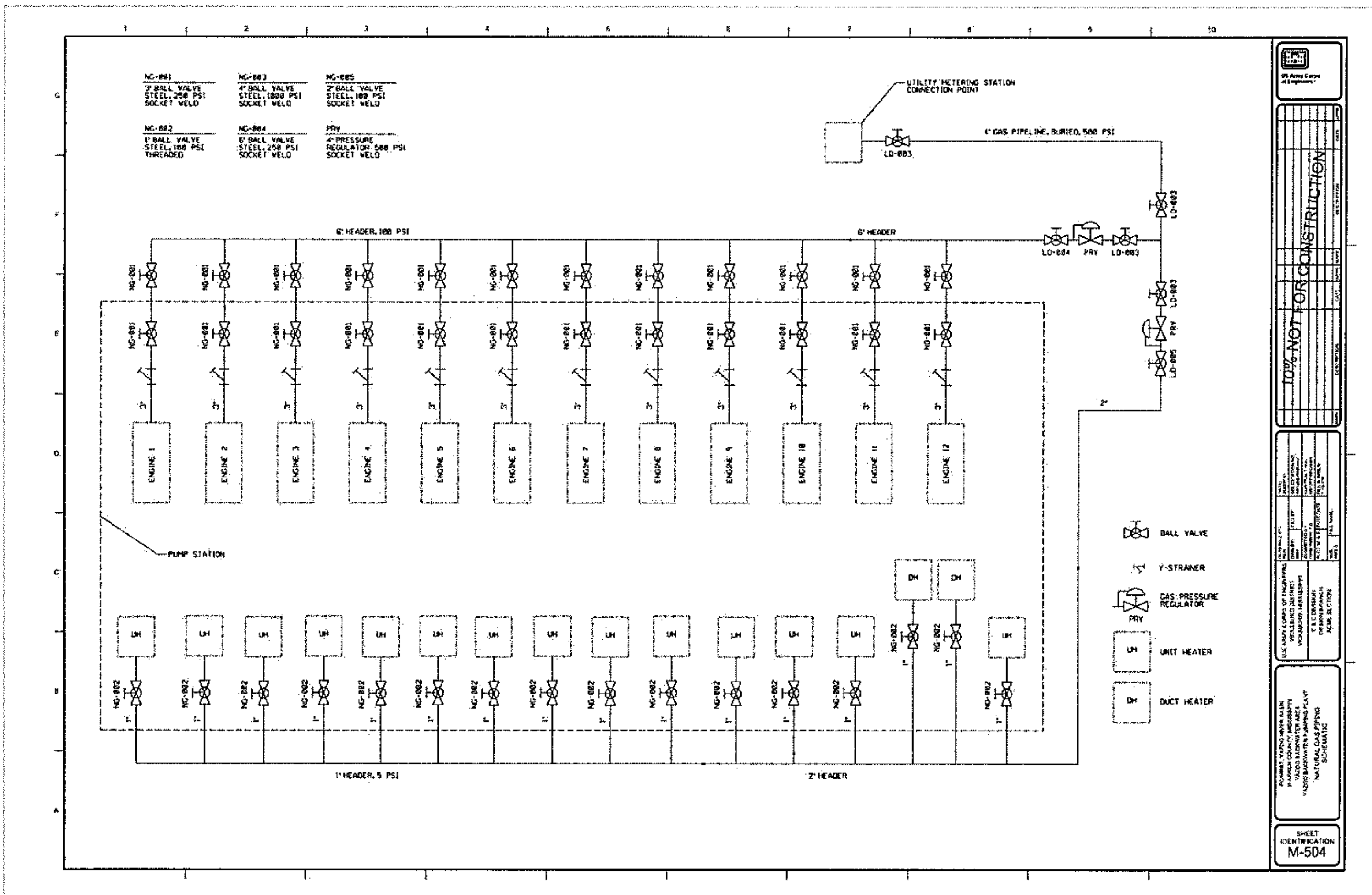
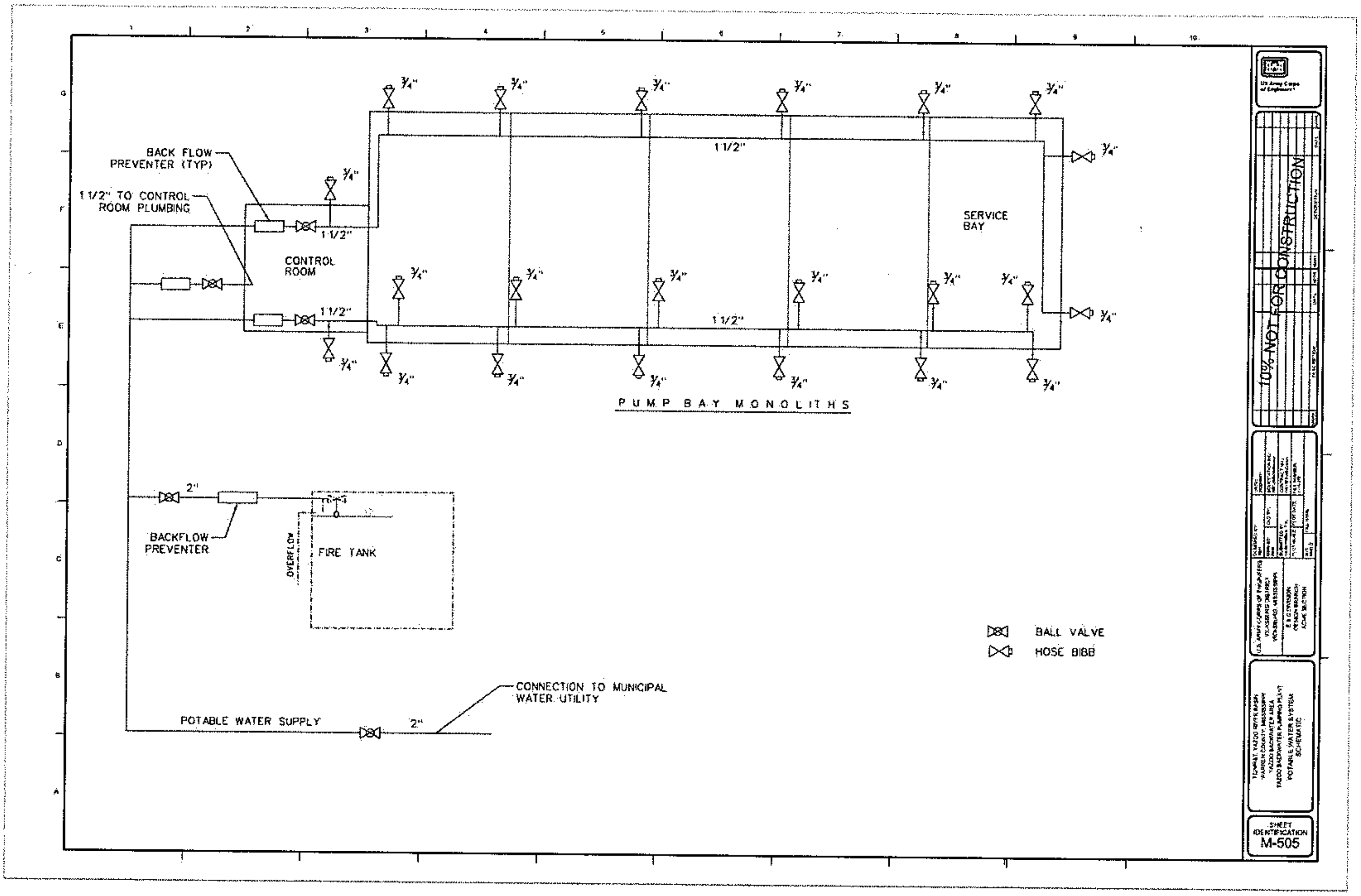


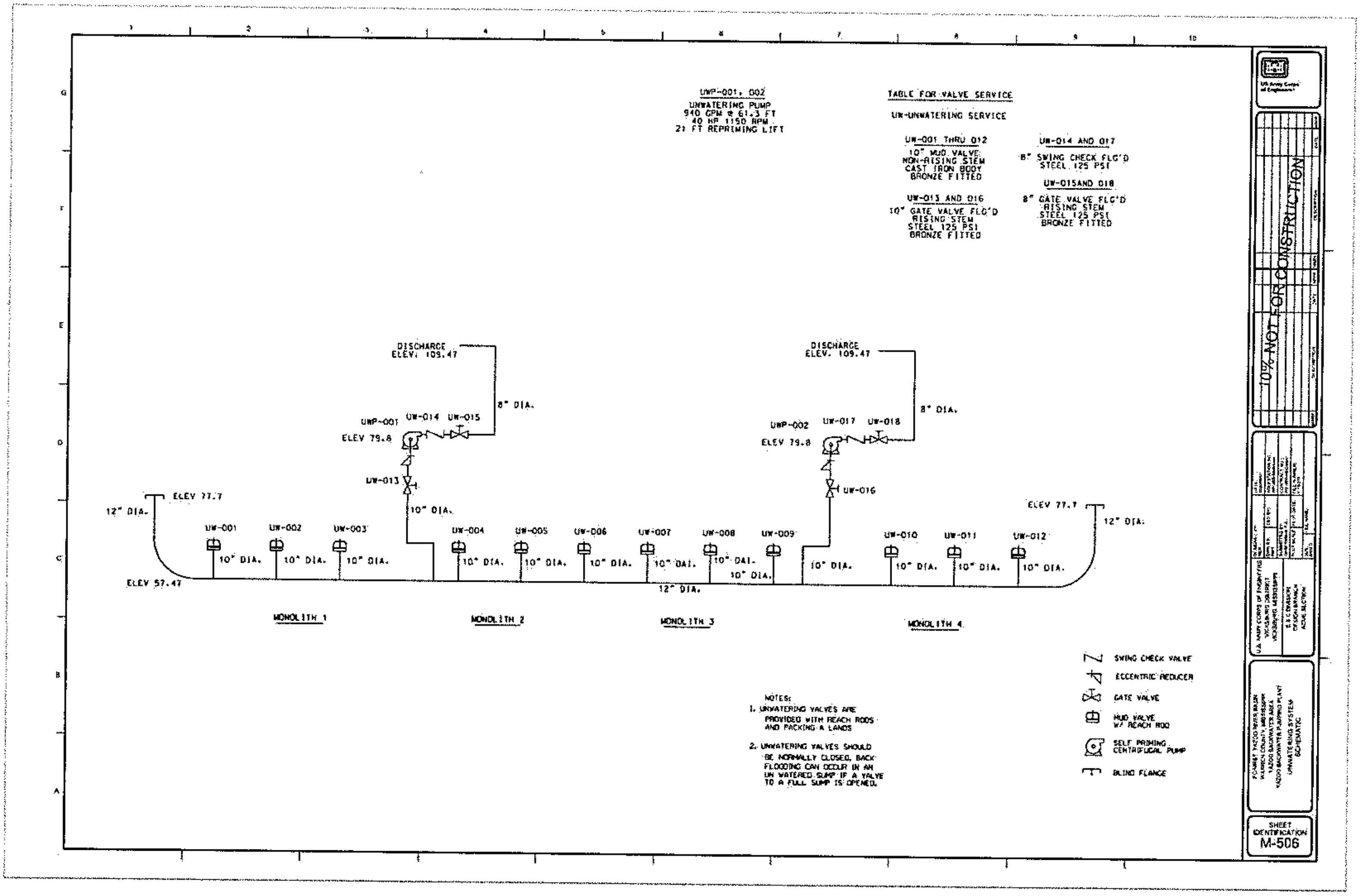
TABLE FOR COMPRESSED AIR SERVICE
IDENTIFICATION
A-001
IDENTIFICATION NO.
SERVICE
CA-001, 002
ROTARY SCREW COMPRESSOR 92 CFM @150 PSI 25 HP 460-3-60 WITH MOSITURE SEPARATOR, REFRIGERATED DRYER, VALVED BYPASS AND CONDENSATE DRAINS
AT-001 THRU 004
240 GALLON, VERTICAL 200 PSI WORKING PRESSURE
A-001 THRU 012
2" BALL VALVE STEEL, 150 PSI GAS
A-013 THRU 027
029 THRU 031
033 THRU 036
038, 051
3/4" BALL VALVE STEEL, 150 PSI GAS
A-028, 032, 037
043 THRU 046
052
1/2" BALL VALVE STEEL, 150 PSI GAS
A-039 THRU 042
1 1/2" BALL VALVE STEEL, 150 PSI GAS
A-047, 048
1 1/2" SWING CHECK VALVE STEEL, 150 PSI GAS
AR-002, AR-004
AR-006, AR-007
AIR FILTER + REGULATER 3/4" 150 PSI RATING WITH GAUGE
AR-001, AR-003, AR-005
REGULATER 3/4" 150 PSI RATING WITH GAUGE
APS-001
PRESSURE SENSOR 0-150 PSI DIGITAL OUTPUT
PRESSURE SENSOR
COMBINATION FILTER-REGULATOR WITH GAUGE
REGULATOR WITH GAUGE
BALL VALVE WITH QUICK CONNECT
BALL VALVE WITH DRAIN
BALL VALVE
CHECK VALVE
NOTES:
1. DASHED LINES, CW AND CWP INDICATE COOLING WATER SYSTEM (SHEET 12/03)
2. GFE INDICATES GOVERNMENT FURNISHED EQUIPMENT
ENGINE 1
ENGINE 2
ENGINE 3
ENGINE 4
ENGINE 5
ENGINE 6
ENGINE 7
ENGINE 8
ENGINE 9
ENGINE 10
ENGINE 11
ENGINE 12
AIR TANK 001
AIR TANK 002
AIR TANK 003
AIR TANK 004
AIR COMP (CA-001)
AIR COMP (CA-002)
APS-001
AR-007
LOCATED IN SERVICE BAY
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
COMPRESSED AIR SCHEMATIC
SHEET IDENTIFICATION
M-502


ENGINE #1 SUMP
ENGINE #2 SUMP
ENGINE #3 SUMP
ENGINE #4 SUMP
ENGINE #5 SUMP
ENGINE #6 SUMP
ENGINE #7 SUMP
ENGINE #8 SUMP
ENGINE #9 SUMP
ENGINE #10 SUMP
ENGINE #11 SUMP
ENGINE #12 SUMP
CAP
2 1/2"
LO-829
3"
LO-848
LOT-804
WASTE ENGINE OIL
LO-835
100 GPM
LOP-802
LO-811
TABLE FOR LUBE OIL SERVICE
LOT-804
4000 GALLON
STEEL TANK
LOP-802, 806
GEAR PUMP
100 GPM @ 85 PSIG
7 1/2 HP 460-3-60
LO-801
2" BALL VALVE
200 LB FLANGED
LO-848
3" GATE VALVE
STEEL, 250 LB
FLANGED WITH
TRUCK CONNECTION
LO-829
2 1/2" BALL VALVE
STEEL, 200 LB FLANGED
LO-835
3" BALL VALVE
STEEL, 250 LB
FLANGED
LO-811
3" CHECK VALVE
STEEL, 250 LB
SWING TYPE
EMERGENCY VENT
MAN HOLE
GAUGE GLASS
VENT
REDUCER
BALL VALVE
GATE VALVE
Y-STRAINER
10% NOT FOR CONSTRUCTION
WASTE ENGINE OIL
SCHEMATIC
SHEET IDENTIFICATION
M-503


NG-001
3" BALL VALVE
STEEL, 250 PSI
SOCKET WELD
NG-002
1" BALL VALVE
STEEL, 100 PSI
THREADED
NG-003
4" BALL VALVE
STEEL, 1000 PSI
SOCKET WELD
NG-004
6" BALL VALVE
STEEL, 250 PSI
SOCKET WELD
NG-005
2" BALL VALVE
STEEL, 100 PSI
SOCKET WELD
PRV
4" PRESSURE
REGULATOR 500 PSI
SOCKET WELD
UTILITY METERING STATION
CONNECTION POINT
4" GAS PIPELINE, BURIED, 500 PSI
6" HEADER, 100 PSI
6" HEADER
PUMP STATION
1" HEADER, 5 PSI
2" HEADER
BALL VALVE
Y-STRAINER
GAS PRESSURE
REGULATOR
UNIT HEATER
DUCT HEATER
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
NATURAL GAS PIPING
SCHEMATIC
SHEET
IDENTIFICATION
M-504



BACK FLOW PREVENTER (TYP)
1 1/2" TO CONTROL ROOM PLUMBING
CONTROL ROOM
SERVICE BAY
1 1/2"
¾"
PUMP BAY MONOLITHS
2"
BACKFLOW PREVENTER
OVERFLOW
FIRE TANK
BALL VALVE
HOSE BIBB
CONNECTION TO MUNICIPAL WATER UTILITY
POTABLE WATER SUPPLY
US Army Corps of Engineers
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
E & C DIVISION
DESIGN BRANCH
ACME SECTION
YAZOO BACKWATER AREA
YAZOO BACKWATER PUMPING PLANT
POTABLE WATER SYSTEM
SCHEMATIC
SHEET IDENTIFICATION
M-505



UWP-001, 002
UNWATERING PUMP
940 GPM @ 61.3 FT
40 HP 1150 RPM
21 FT REPRIMING LIFT
TABLE FOR VALVE SERVICE
UW-UNWATERING SERVICE
UW-001 THRU 012
10" MUD VALVE
NON-RISING STEM
CAST IRON BODY
BRONZE FITTED
UW-013 AND 016
10" GATE VALVE FLG'D
RISING STEM
STEEL 125 PSI
BRONZE FITTED
UW-014 AND 017
8" SWING CHECK FLG'D
STEEL 125 PSI
UW-015AND 018
8" GATE VALVE FLG'D
RISING STEM
STEEL 125 PSI
BRONZE FITTED
10% NOT FOR CONSTRUCTION
DISCHARGE
ELEV. 109.47
8" DIA.
UWP-001
UW-014
UW-015
ELEV 79.8
UW-013
10" DIA.
UWP-002
UW-017
UW-018
UW-016
ELEV 77.7
12" DIA.
UW-001
UW-002
UW-003
UW-004
UW-005
UW-006
UW-007
UW-008
UW-009
UW-010
UW-011
UW-012
10" DIA.
10" DAI.
ELEV 57.47
12" DIA.
MONOLITH 1
MONOLITH 2
MONOLITH 3
MONOLITH 4
NOTES:
1. UNWATERING VALVES ARE PROVIDED WITH REACH RODS AND PACKING A LANDS
2. UNWATERING VALVES SHOULD BE NORMALLY CLOSED, BACK FLOODING CAN OCCUR IN AN UN WATERED SUMP IF A VALVE TO A FULL SUMP IS OPENED.
SWING CHECK VALVE
ECCENTRIC REDUCER
GATE VALVE
MUD VALVE W/ REACH ROD
SELF PRIMING CENTRIFUGAL PUMP
BLIND FLANGE
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
E & C DIVISION
DESIGN BRANCH
ACME SECTION
YAZOO BACKWATER PUMPING PLANT
UNWATERING SYSTEM
SCHEMATIC
SHEET IDENTIFICATION
M-506

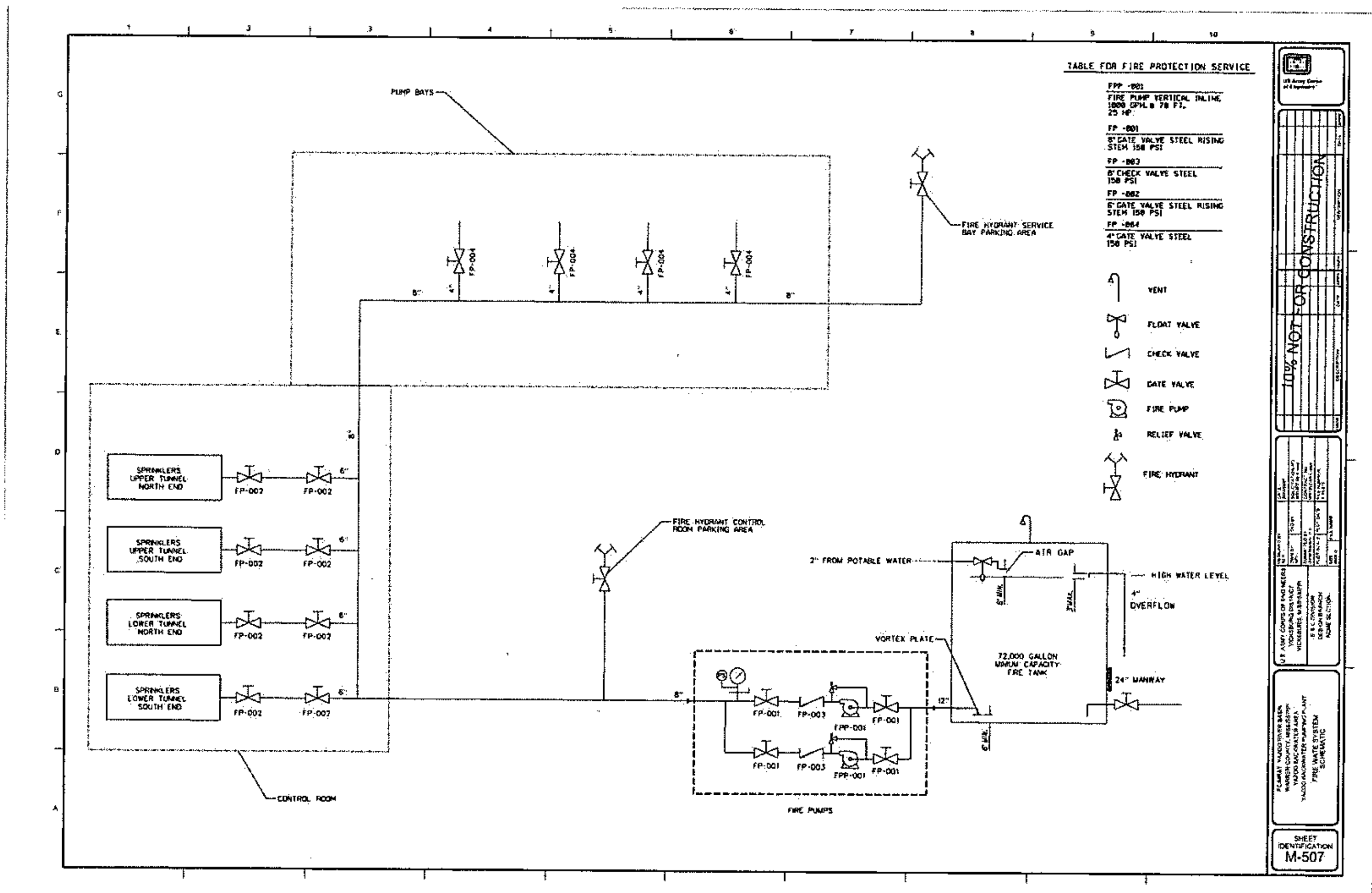


TABLE FOR FIRE PROTECTION SERVICE
FPP -001
FIRE PUMP VERTICAL INLINE 1000 GPM @ 70 FT, 25 HP
FP -001
8" GATE VALVE STEEL RISING STEM 150 PSI
FP -003
8" CHECK VALVE STEEL 150 PSI
FP -002
6" GATE VALVE STEEL RISING STEM 150 PSI
FP -004
4" GATE VALVE STEEL 150 PSI
VENT
FLOAT VALVE
CHECK VALVE
GATE VALVE
FIRE PUMP
RELIEF VALVE
FIRE HYDRANT
PUMP BAYS
FIRE HYDRANT SERVICE BAY PARKING AREA
FP-004
SPRINKLERS UPPER TUNNEL NORTH END
SPRINKLERS UPPER TUNNEL SOUTH END
SPRINKLERS LOWER TUNNEL NORTH END
SPRINKLERS LOWER TUNNEL SOUTH END
FP-002
FIRE HYDRANT CONTROL ROOM PARKING AREA
2" FROM POTABLE WATER
AIR GAP
HIGH WATER LEVEL
4" OVERFLOW
VORTEX PLATE
72,000 GALLON MINIMUM CAPACITY FIRE TANK
24" MANWAY
FP-001
FP-003
FPP-001
CONTROL ROOM
FIRE PUMPS
10% NOT FOR CONSTRUCTION
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
FIRE WATE SYSTEM SCHEMATIC
SHEET IDENTIFICATION
M-507

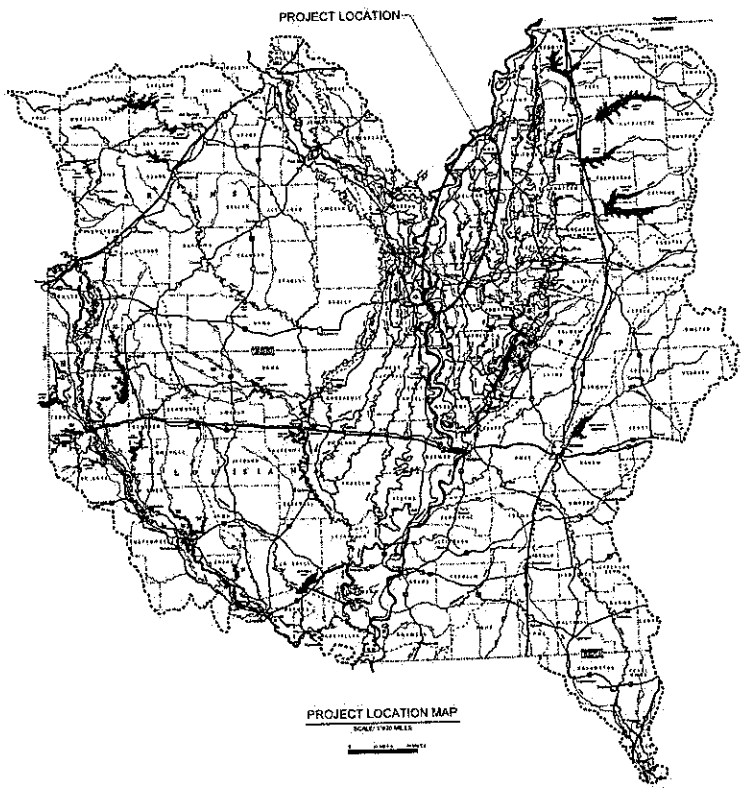
Attachment B

US Army Corps of Engineers®
Vicksburg District

FC/MR&T, YAZOO RIVER BASIN
COAHOMA, BOLIVAR, & WASHINGTON COUNTIES, MS
YAZOO BACKWATER AREA
YAZOO BACKWATER WELL SITES

Solicitation: W912EE-XX-X-XXXX
Contract: W912EE-XX-X-XXXX
August 2020

SHEET IDENTIFICATION
G-001



PROJECT LOCATION MAP

INDEX TO DRAWINGS

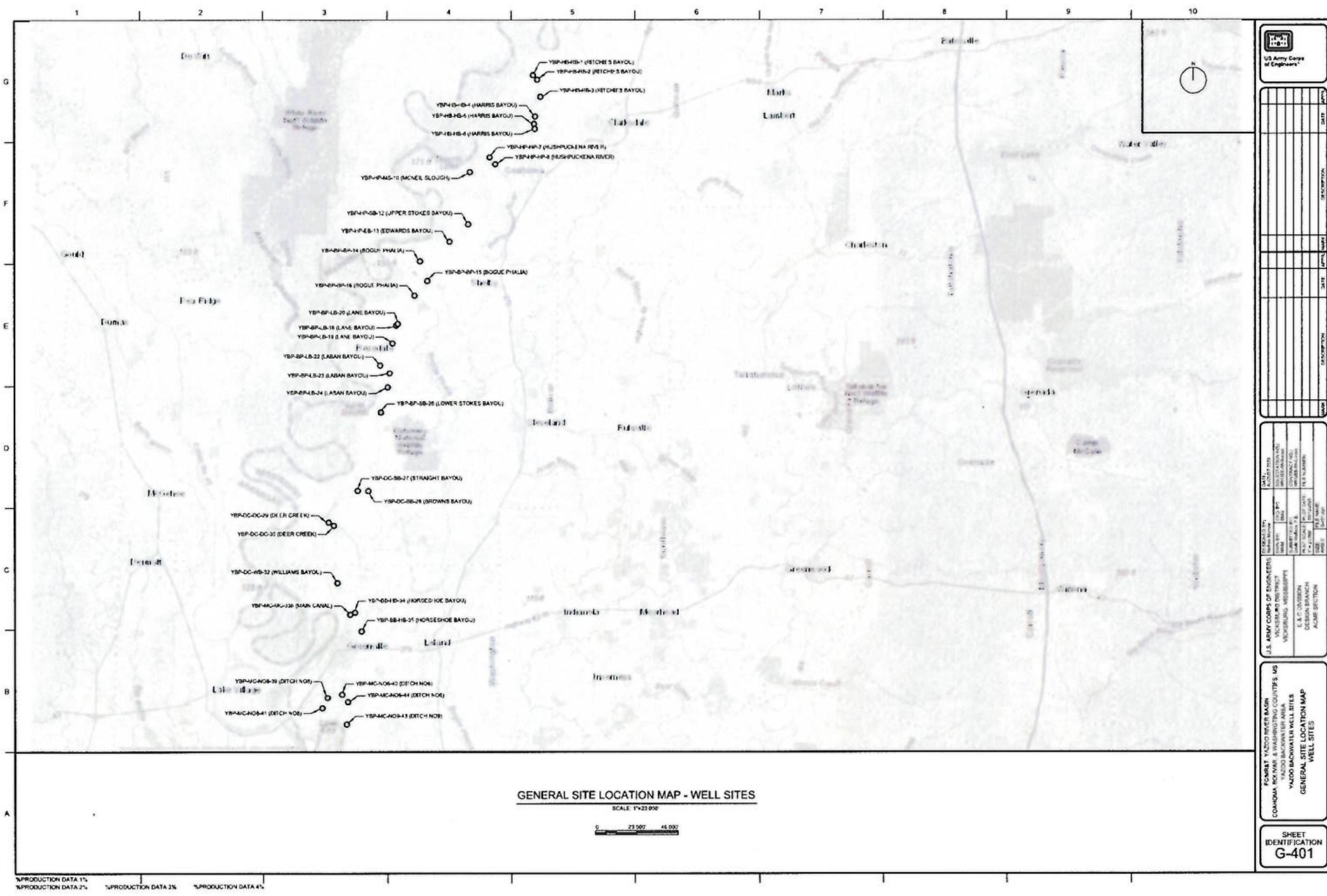
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI

E.&C. DIVISION
DESIGN BRANCH
ASAP SECTION

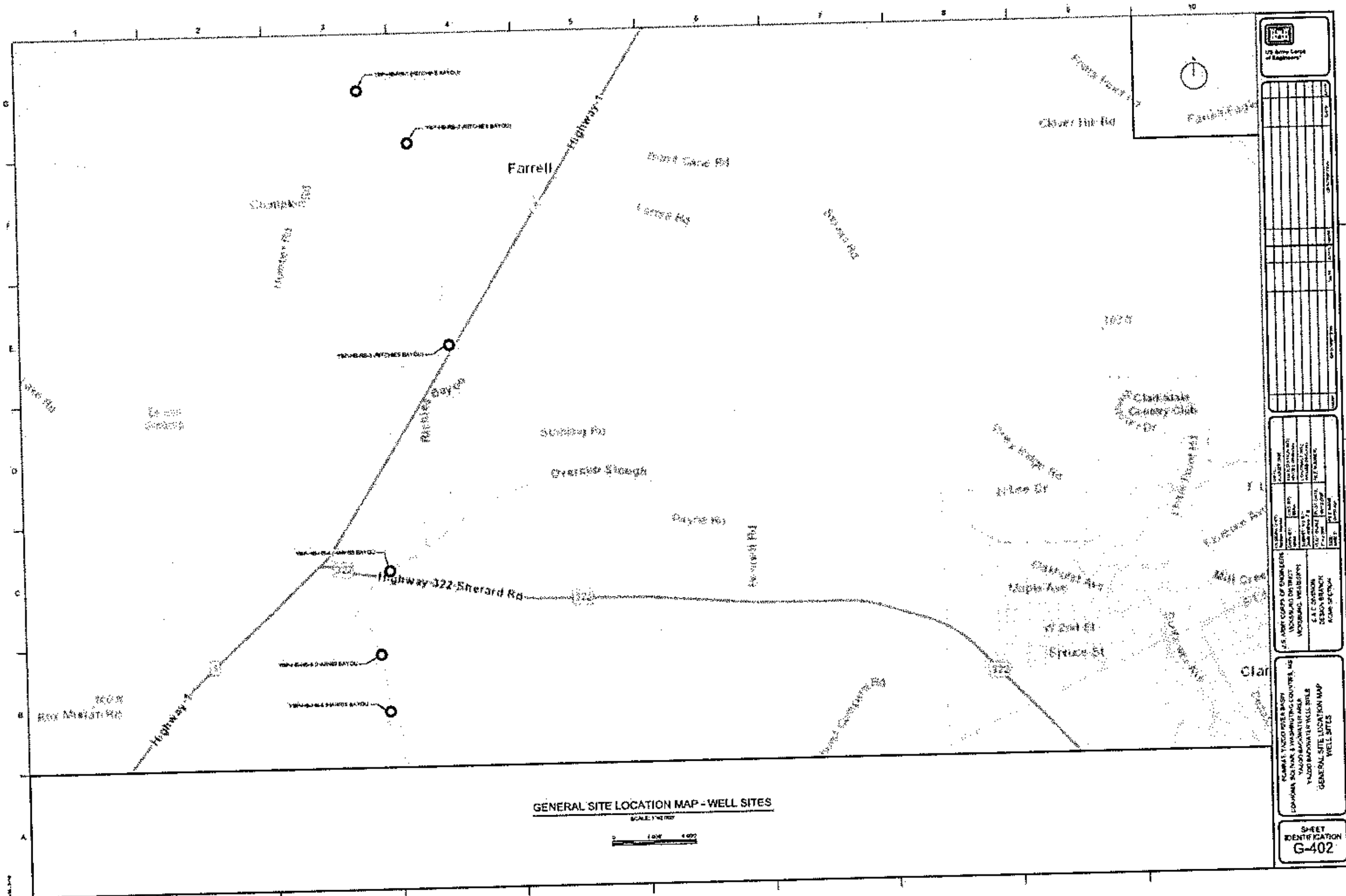
YAZOO RIVER BASIN
COAHOMA, BOLIVAR, & WASHINGTON COUNTIES, MS
YAZOO BACKWATER AREA
YAZOO BACKWATER WELL SITES

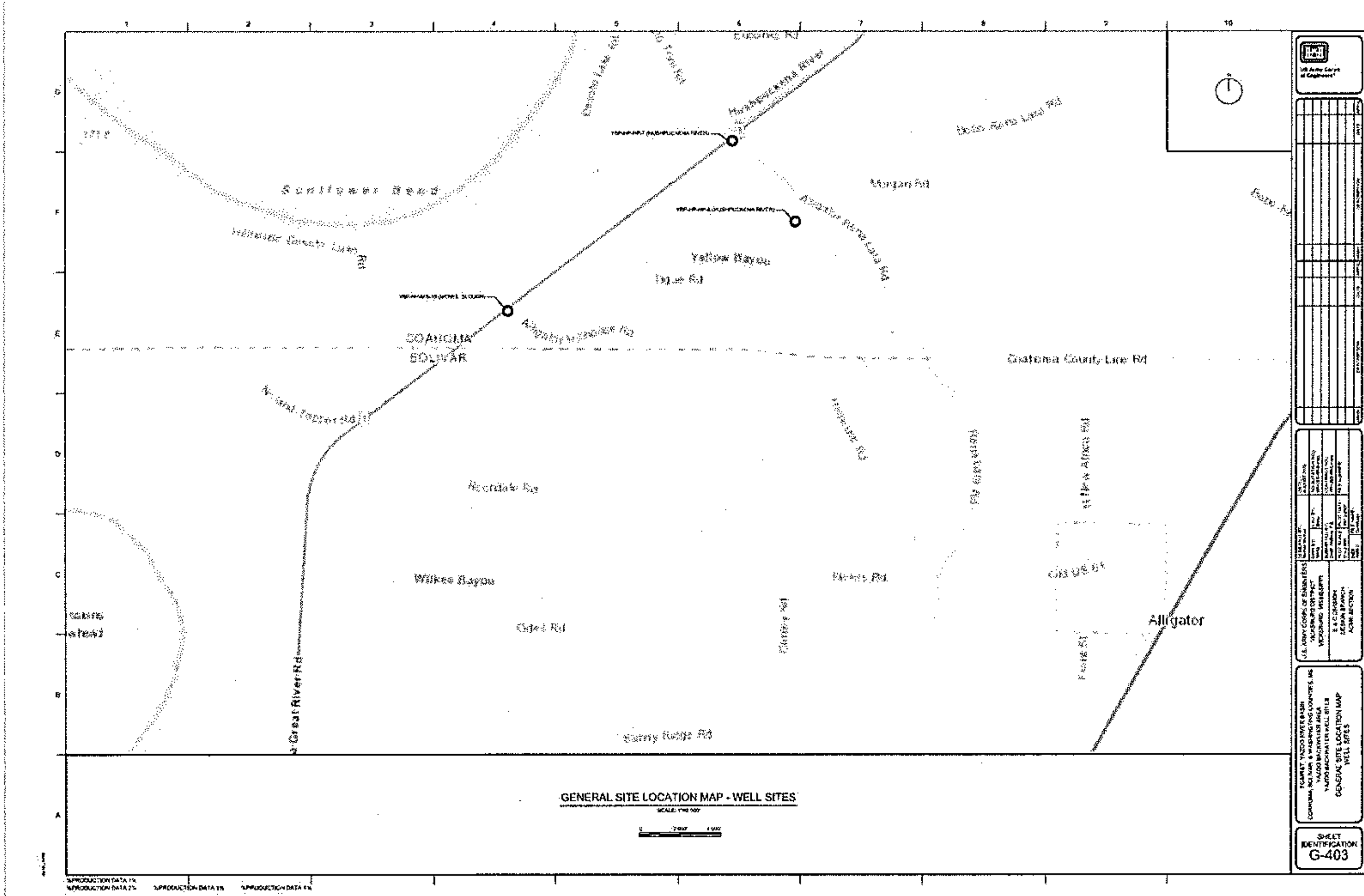
PROJECT LOCATION MAP
AND INDEX TO DRAWINGS

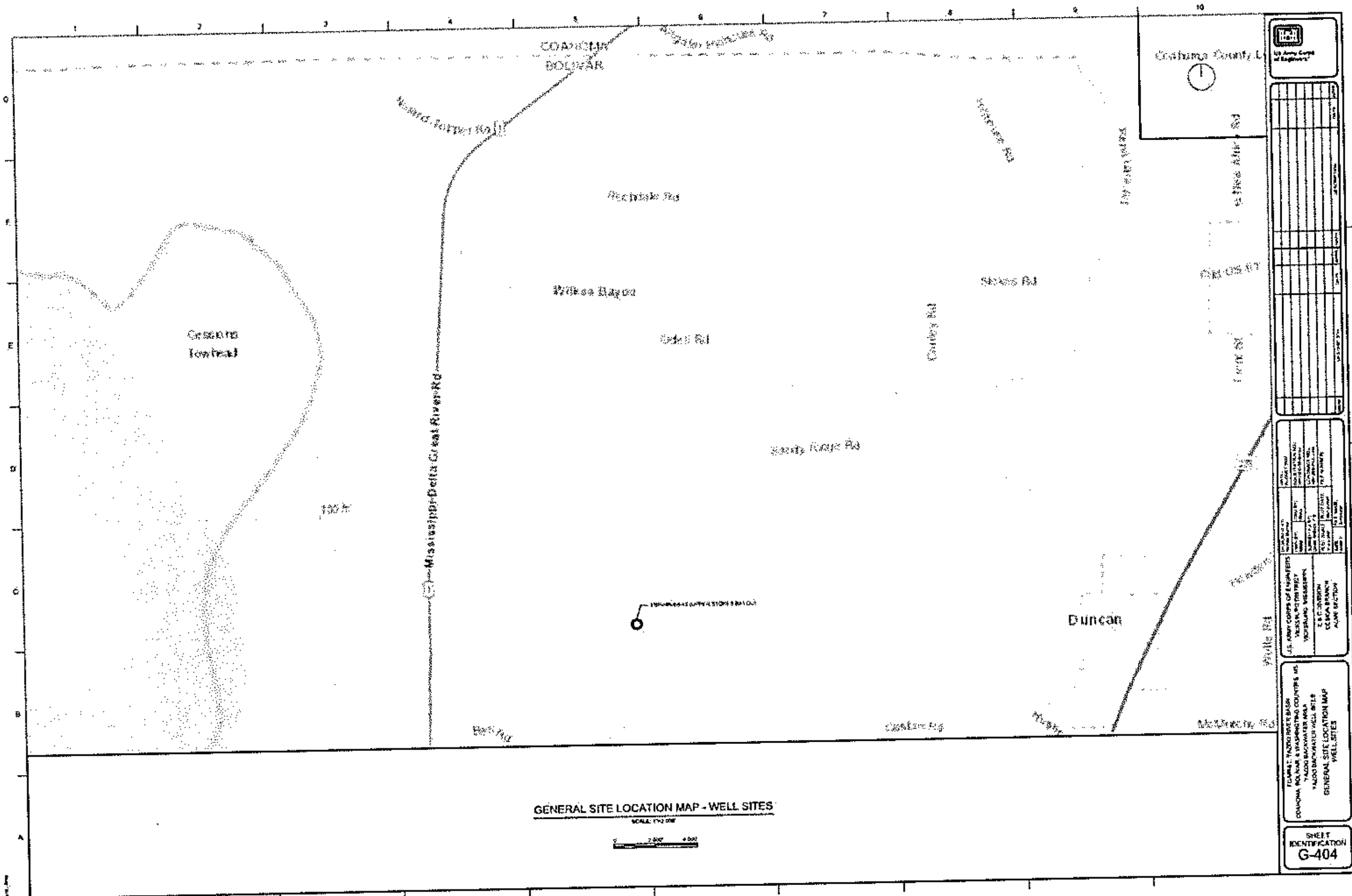
SHEET IDENTIFICATION
G-002

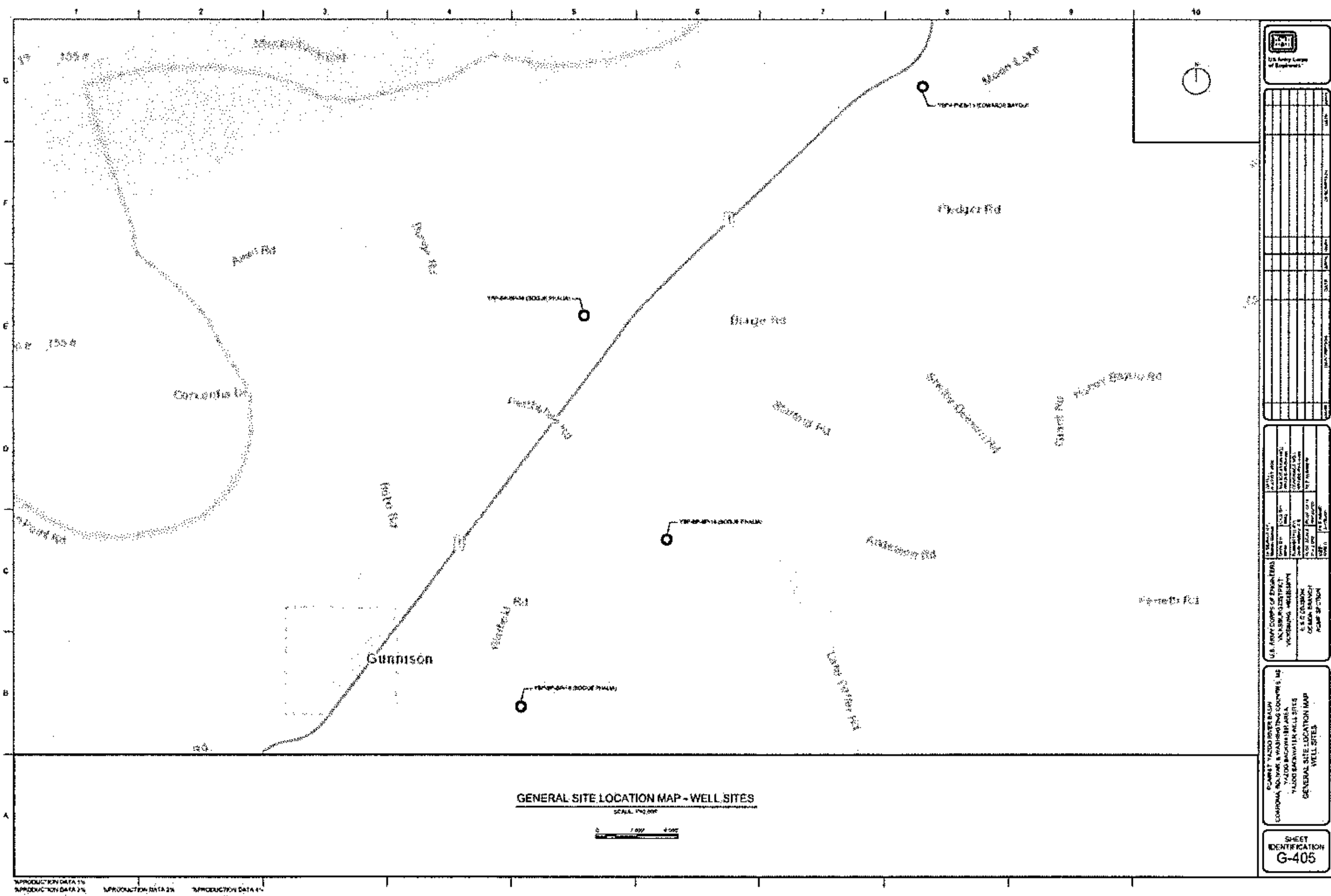


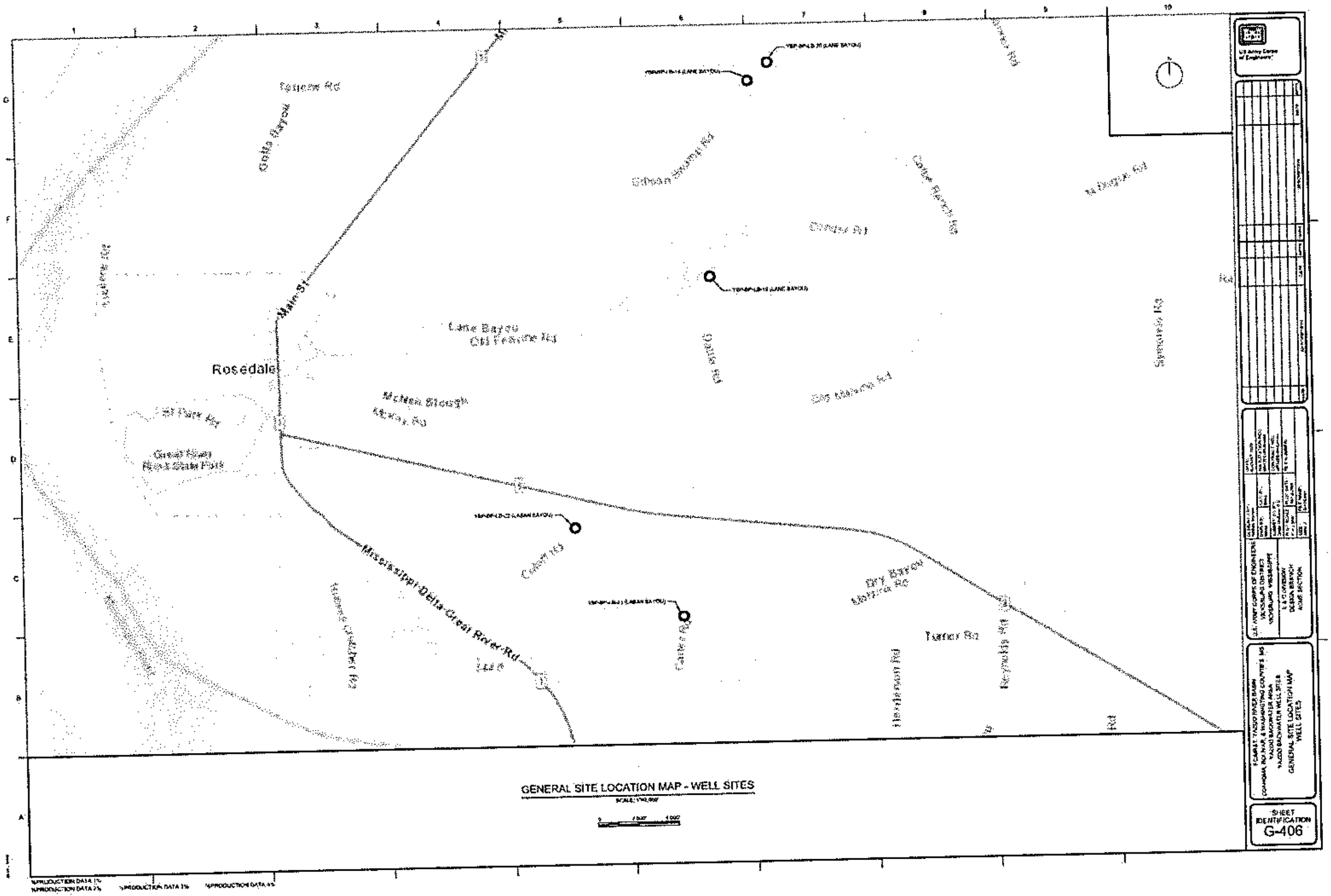
YBP-HB-RB-1 (RITCHIE S BAYOU)
YBP-HB-RB-2 (RITCHIE S BAYOU)
YBP-HB-RB-3 (RITCHIE S BAYOU)
YBP-HB-HB-4 (HARRIS BAYOU)
YBP-HB-HB-5 (HARRIS BAYOU)
YBP-HB-HB-6 (HARRIS BAYOU)
YBP-HP-HP-7 (HUSHPUCKENA RIVER)
YBP-HP-HP-8 (HUSHPUCKENA RIVER)
YBP-HP-MS-10 (MCNEIL SLOUGH)
YBP-HP-SB-12 (UPPER STOKES BAYOU)
YBP-HP-EB-13 (EDWARDS BAYOU)
YBP-BP-BP-14 (BOGUE PHALIA)
YBP-BP-BP-15 (BOGUE PHALIA)
YBP-BP-BP-16 (BOGUE PHALIA)
YBP-BP-LB-20 (LANE BAYOU)
YBP-BP-LB-18 (LANE BAYOU)
YBP-BP-LB-19 (LANE BAYOU)
YBP-BP-LB-22 (LABAN BAYOU)
YBP-BP-LB-23 (LABAN BAYOU)
YBP-BP-LB-24 (LABAN BAYOU)
YBP-BP-SB-26 (LOWER STOKES BAYOU)
YBP-DC-SB-27 (STRAIGHT BAYOU)
YBP-DC-BB-28 (BROWNS BAYOU)
YBP-DC-DC-29 (DEER CREEK)
YBP-DC-DC-30 (DEER CREEK)
YBP-DC-WB-32 (WILLIAMS BAYOU)
YBP-BB-HB-34 (HORSESHOE BAYOU)
YBP-BB-HB-35 (HORSESHOE BAYOU)
YBP-MC-NO6-39 (DITCH NO6)
YBP-MC-NO6-40 (DITCH NO6)
YBP-MC-NO6-44 (DITCH NO6)
YBP-MC-NO6-41 (DITCH NO6)
YBP-MC-NO9-43 (DITCH NO9)
Batesville
Marks
Lambert
Clarksdale
Charleston
Cleveland
Ruleville
Greenwood
Indianola
Moorhead
Leland
Greenville
Lake Village
Dermott
McGehee
Dumas
US Army Corps of Engineers
GENERAL SITE LOCATION MAP - WELL SITES
SHEET IDENTIFICATION
G-401


Farrell
Highway 1
Highway 322 Sherard Rd
Clover Hill Rd
Spruce St
GENERAL SITE LOCATION MAP - WELL SITES
SHEET IDENTIFICATION
G-402

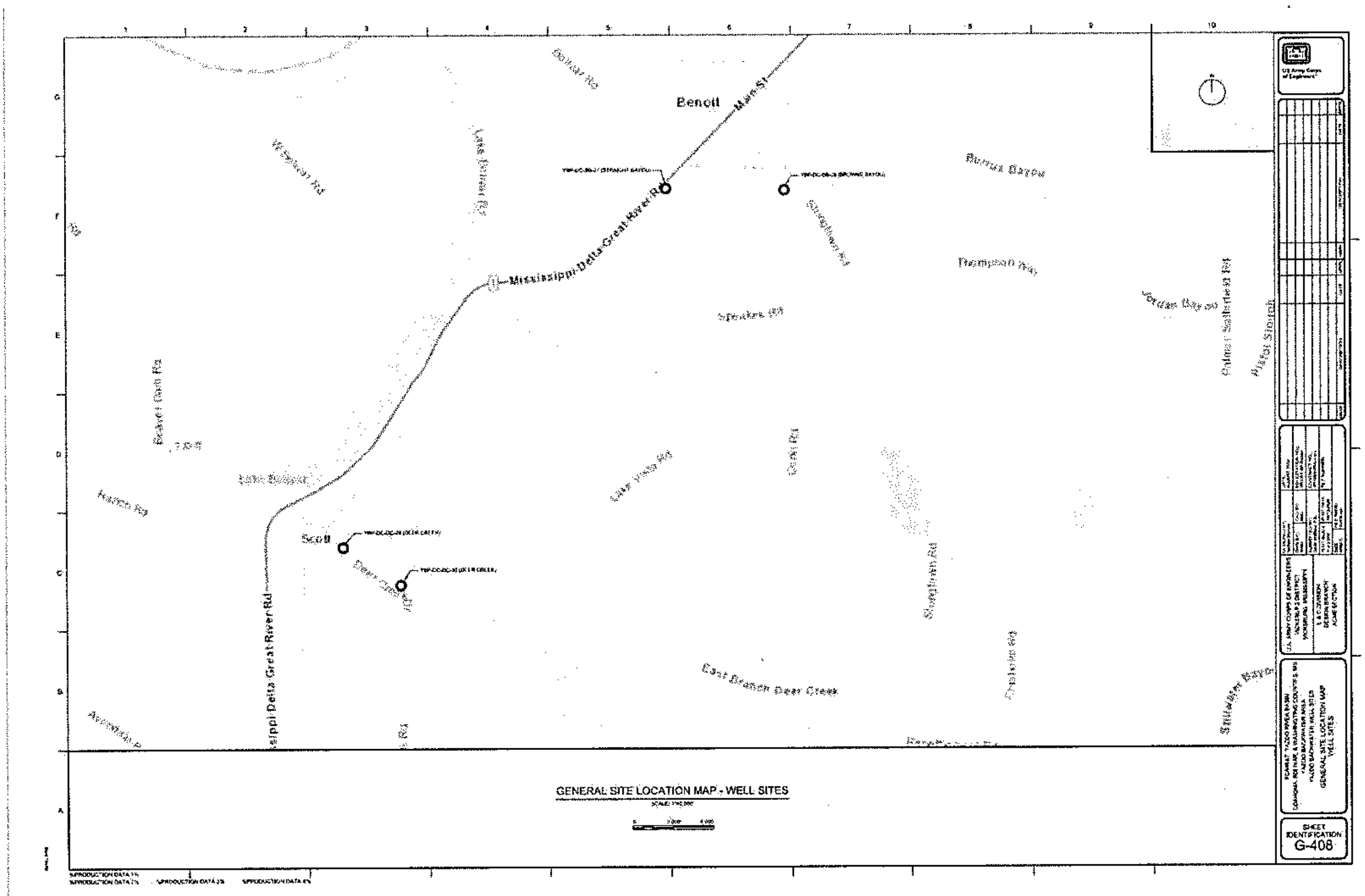

Hushpuckena River
Morgan Rd
Yellow Bayou
Tigue Rd
COAHOMA
BOLIVAR
Coahoma County Line Rd
Wilkes Bayou
Great River Rd
Alligator
GENERAL SITE LOCATION MAP - WELL SITES
SHEET IDENTIFICATION
G-403


COAHOMA
BOLIVAR
Coahoma County, L
US Army Corps of Engineers
Mississippi Delta Great River Rd
Wilkes Bayou
Odell Rd
Stokes Rd
Gessoria Towhead
Sandy Ridge Rd
Duncan
Bell Rd
McWherter Rd
GENERAL SITE LOCATION MAP - WELL SITES
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
GENERAL SITE LOCATION MAP
WELL SITES
SHEET IDENTIFICATION
G-404

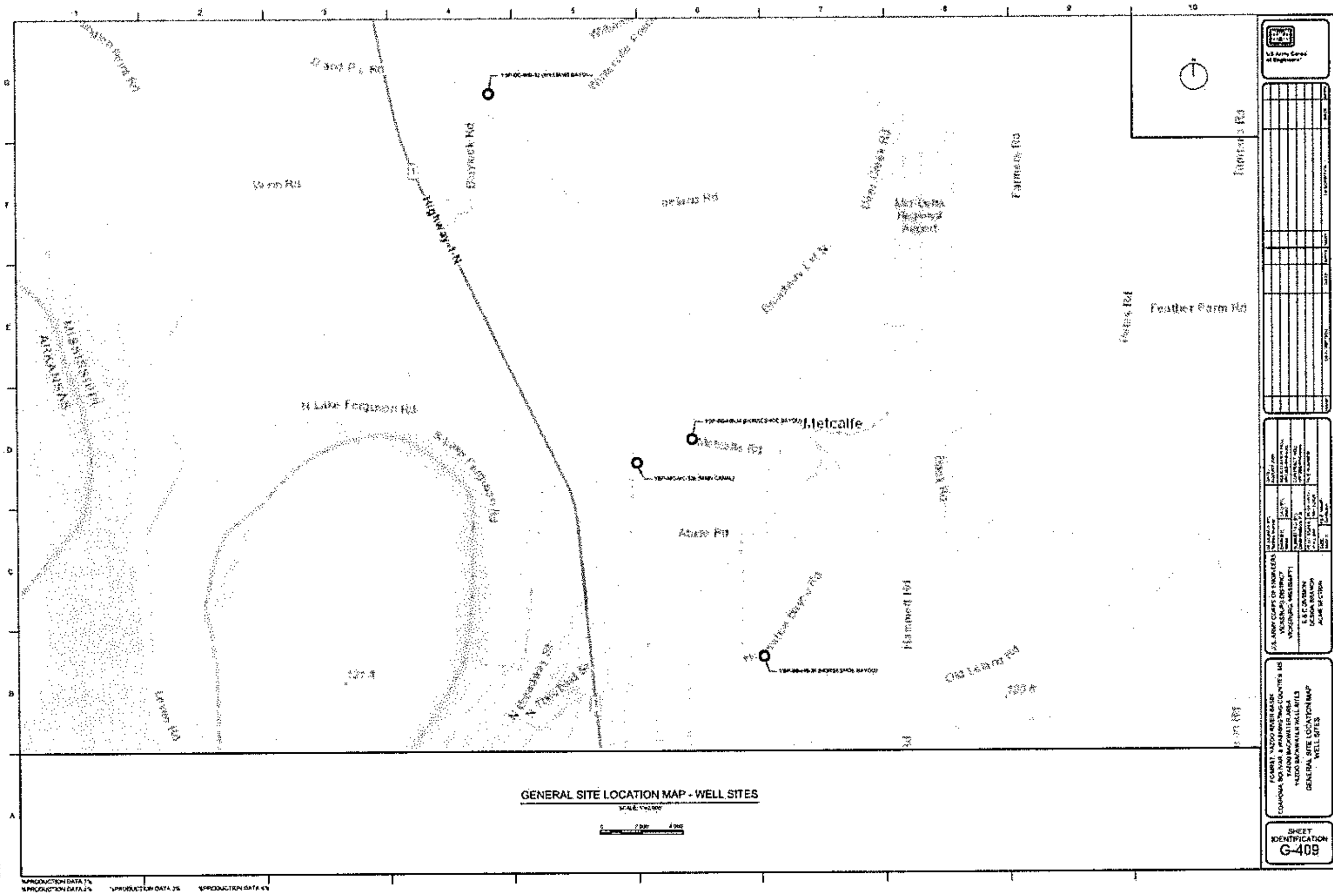

Gunnison
Anderson Rd
Concordia Dr
GENERAL SITE LOCATION MAP - WELL SITES
SHEET IDENTIFICATION
G-405


Rosedale
Main St
Lane Bayou
McNeil Slough
Mississippi-Delta-Great River Rd
Dry Bayou
GENERAL SITE LOCATION MAP - WELL SITES
SHEET IDENTIFICATION
G-406

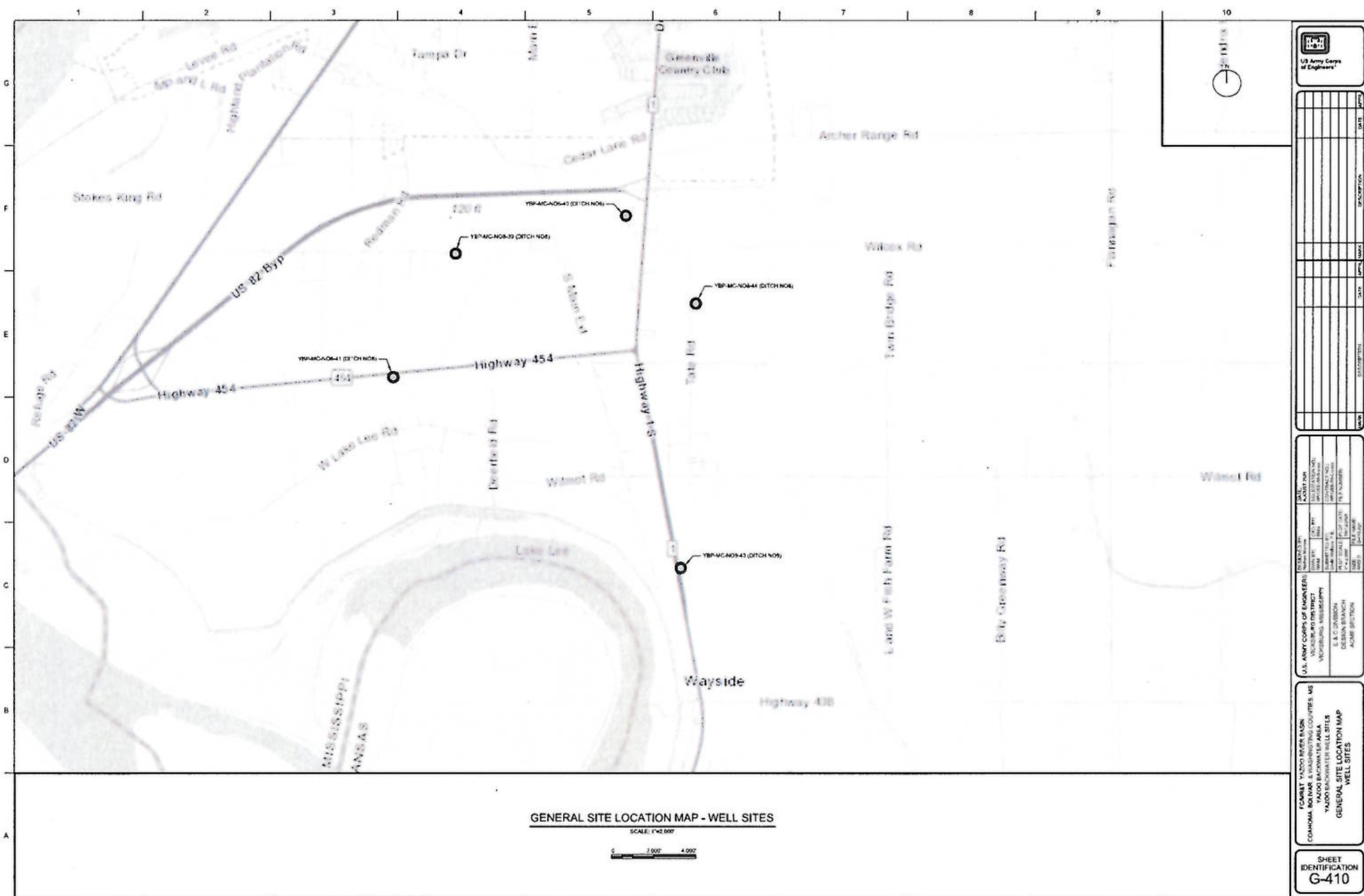

GENERAL SITE LOCATION MAP - WELL SITES
Beulah
Lake Beulah
Mississippi-Delta Great River Rd
River Rd
Turner Rd
Henderson Bayou
Bexian Rd
Thousand Yard Bayou
Walton Rd
Mound City Rd
Newman Rd
Half Moon Bayou
Peterson Rd
Frazier Rd
Logan Rd
Dahomey National Wildlife Refuge
Stokes Bayou
Spicewood Rd
YBP-BP-LB-24 (LABAN BAYOU)
YBP-BP-SB-26 (LOWER STOKES BAYOU)
US Army Corps of Engineers
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
GENERAL SITE LOCATION MAP
WELL SITES
SHEET IDENTIFICATION
G-407



Benoit
Main St
Mississippi Delta Great River Rd
Burrus Bayou
Thompson Rd
Jordan Bayou
Scott
Deer Creek
Lake Vista Rd
East Branch Deer Creek
Stillwater Bayou
Beaver Dam Rd
Harris Rd
GENERAL SITE LOCATION MAP - WELL SITES
SHEET IDENTIFICATION
G-408



Metcalfe
Feather Farm Rd
N Lake Ferguson Rd
Abide Rd
Hammett Rd
Old Leland Rd
GENERAL SITE LOCATION MAP - WELL SITES
SHEET IDENTIFICATION
G-409



Tampa Dr
Greenville Country Club
Levee Rd
Highland Plantation Rd
Archer Range Rd
Cedar Lane Rd
Stokes King Rd
Redman Rd
US 82 Byp
Wilcox Rd
S Main Ext
Tate Rd
Twin Bridge Rd
Highway 454
Highway 1-S
W Lake Lee Rd
Deerfield Rd
Wilmot Rd
Lake Lee
Wayside
Highway 438
MISSISSIPPI
ARKANSAS
US Army Corps of Engineers
GENERAL SITE LOCATION MAP - WELL SITES
SHEET IDENTIFICATION
G-410


YBP-HB-RB-1 (RITCHIES BAYOU)
YBP-HB-RB-2 (RITCHIES BAYOU)
YBP-HB-RB-3 (RITCHIES BAYOU)
YBP-HB-HB-4 (HARRIS BAYOU)
AERIAL PHOTOGRAPH - WELL SITES
US Army Corps of Engineers
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
E & C DIVISION
DESIGN BRANCH
ACMF SECTION
FC&MR&T YAZOO RIVER BASIN
COAHOMA, BOLIVAR & WASHINGTON COUNTIES, MS
YAZOO BACKWATER AREA
YAZOO BACKWATER WELL SITES
AERIAL PHOTOGRAPH
WELL SITES
SHEET IDENTIFICATION
G-411

AERIAL PHOTOGRAPH - WELL SITES

SCALE: 1"=2,000'

0 2,000' 4,000'

SHEET IDENTIFICATION
G-412

AERIAL PHOTOGRAPH - WELL SITES

SCALE: 1"=2,000'

0 2,000' 4,000'

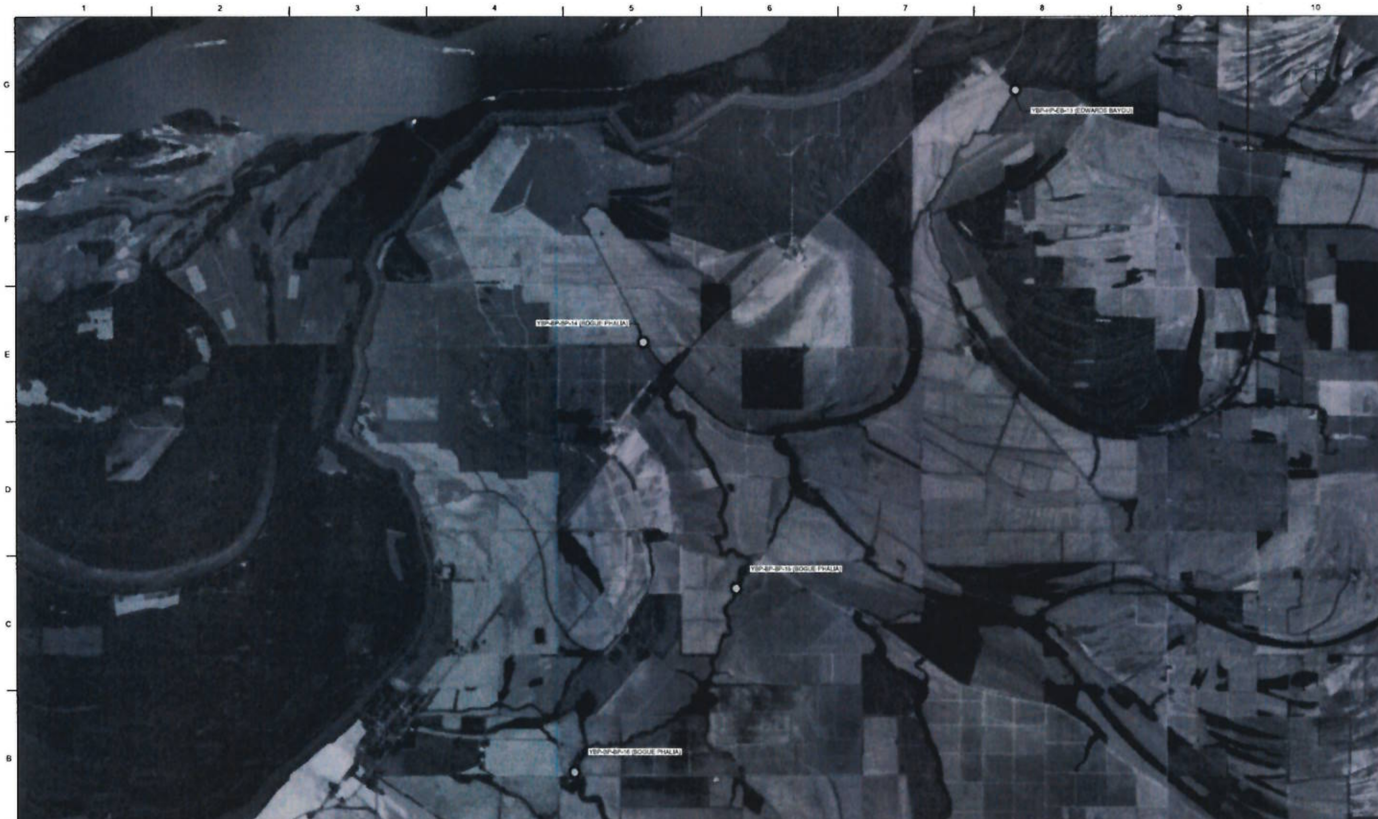
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI

E & C DIVISION
DESIGN BRANCH
ACMF SECTION

FCAR&T YAZOO RIVER BASIN
COAHOMA, BOLIVAR, & WASHINGTON COUNTIES, MS
YAZOO BACKWATER AREA
YAZOO BACKWATER WELL SITES

AERIAL PHOTOGRAPH
WELL SITES

SHEET IDENTIFICATION
G-413



AERIAL PHOTOGRAPH - WELL SITES

SCALE: 1"=2,000'

0 2,000' 4,000'

US Army Corps of Engineers

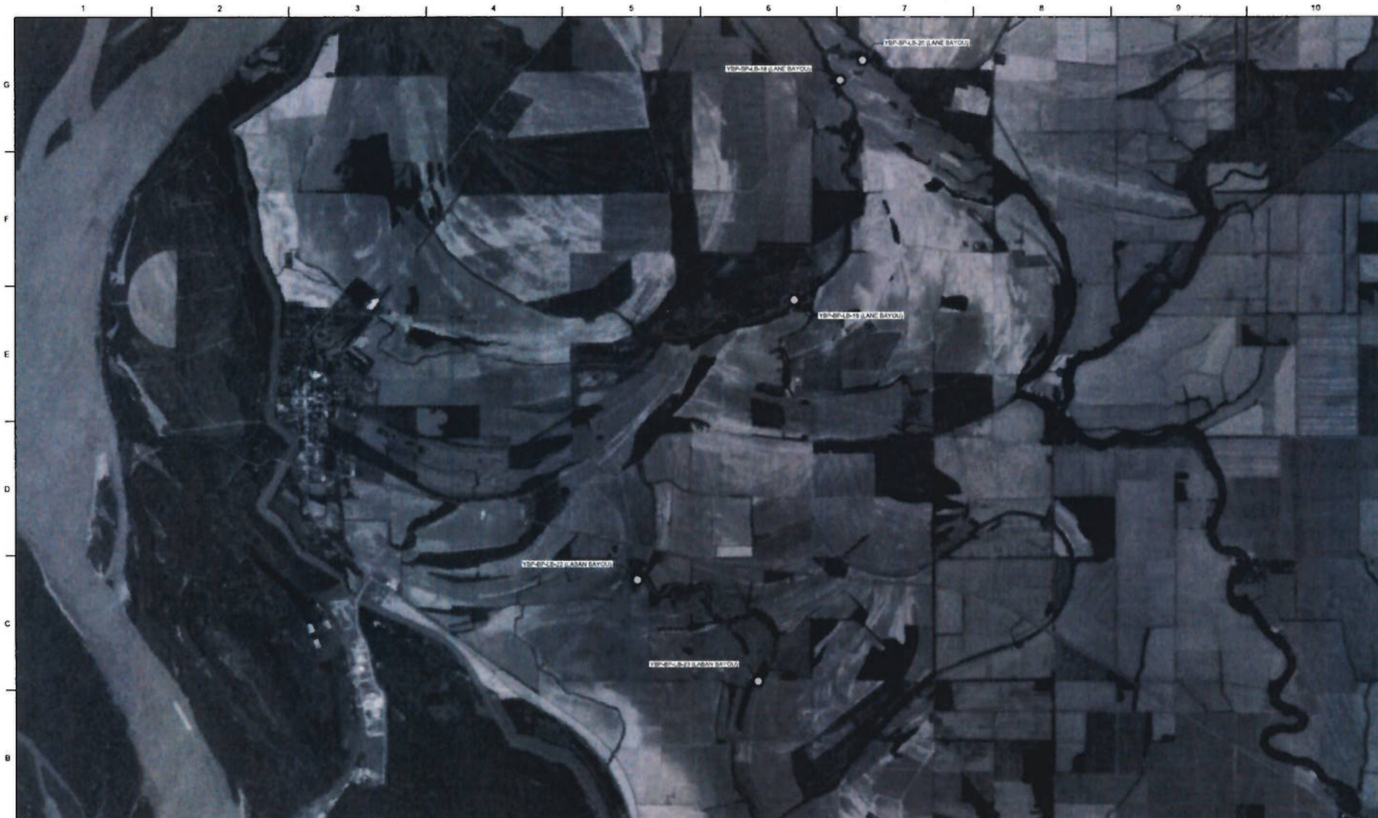
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI

E & C DIVISION
DESIGN BRANCH
ACME SECTION

FC/MR&T YAZOO RIVER BASIN
COAHOMA, BOLIVAR, & WASHINGTON COUNTIES, MS
YAZOO BACKWATER AREA
YAZOO BACKWATER WELL SITES

AERIAL PHOTOGRAPH
WELL SITES

SHEET IDENTIFICATION
G-414



AERIAL PHOTOGRAPH - WELL SITES

SCALE: 1"=2,000'

0 2,000' 4,000'

US Army Corps of Engineers

U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI

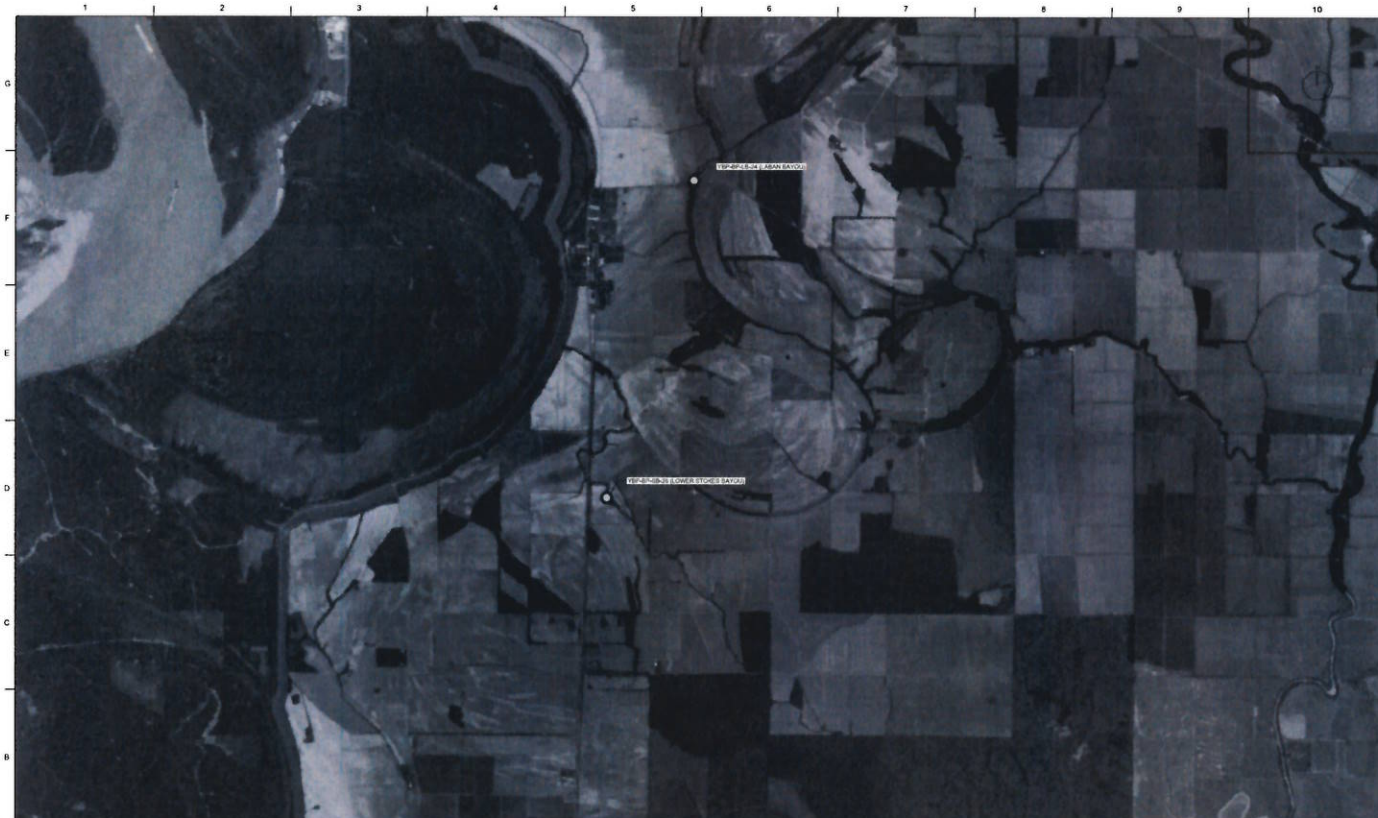
E & C DIVISION
DESIGN BRANCH
ACME SECTION

FCAMRAT YAZOO RIVER BASIN
COAHOMA, BOLIVAR, & WASHINGTON COUNTIES, MS
YAZOO BACKWATER AREA
YAZOO BACKWATER WELL SITES

AERIAL PHOTOGRAPH
WELL SITES

SHEET IDENTIFICATION
G-415

%PRODUCTION DATA 1% %PRODUCTION DATA 2% %PRODUCTION DATA 3% %PRODUCTION DATA 4%



AERIAL PHOTOGRAPH - WELL SITES

SCALE: 1"=2,000'

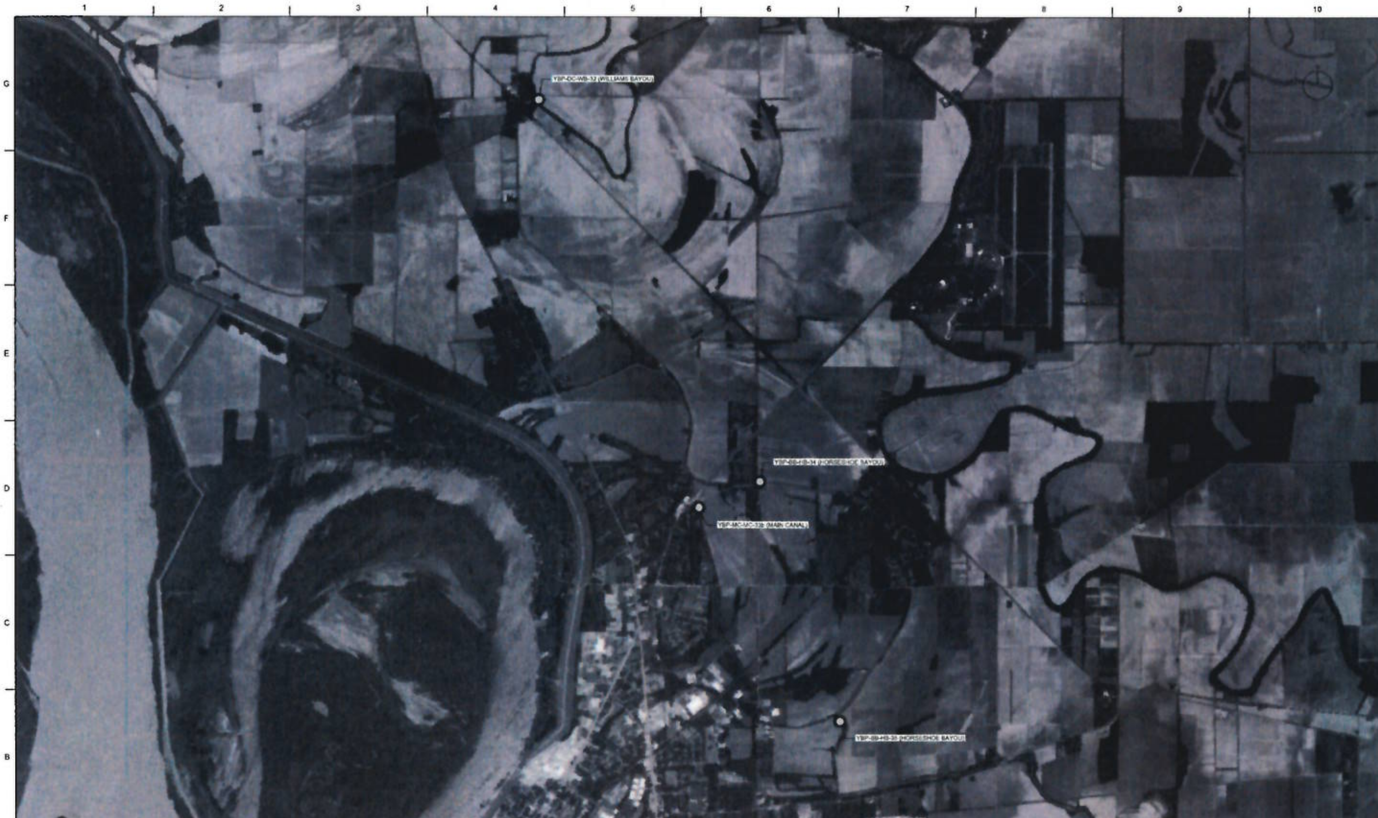
FCMR&T YAZOO RIVER BASIN
COAHOMA, BOLIVAR, & WASHINGTON COUNTIES, MS
YAZOO BACKWATER AREA
YAZOO BACKWATER WELL SITES
AERIAL PHOTOGRAPH
WELL SITES

U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI

E & C DIVISION
DESIGN BRANCH
ACMF SECTION

SHEET IDENTIFICATION
G-416


YBP-DC-SB-27 (STRAIGHT BAYOU)
YBP-DC-SB-28 (BROWNS BAYOU)
YBP-DC-DC-29 (DEER CREEK)
YBP-DC-DC-30 (DEER CREEK)
AERIAL PHOTOGRAPH - WELL SITES
US Army Corps of Engineers
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
L & C DIVISION
DESIGN BRANCH
ACME SECTION
FC&MR&T YAZOO RIVER BASIN
COAHOMA, BOLIVAR, & WASHINGTON COUNTIES, MS
YAZOO BACKWATER AREA
YAZOO BACKWATER WELL SITES
AERIAL PHOTOGRAPH
WELL SITES
SHEET IDENTIFICATION
G-417



AERIAL PHOTOGRAPH - WELL SITES

SCALE: 1"=2,000'

0 2,000' 4,000'

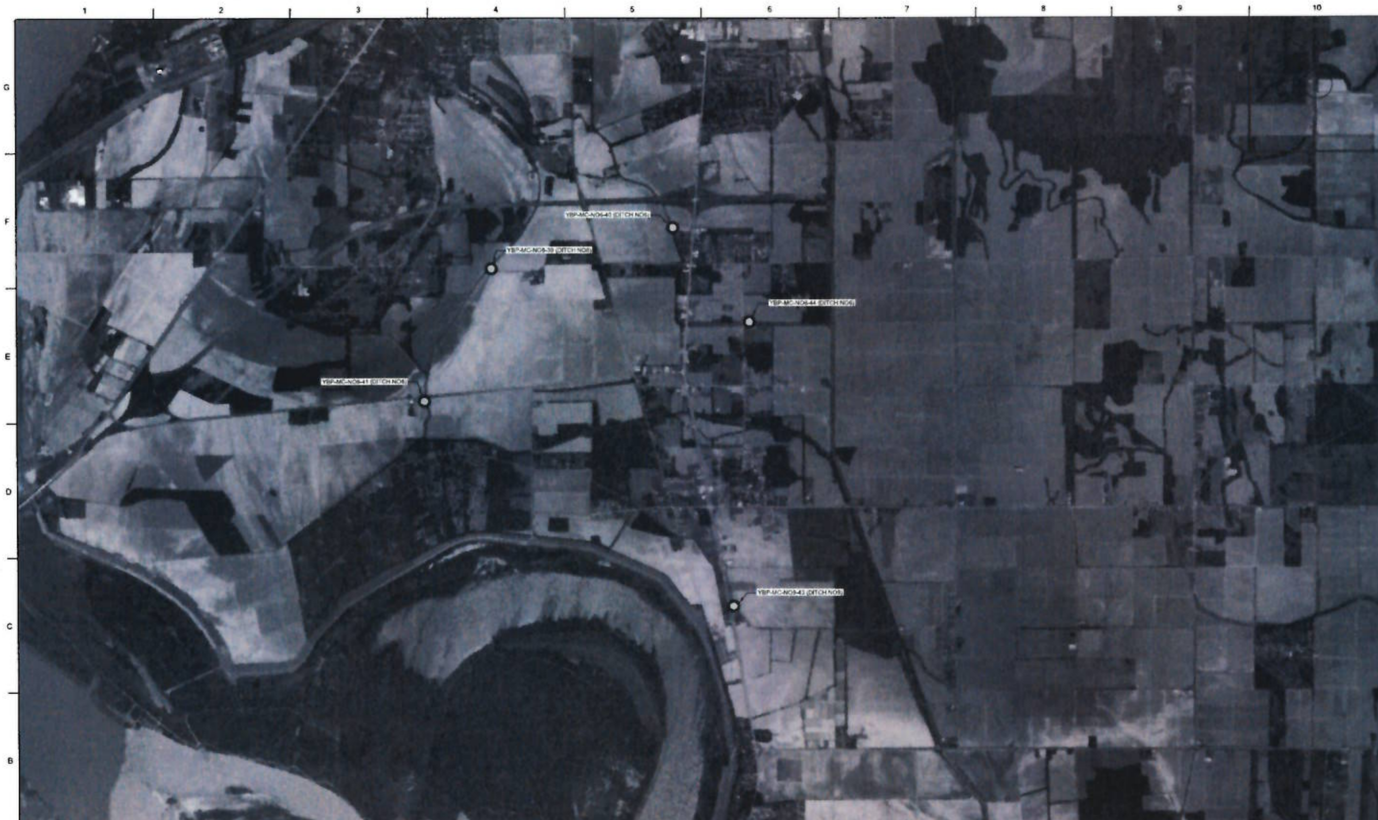
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI

E & C DIVISION
DESIGN BRANCH
ACME SECTION

FC&MR&T YAZOO RIVER BASIN
COAHOMA, BOLIVAR, & WASHINGTON COUNTIES, MS
YAZOO BACKWATER AREA
YAZOO BACKWATER WELL SITES

AERIAL PHOTOGRAPH
WELL SITES

SHEET IDENTIFICATION
G-418



AERIAL PHOTOGRAPH - WELL SITES

SCALE: 1"=2,000'

US Army Corps of Engineers

U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI

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DESIGN BRANCH
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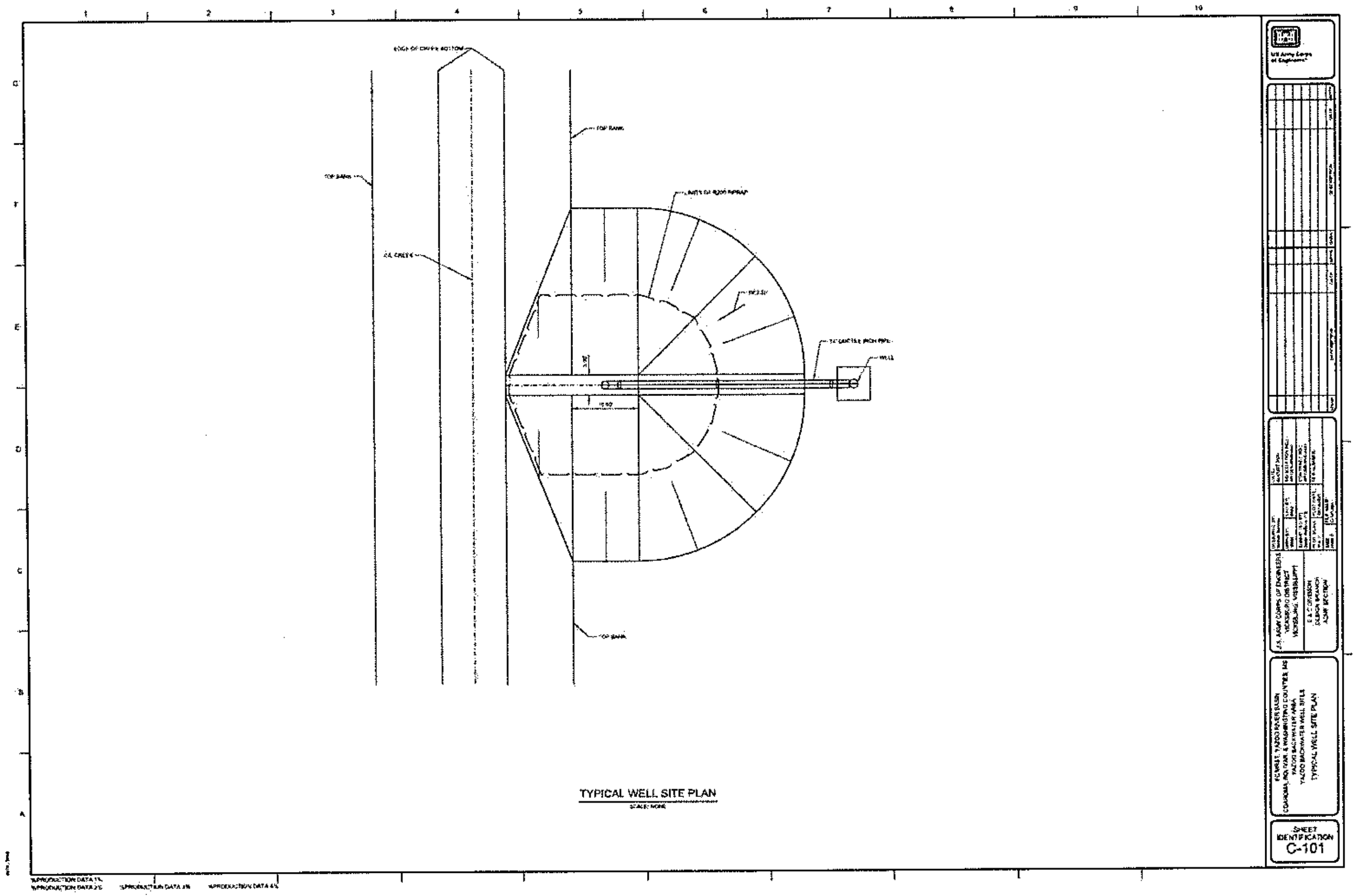
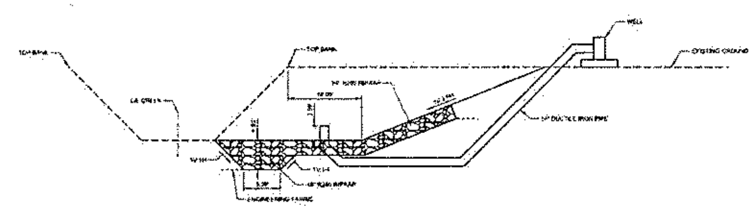
FCARST YAZOO RIVER BASIN
COAHOMA, BOLIVAR, & WASHINGTON COUNTIES, MS
YAZOO BACKWATER AREA
YAZOO BACKWATER WELL SITES

AERIAL PHOTOGRAPH
WELL SITES

SHEET IDENTIFICATION
G-419



TYPICAL WELL SITE PLAN
U.S. ARMY CORPS OF ENGINEERS
VICKSBURG DISTRICT
VICKSBURG, MISSISSIPPI
TYPICAL WELL SITE PLAN
SHEET IDENTIFICATION
C-101



TYPICAL WELL PROFILE

SHEET IDENTIFICATION
C-201